

ADDENDUM NO. 2

TO: All Registered Bidders

DATE: October 16, 2026

PROJECT: Redondo Beach Unified School District – Redondo Union High School Sea Hawk Bowl Improvements – Phase 1 Demolition and Abatement

DSA PROJECT NO. 03-126194

CONTACT: Caleb Pico, Balfour Beatty US, LLC., CPico@BalfourBeattyUS.com

This Addendum forms a part of the Contract Documents for the Project described above and shall supersede referenced sections of the original Bidding Documents unless specifically noted otherwise. This Addendum is an integral part of said Bidding Documents and shall be acknowledged in the Contractor's Bid Proposal form. Failure to acknowledge receipt of this Addendum in the Bid may cause the Bid to be rejected.

This addendum is divided into six (6) parts: Instructions and Procedures, Project Specifications, Drawings, Changes to Prior Addenda, Other Documents, and Pre-Bid Questions.

I. INSTRUCTIONS AND PROCEDURES

1. The following changes, omissions and/or additions to the Bid Specification shall apply to proposals made for and to the execution of the various parts of the work affected thereby, and all other conditions shall remain the same.
2. Careful note of the Addendum shall be taken by all parties of interest so that the proper allowance may be made in all computations, estimates and all trades affected shall be fully advised in the performance of the work which will be required of them.
3. In case of conflict between the Drawings, Specifications, and this Addendum, this Addendum shall govern.
4. Upon receipt of this Addendum, please acknowledge receipt on the bid form.

II. PROJECT SPECIFICATION

Revise the project name in its entirety and replace it with the following:

1. PROJECT: REDONDO UNION HIGH SCHOOL SEA HAWK BOWL IMPROVEMENTS – PHASE 1 DEMOLITION AND ABATEMENT

III. DRAWINGS

Delete the previously issued drawing sheets and replace them with the following:

1. A1.2 FIRE ACCESS SITE PLAN & CODE ANALYSIS – Rev A 10/14/2026
2. A1.3 ENLARGED SITE PLANS – Rev A 10/14/2026
3. A11.10 FLOOR PLANS – Rev A 10/14/2026
4. A11.20 REFLECTED CEILING PLANS – Rev A 10/14/2026
5. A21.20 ENTRANCE – EXTERIOR ELEVATIONS – Rev A 10/14/2026
6. A21.30 WEST RESTROOM BLDG EXTERIOR ELEVATIONS – Rev A 10/14/2026
7. A21.31 WEST RESTROOM BLDG EXTERIOR ELEVATIONS – Rev A 10/14/2026
8. A31.10 BUILDING SECTIONS – Rev A 10/14/2026
9. A52.1 SCHEDULES – Rev A 10/14/2026
10. A60.1 SITE DETAILS – Rev A 10/14/2026
11. A60.2 SITE DETAILS – Rev A 10/14/2026
12. A60.3 ACCESSIBILITY DETAILS – Rev A 10/14/2026
13. A62.1 ROOF ASSEMBLY DETAILS – Rev A 10/14/2026
14. A62.3 ROOF AND CEILING DETAILS – Rev A 10/14/2026
15. C3.00 HORIZONTAL CONTROL PLAN – WEST – Rev A 10/14/2026
16. C6.00 CIVIL DETAILS – Rev A 10/14/2026
17. S-0.1 GENERAL NOTES – Rev A 10/14/2026
18. S-0.2 GENERAL NOTES – Rev A 10/14/2026
19. S-1.1 TYP REBAR & S.O.G – Rev A 10/14/2026
20. S-1.4 TYPICAL DETAILS – METAL STUD – Rev A 10/14/2026
21. S-2.1 FOUNDATION PLAN – Rev A 10/14/2026
22. S-3.1 ROOF FRAMING PLAN – Rev A 10/14/2026
23. S-4.1 FOUNDATION DETAILS – Rev A 10/14/2026
24. S-5.1 FRAMING DETAILS – Rev A 10/14/2026

IV. CHANGES TO PRIOR ADDENDA

1. None

V. OTHER DOCUMENTS

Delete the previously issued Exhibit B-3: Project-Specific Scope of Work in its entirety and replace it with the following:

1. EXHIBIT B-3: PROJECT SPECIFIC SCOPE OF WORK REDONDO UNION HIGH SCHOOL SEA HAWK BOWL IMPROVEMENTS PROJECT – PHASE 1 DEMOLITION AND ABATEMENT

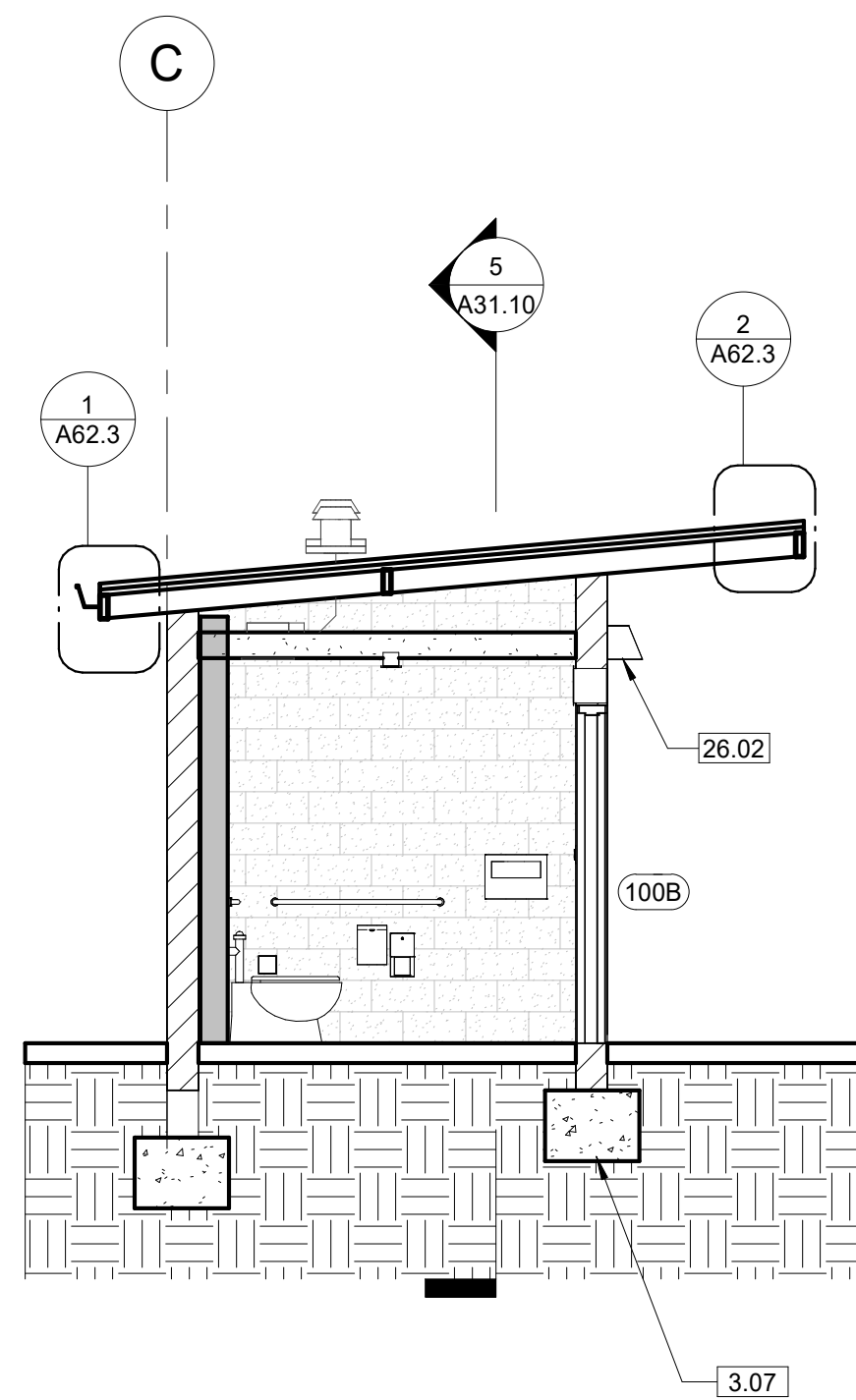
VI. PRE-BID QUESTIONS

1. None

END OF ADDENDUM NO. 2 NARRATIVE

6 HUTTON CENTRE DR, SUITE 1150 SANTA ANA, CA 92707
T 949.809.3380 WWW.SVA-ARCHITECTS.COM

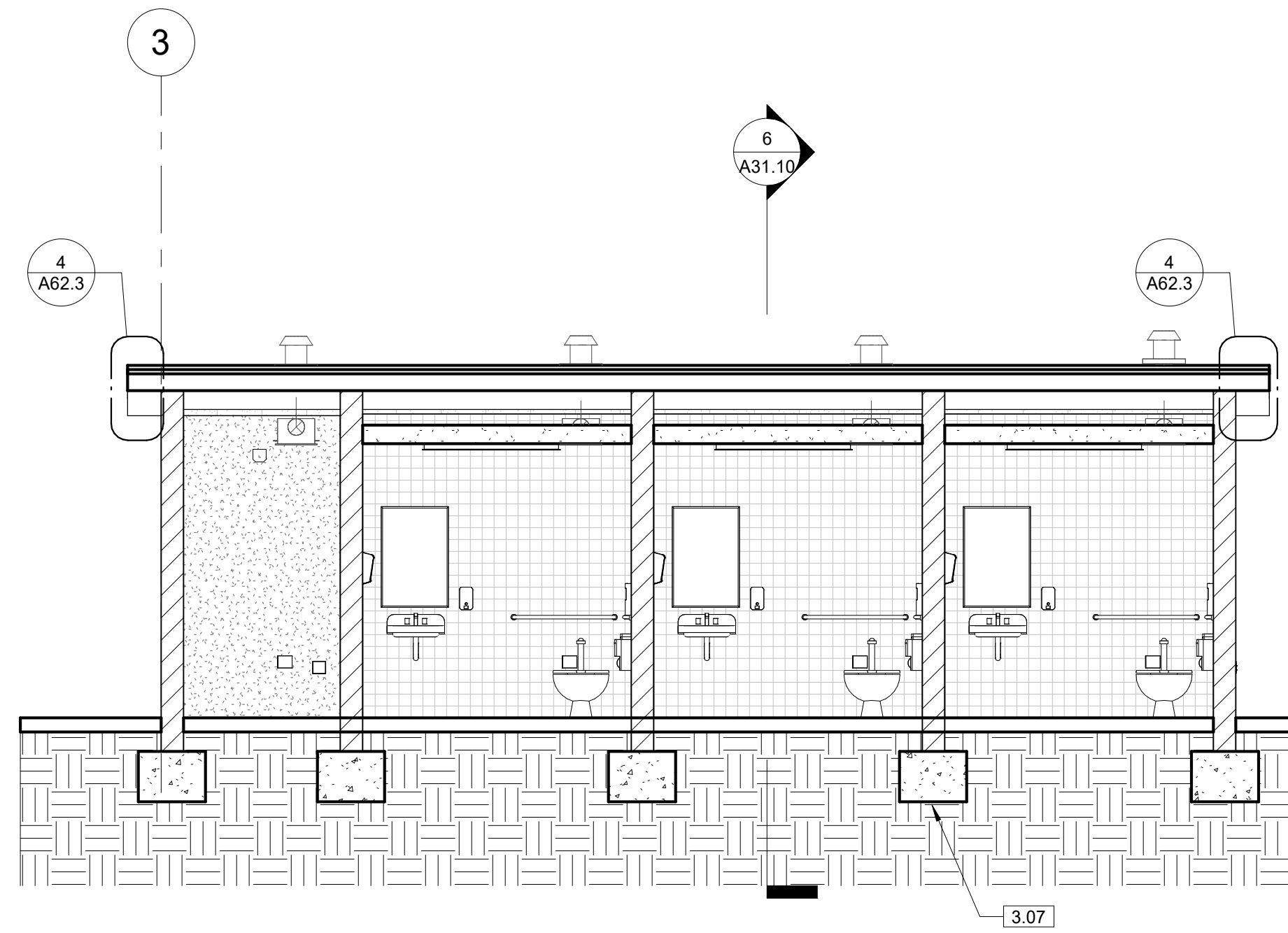
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EAST RESTROOM - BUILDING SECTION 6

1/4" = 1'-0"

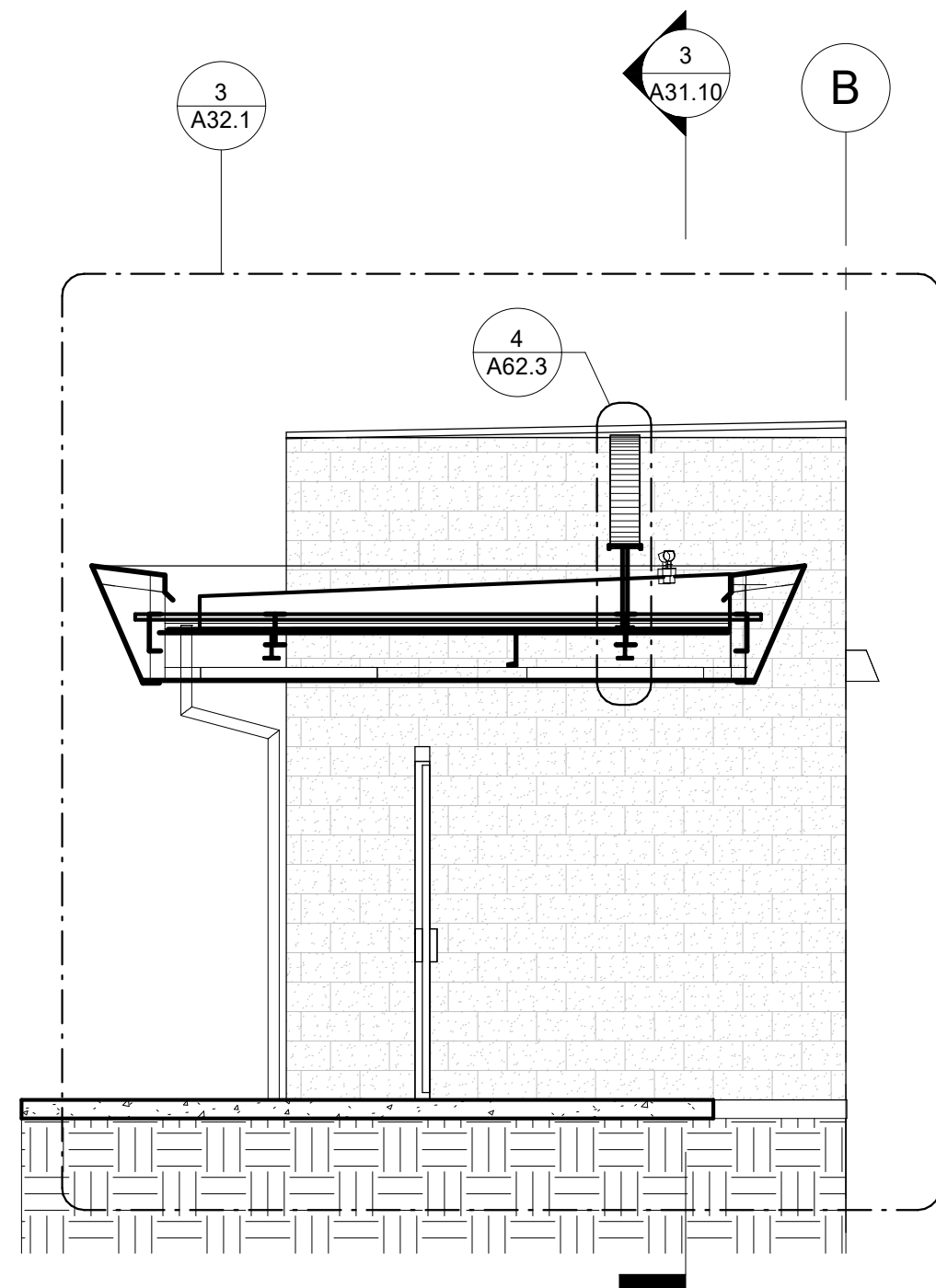
6



EAST RESTROOM - BUILDING SECTION 5

1/4" = 1'-0"

5



ENTRY GATE - SECTION 4

1/4" = 1'-0"

4



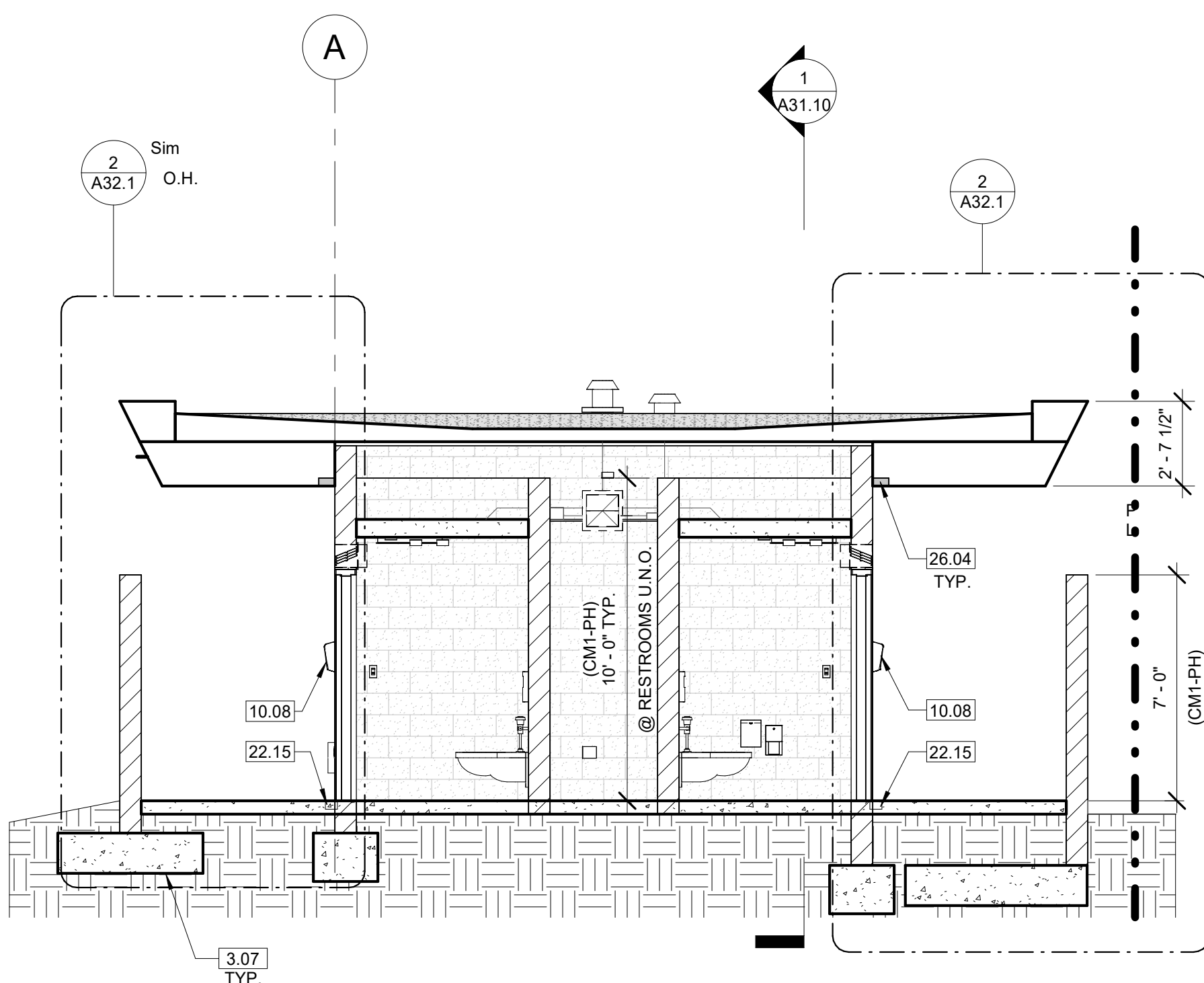
ENTRY GATE - SECTION 3

1/4" = 1'-0"

3

	EARTH
	8" CMU BLOCK WALL PER WALL TYPES SHEET A61.2
	METAL STUD W/ TILE FINISH PER WALL TYPES SHEET A61.2
	METAL STUD WALL W/ GYPSUM BOARD PER WALL TYPES SHEET A61.2
	ROOFING WITH R-30 CONTINUOUS RIGID INSULATION OVER W2 METAL DECK
	GYPSUM BOARD CEILING
	8" CMU BLOCK WALL
	TILE PER SCHEDULE
	GYPSUM BOARD, FINISH PER SCHEDULE

BUILDING SECTIONS LEGEND



Technical drawings of three gate types:

- F FLUSH:** A side elevation of a flush gate. It shows a height of 7'-10" and a width labeled "WIDTH PER SCHED.". A dashed line indicates the internal structure. A note points to the top: "TRANSOM LOUVERS PER TYPICAL UNLESS NOTED OTHERWISE". A circular callout shows a 4x4 grid with "A63.1" in the center.
- SG SINGLE GATE:** A side elevation of a single gate with vertical slats.
- DG DOUBLE GATE:** A side elevation of a double gate with two panels. It features "PANIC HARDWARE" on the top of each panel and a "10' MIN. CLR." (clearance) at the bottom.

ROOM FINISH SCHEDULE																		
ROOM NUMBER	ROOM NAME	FLOOR		BASE	WALLS								CEILING			COMMENTS	ROOM NUMBER	
		MAT.	FIN.		NORTH		EAST		SOUTH		WEST		MATERIAL	FINISH	HEIGHT			
					MAT.	FIN.	MAT.	FIN.	MAT.	FIN.	MAT.	FIN.						
100A	TOILET A	CONC	EP	EP-1	CMU	PT-1	CMU	PT-1	CBU	CT-1	CMU	PT-1	GBCM	SCPT-1	8'-2"		100A	
100B	TOILET B	CONC	EP	EP-1	CMU	PT-1	CMU	PT-1	CBU	CT-1	CMU	PT-1	GBCM	SCPT-1	8'-2"		100B	
100C	TOILET C	CONC	EP	EP-1	CMU	PT-1	CMU	PT-1	CBU	CT-1	CMU	PT-1	GBCM	SCPT-1	8'-2"		100C	
100E	UTIL & STOR.	CONC	CS-1	--	CMU	PT-1	CMU	PT-1	GBCM	PT-1	CMU	PT-1	EXP	--	--		100D	
100F	HAND WASH	CONC	CS-1	CTB-1	CMU	PT-1	CBU	CT-2	CMU	PT-1	CBU	CT-2	AC	GC	8'-0"		100E	
100F	ACC TOILET	CONC	EP	EP-1	CMU	PT-1	CMU	PT-1	CMU	PT-1	CMU	PT-1	GBCM	SCPT-1	8'-2"		100F	
100G	S TOILET	CONC	EP	EP-1	CMU	PT-1	GBCM	PT-1	--	--	GBCM	PT-1	GBCM	SCPT-1	8'-2"	AMBULATORY RESTROOM STALL	100G	
100J	TOILET	CONC	EP	EP-1	CMU	PT-1	CMU	PT-1	--	--	CMU	PT-1	GBCM	SCPT-1	8'-2"		100H	
100J	TOILET	CONC	EP	EP-1	CMU	PT-1	CMU	PT-1	--	--	CMU	PT-1	GBCM	SCPT-1	8'-2"		100J	
100K	TOILET	CONC	EP	EP-1	CMU	PT-1	CMU	PT-1	--	--	GBCM	PT-1	GBCM	SCPT-1	8'-2"		100K	
100L	CUST. / UTILITY & STORAGE	CONC	--	--	CMU	PT-1	CMU/GBCM	PT-1	CMU/GBCM	PT-1/FRP	CMU	PT-1/FRP	EXP	--	--		100L	
100M	TOILET	CONC	EP	EP-1	--	--	CMU	PT-1	CMU	PT-1	CMU	PT-1	GBCM	SCPT-1	8'-2"		100M	
100N	TOILET	CONC	EP	EP-1	--	--	CMU	PT-1	CMU	PT-1	CMU	PT-1	GBCM	SCPT-1	8'-2"		100N	
100P	TOILET	CONC	EP	EP-1	--	--	CMU	PT-1	CMU	PT-1	CMU	PT-1	GBCM	SCPT-1	8'-2"		100P	
100Q	S TOILET	CONC	EP	EP-1	--	--	GBCM	PT-1	CMU	PT-1	GBCM	PT-1	GBCM	SCPT-1	8'-2"	AMBULATORY RESTROOM STALL	100Q	
100R	ACC TOILET	CONC	EP	EP-1	CMU	PT-1	CMU	PT-1	CMU	PT-1	CMU	PT-1	GBCM	SCPT-1	8'-2"		100R	
100S	CHASE	CONC	CS-1	--	CMU	PT-1	CMU	PT-1	CMU	PT-1	CMU	PT-1	EXP	--	--		100S	

1. OPERABLE BY A SINGLE EFFORT, NOT GRASPING WRIST MOVEMENT (LEVERS, PANIC DEVICES, OR PULLS).

2. 5-LB CLOSURE PRESSURE AT INTERIOR DOORS.

3. 5-LB CLOSURE PRESSURE AT EXTERIOR DOORS.

4. 5-LB CLOSURE PRESSURE AT FIRE DOORS.

5. WHEN FIRE DOORS ARE REQUIRED, THE MAXIMUM EFFORT TO OPERATE THE DOOR BE INCREASED TO THE MINIMUM ALLOWABLE BY THE APPROPRIATE ADMINISTRATIVE AUTHORITY, NOT TO EXCEED 15 POUNDS.

MAY

12. ALL DOORS INDICATED AS FIRE-RATED ASSEMBLIES TO HAVE CLOSURES AND SMOKE SEALS.

13. ON DOORS IN ALUMINUM FRAMES, PROVIDE ARMORED STRIKE ON JAMB OF ALUMINUM FRAME.

14. EXIT DOORS MUST HAVE A READILY VISIBLE, DURABLE SIGN ON OR ADJACENT TO THE DOOR RATING. "THIS DOOR TO REMAINED UNLOCKED DURING BUSINESS HOURS." THE SIGN SHALL HAVE 1" HIGH LETTERS ON A CONTRASTING BACKGROUND. THE LOCKING DEVICE MUST BE OF A TYPE THAT WILL BE READILY DISTINGUISHABLE AS LOCKED.

15. DOORS SHALL BE A MINIMUM OF 36" WIDE X 80" HIGH WITH NO SINGLE LEAF EXCEEDING 4'-0" IN WIDTH.

16. BOTTOM 10" OF DOOR TO HAVE SMOOTH UNINTERRUPTED SURFACE FOR OPENING BY WHEELCHAIR FOOT REST.

17. CENTER OF HARDWARE TO BE 30" TO 40" ABOVE FLOOR. LATCHING AND LOCKING DOOR TO BE OPERABLE WITH A SINGLE EFFORT BY LEVER OR PUSH-PULL TYPE HARDWARE.

18. PROVIDE DOOR SYMBOLS ON DOORWAYS LEADING TO SANITARY FACILITIES (SEE TOILET SIGN). PROVIDE DOOR SYMBOLS ON DOORS AT 60" HEIGHT, AND FINISH IN COLOR CONTRASTING TO THAT OF THE DOOR.

19. THRESHOLDS SHALL BE 1/2" HIGH MAXIMUM WITH A 2:1 DEGREE BEVELED EDGE.

20. THRESHOLD HEIGHT BETWEEN 1/4" AND 1/2" SHALL BE BEVELED AT MAXIMUM 50% SLOPE.

21. ALL EXTERIOR DOOR SHALL HAVE PERIMETER DOOR SOUND SEAL/GASKET.

22. EACH DOOR IN A MEANS OF EGRESS FROM A GROUP A, OR ASSEMBLY AREA NOT CLASSIFIED AS AN ASSEMBLY OCCUPANCY, HAVING AN OCCUPANT LOAD OF 80 OR MORE PERSONS, SHALL BE PROVIDED WITH A LATCH OR LOCK UNLESS IT IS PANIC HARDWARE OR FIRE EXIT HARDWARE. BUILDING CODE 1008.1.10 FIRE CODE 1008.1.10

AL	-	ALUMINUM
AN	-	ANODIZED CLEAR
BZ	-	ANODIZED BRONZE
CC	-	CONCEALED CLOSER
CL	-	CLEAR
FC	-	FLOOR CLOSER
FF	-	FACTORY FINISH
FLFD	-	FUSIBLE LINK FIRE DAMPER
FP	-	FIRE PAINT
FPF	-	FACTORY PRIMED, FIRE PAINTED
FRA	-	FIRE RATED ASSEMBLY
FS	-	FIRE STAINED
GA	-	GALVANIZED
GL	-	GLASS OR GLAZING
GS	-	GALVANIZED STEEL
H	-	HOLLOW METAL
LMV	-	FLUOROPOLYMER FINISH (SEE SPECS)
LY	-	LOUVER
MR	-	MILL FINISH
MR	-	MIRRORED GLASS
MAG	-	MAGNETIC HOLD OPEN
P	-	PAINTED
PG	-	PANT GRADE
PH	-	PANIC HARDWARE
PR	-	PAIR
SCW	-	SOLID CORE WOOD
SG	-	STEIN GRAIN
SMC	-	SURFACE MOUNTED CLOSER
ST	-	STEEL
SS	-	STAINLESS STEEL
TEMP	-	TEMPERED

DOOR GENERAL NOTES

C 1	EXPOSED CONCRETE FLOOR SLAB; CLEANED AND SEALED
B 1	COVE BASE: Daltile Color Wheel Classic - 6" Glossy Ceramic S

T RESTROOM WALL TILE T-1 - DAL TILE, CERAMIC WALL TILE; DTEL41; 4" X 16"

 GENERAL AREA PAINT COLOR P-1 - DUNN EDWARDS, DEW341 SWISS COFFEE

 TAN PAINT, EXTERIOR OF EXISTING PRESS BOX

REDONDO UNION HIGH SCHOOL SEA HAWK BOWL IMPROVEMENTS

1 SEA HAWK WAY, REDONDO BEACH, CA 90277

[illegible]

PROJECT NO: 2025-40137

ISSUANCE: A#03-126194

DATE ISSUED: 06/22/2026

SCALE: 1/4" = 1'-0"

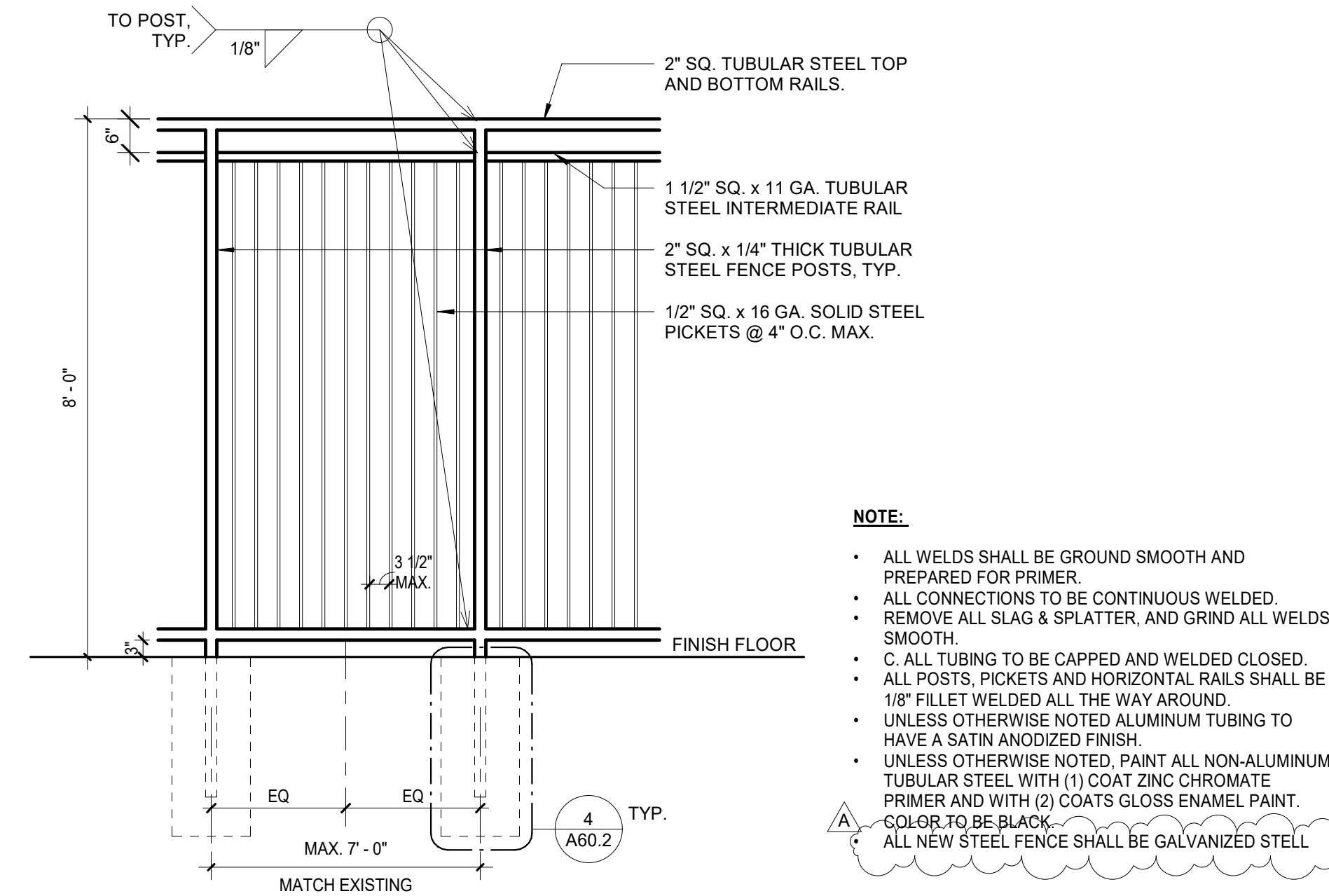
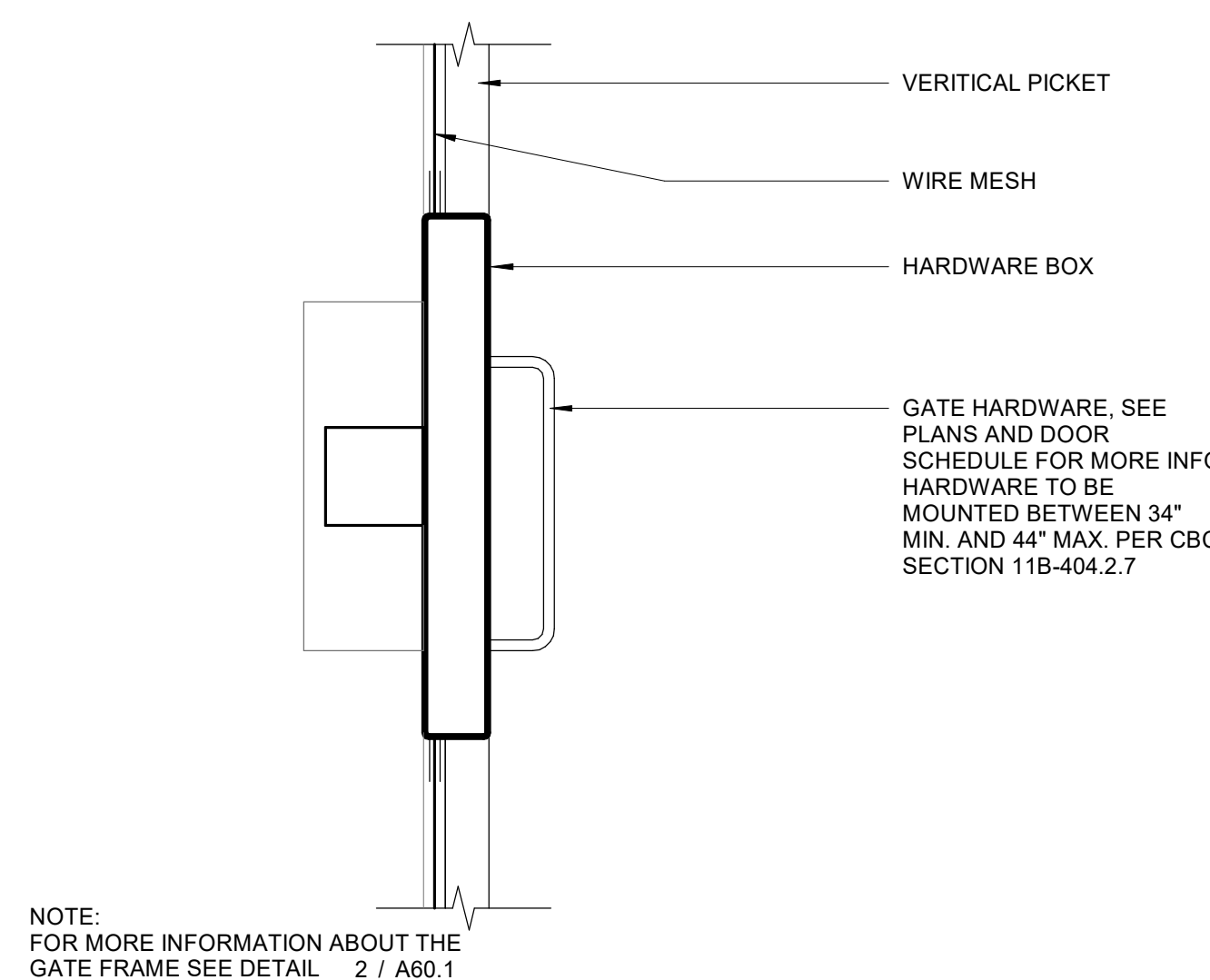
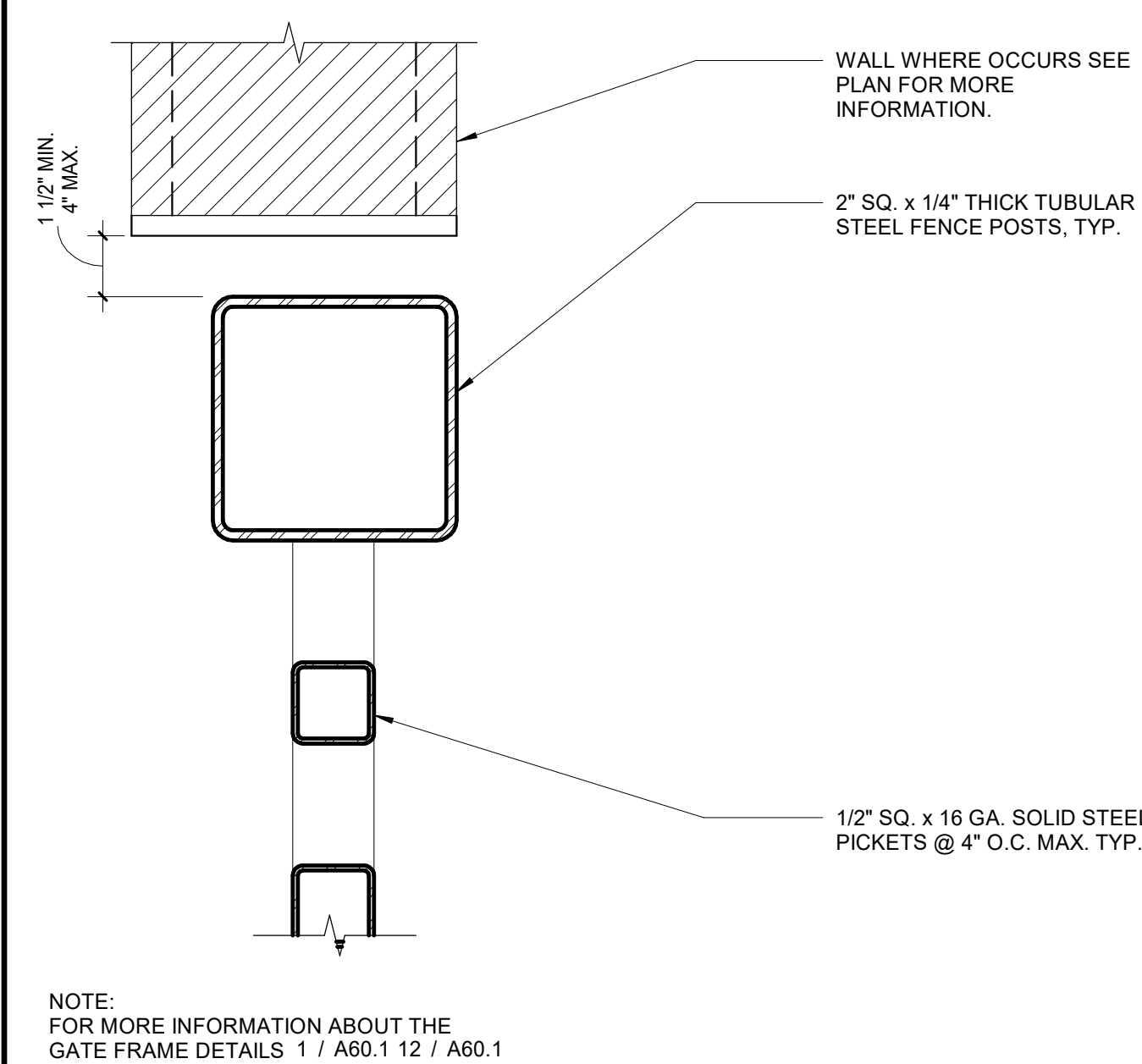
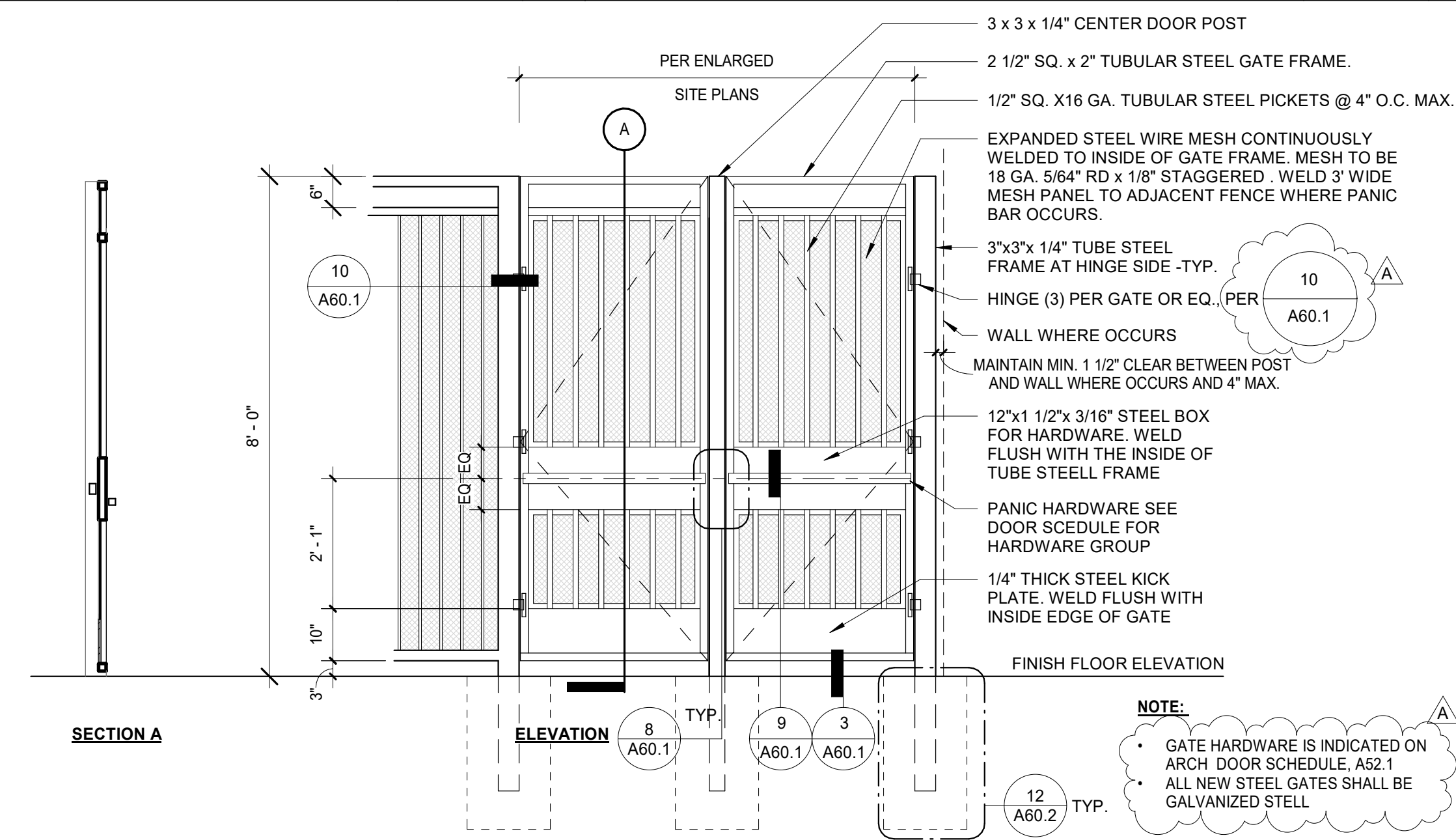
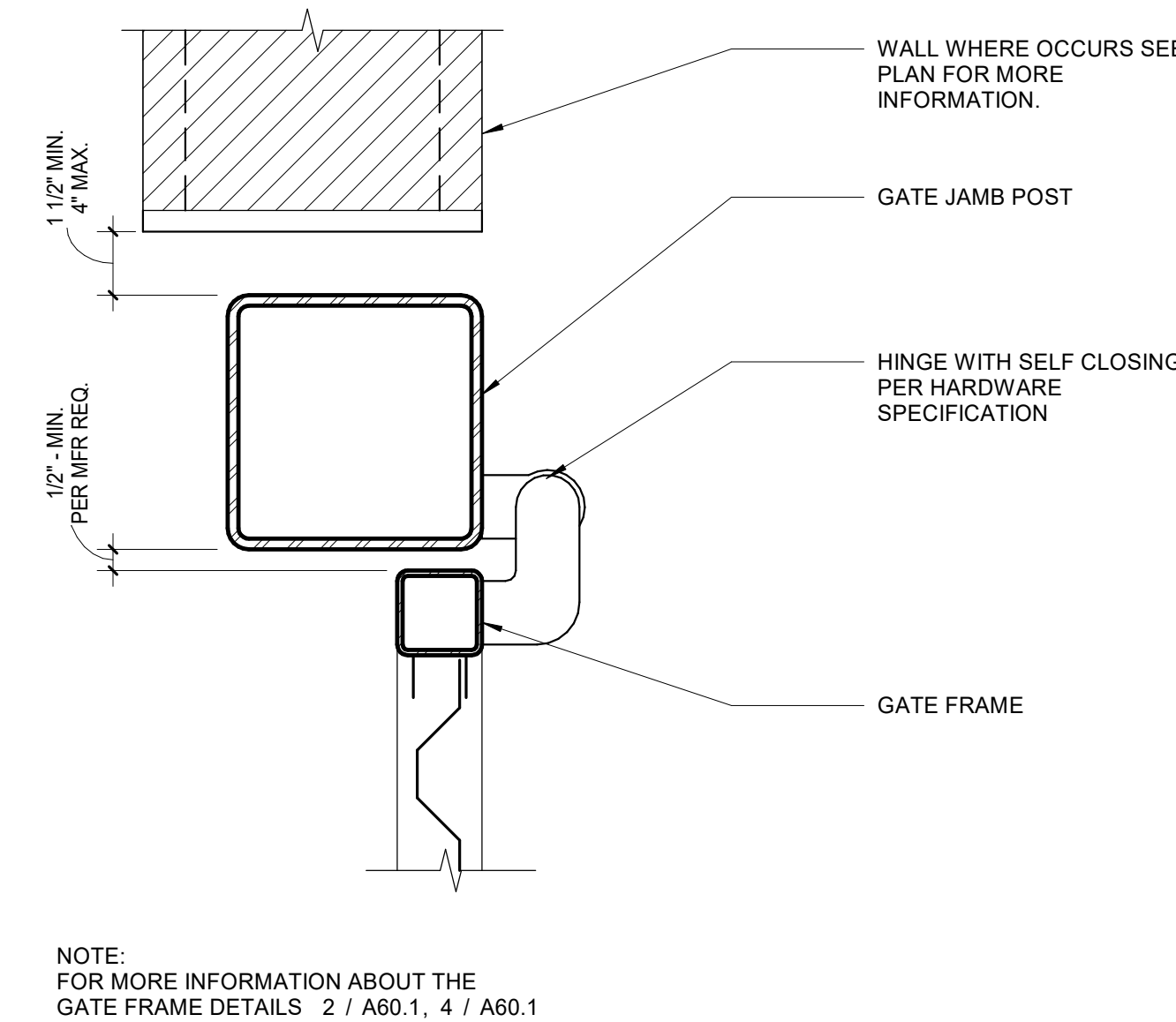
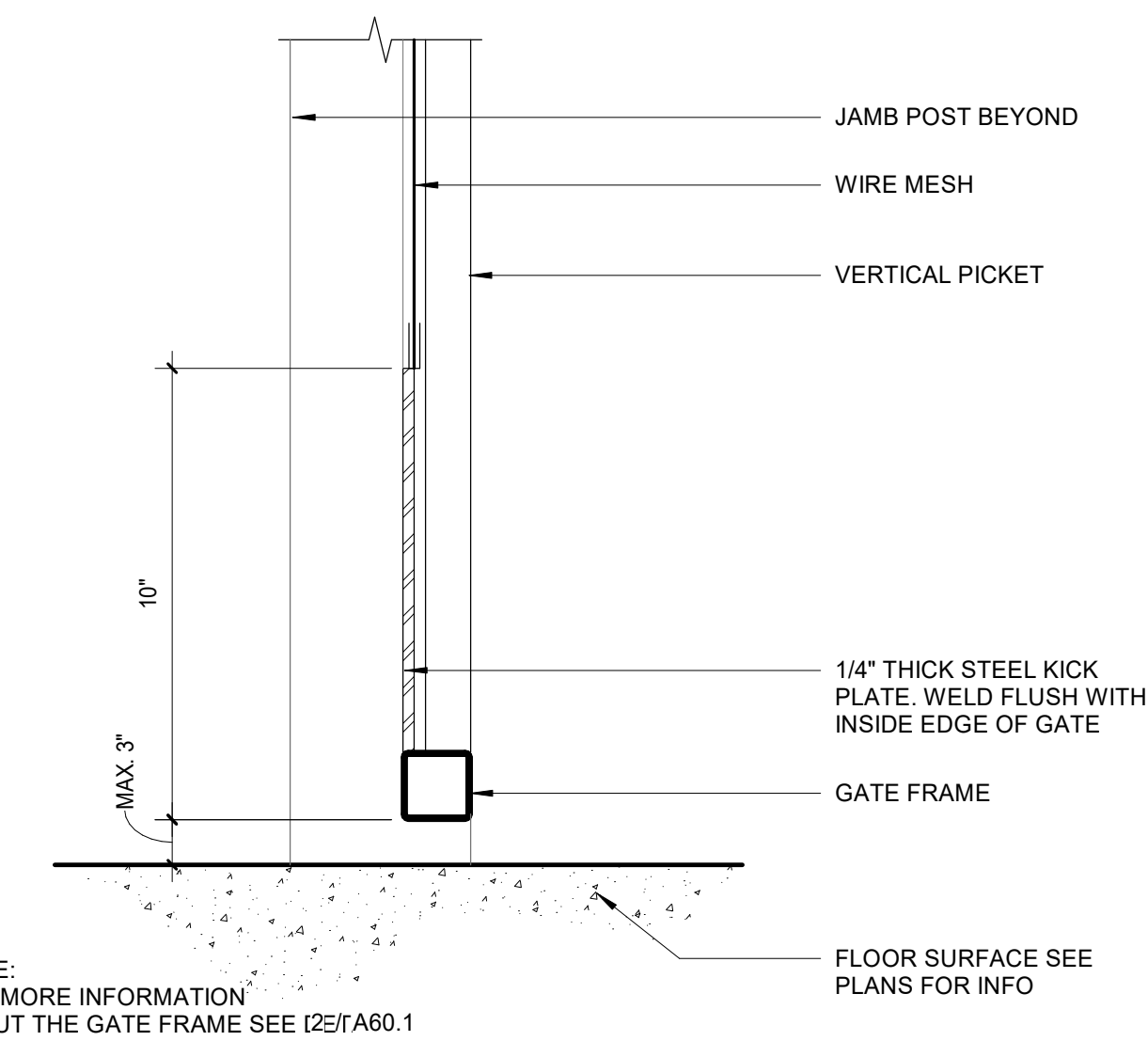
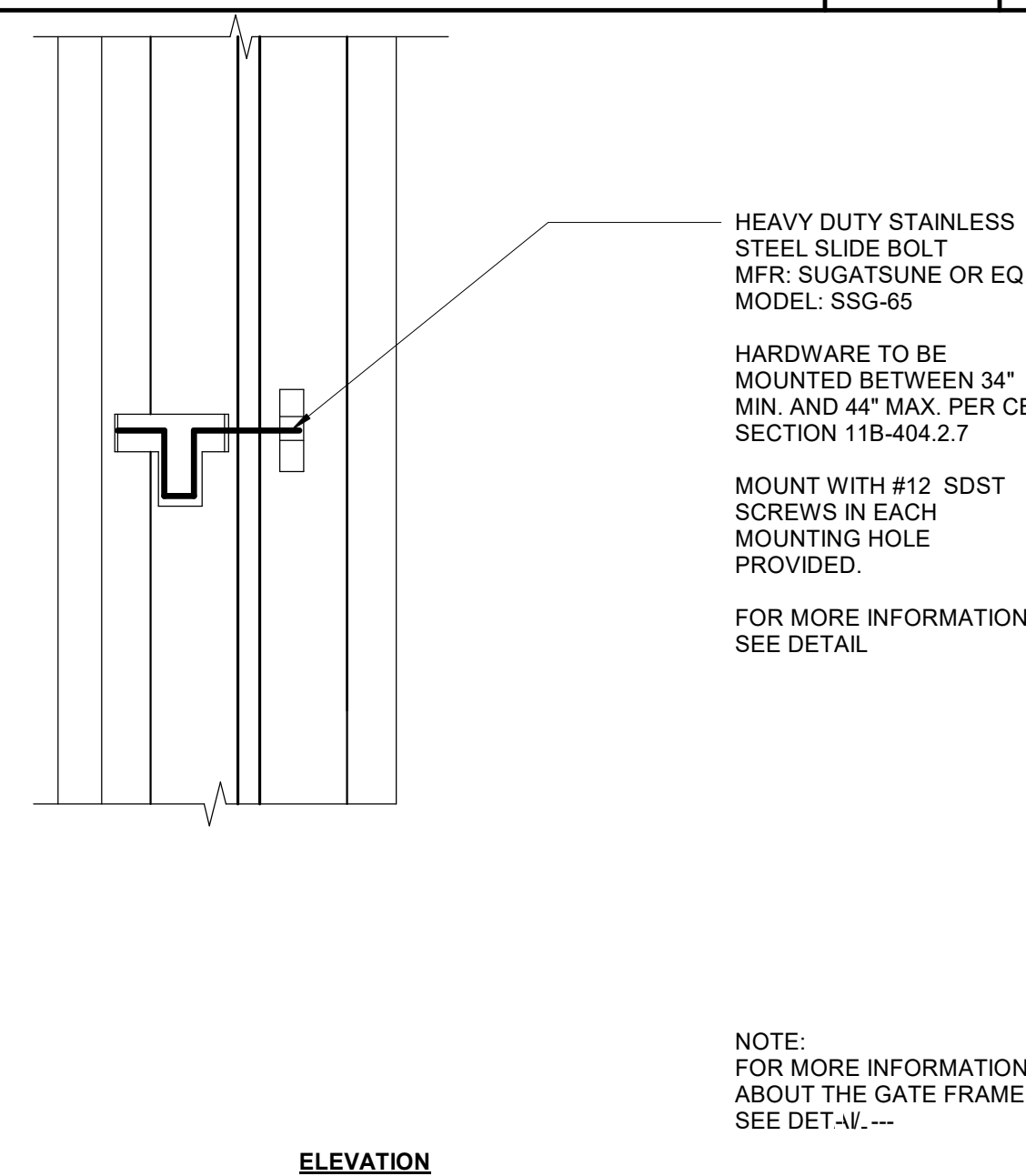
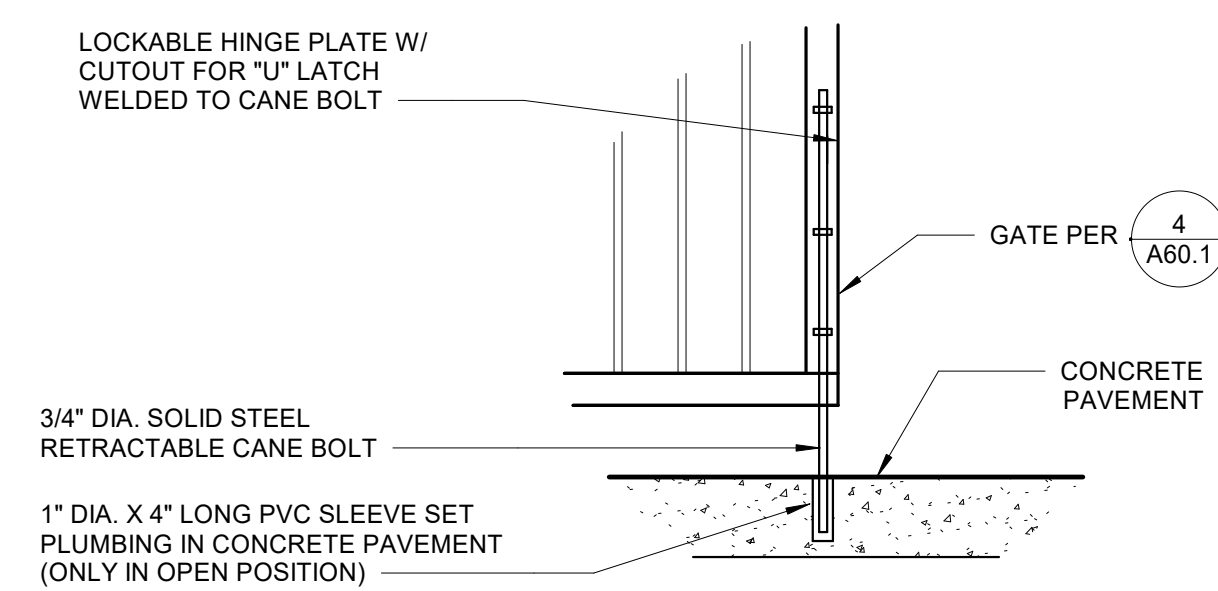
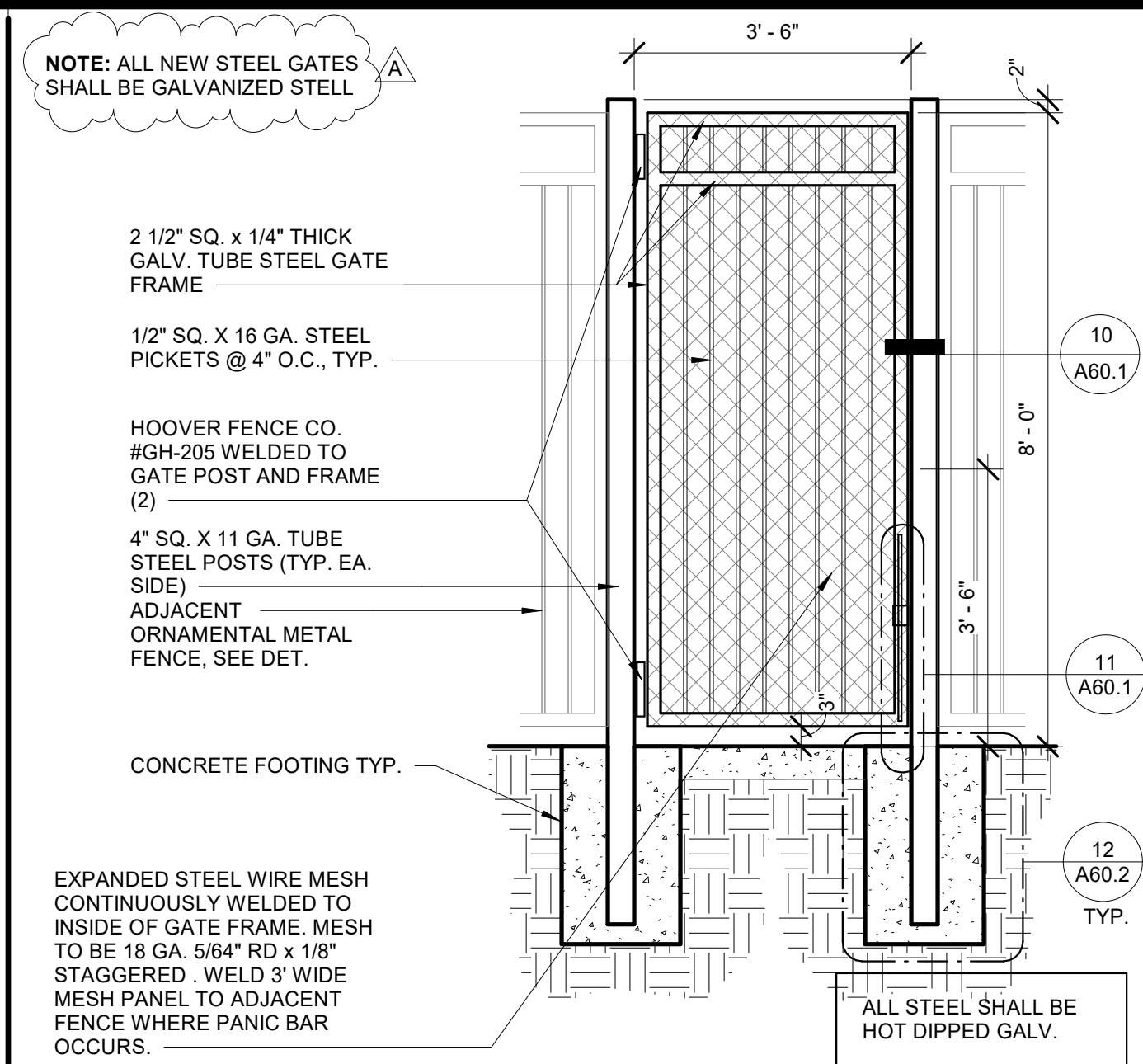
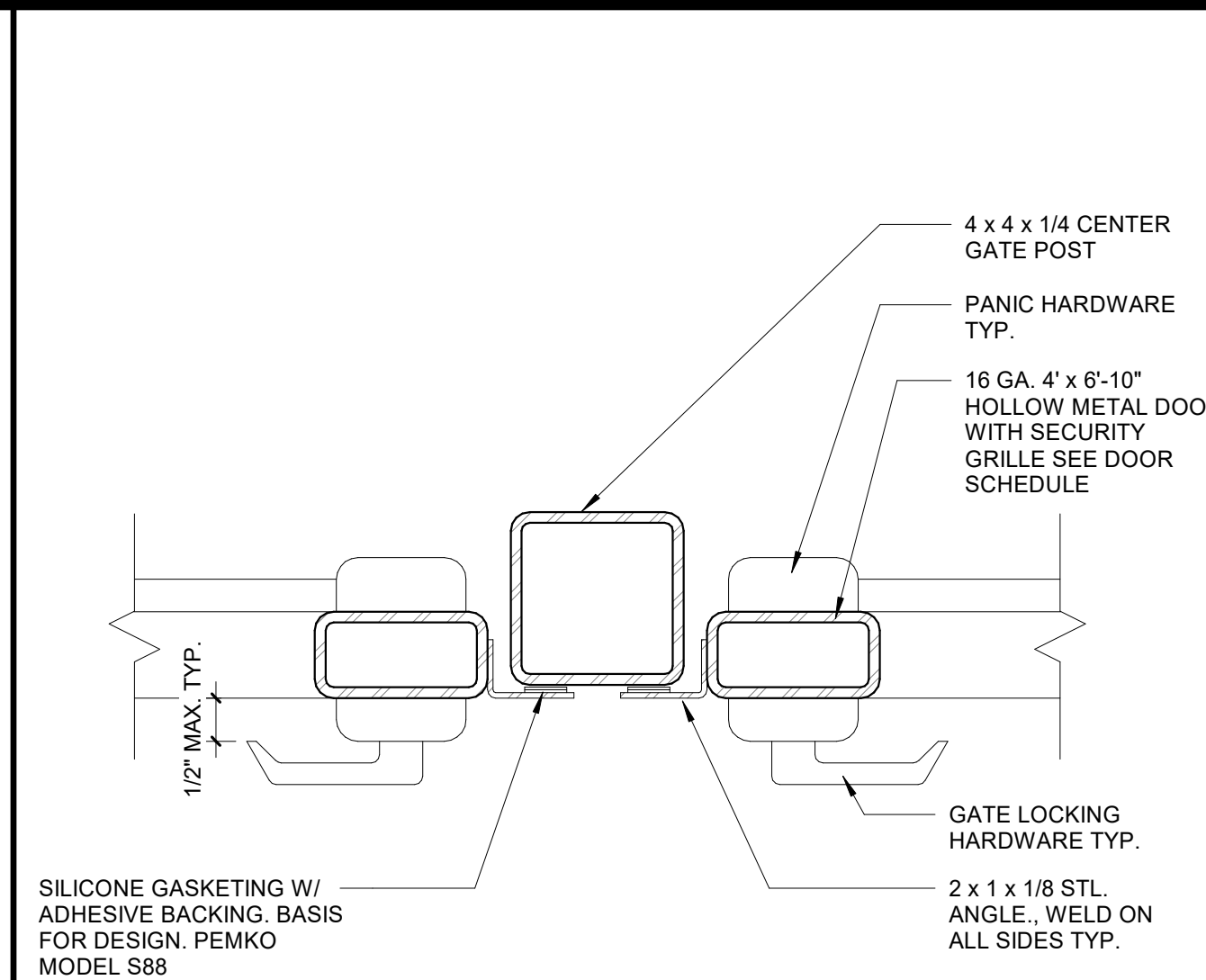
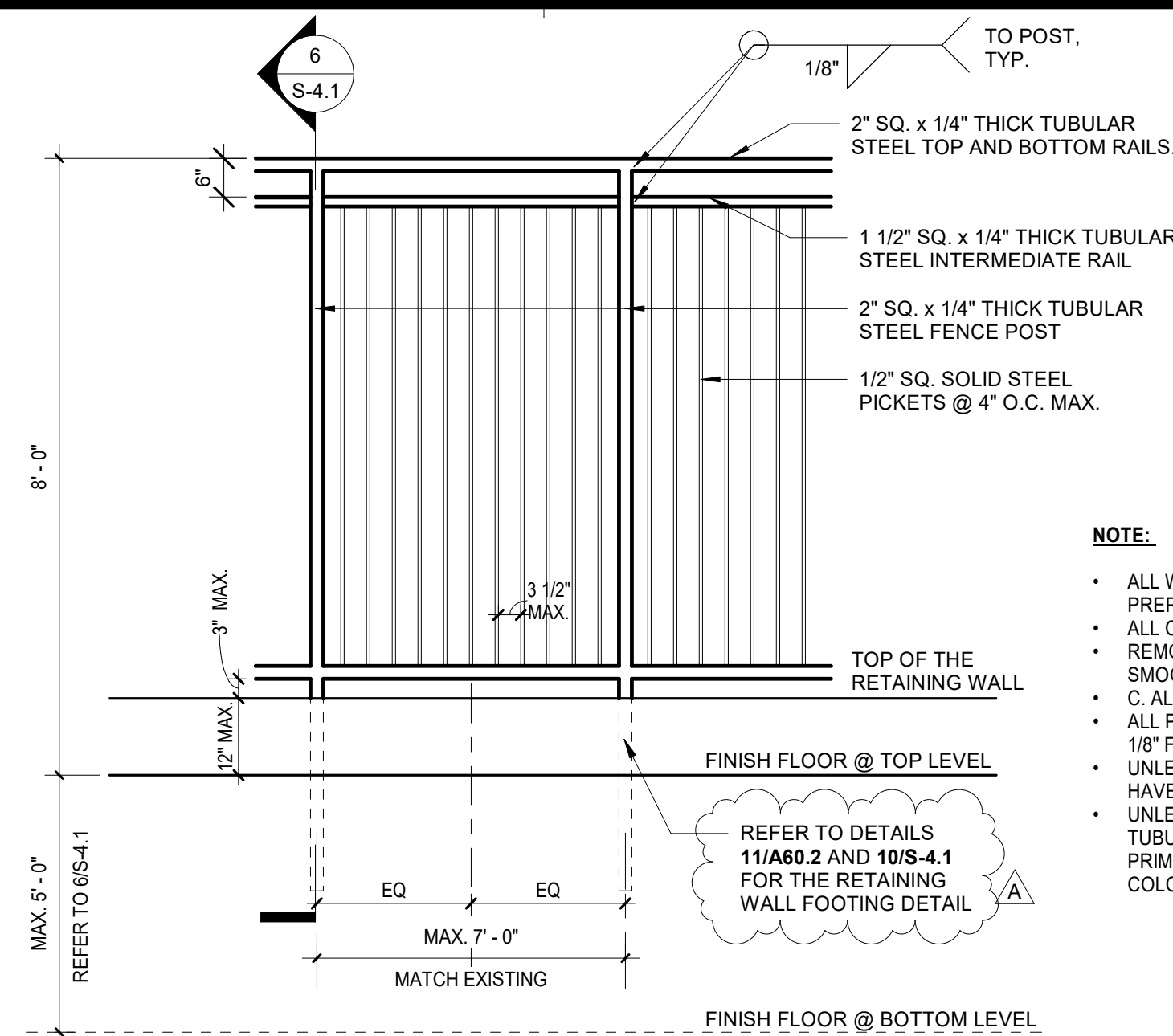
A52.1

SCHEDULES

<u>ABBREVIATIONS</u>	<u>DESCRIPTION</u>
AC	ALUMINUM CLADDING
CBU	CEMENTITIOUS BACKER UNIT
CC	COLOR COATED FINISH
CMU	CONCRETE MASONRY UNIT
CONC	CONCRETE
CP	CEMENT PLASTER
CS-1	CONCRETE SEALER - COLORLESS
CTB-1	CERAMIC TILE - 6X6 SELF-COVE BAS
CT-1	CERAMIC TILE - 2 x 2 WALL
CT-2	CERAMIC TILE - 6 x 6 WALL
EXP	EPOXY
EXP	EXPOSED STRUCTURE
EP-1	EPOXY BASE W/ IN 3/8" RADIUS
FF	FACTORY FINISH
FRP	FIBER REINFORCED POLYMER
GBCM	CLASS MAT FACED GYPSUM BOARD
PF-1	PAINT - GLOSS
PF-2	PAINT - SEMI-GLOSS
PF-3	PAINT - FLAT
SC	SKIM COAT FINISH

PT-1
PT-2
PT-3
PT-4

FINISH LEGEND



DOOR SIGNAGE

50" MIN. AFF.
60" MAX. AFF.
1'-0"

PLAN

DOOR & WALL-MOUNTED SIGNAGE, SEE DETAIL

34" MIN. CLR.
SIDE DOOR
MIN. CLR.
1'-6"
STRIKE SIDE CLEARANCE

5'-0" MIN. CLR.
32" MIN. CLR.
END DOOR
18" MIN.
4" MAX.
58" MIN. (INWARD SWING DOOR)
WALL MOUNTED TOILET
48" MIN. CLR.
@ END DOOR
60" MIN. CLR.
@ SIDE DOOR

FACE OF WALL
17" MIN. - 18" MAX.
FOF
GRAB BAR, SEE ELEVATION
RECESSED TOILET ACCESSORIES, SEE PLAN & ELEVATIONS

FRONT ELEVATION

3'-0" MIN.
GRAB BAR
1'-0" MIN.
2'-0" MIN.
17'-18" FOF
17'-18" T.O.S.
1'-1/2" MIN.

SIDE ELEVATION

54" MIN.
1'-0" MAX.
3'-6" MIN.
GRAB BAR
24" MIN.
17'-18" T.O.S.
36" T.O. RAIL

URINAL

32" MIN. CLR.
EQ.
EQ.
36" MIN. CLEAR WHERE WALLS OR PROJECTIONS EXTEND MORE THAN 24" FROM FRONT OF CLEAR SPACE
1'-0" MIN. TYP.
2'-0" MIN.
13-1/2" MIN. PROJECTION FROM WALL (IBC 11B-905.2)
ACCESSIBLE
STD.

ELEVATION

FACE OF WALL
1'-6" MIN. TO SIDEWALL OR FIXTURE CLEAR SPACE
4" MAX. PROJECTION OF ACCESSORIES
27" MIN.
20" MIN.
@ APRON
34" MIN.
40" MAX.
BACK SPLASH
ELEVATION

LAVATORY NOTES:

1. THERE SHALL BE NO SHARP OR ABRASIVE SURFACES UNDER THE LAVATORY.
2. ALL PIPING, WATER & DRAIN PIPES ACCESSIBLE UNDER LAVATORIES MUST BE INSULATED OR OTHERWISE COVERED.
3. CLEARANCE HEIGHTS PER AGE GROUP CHART AND DETAIL 2L-2.

4'-0"

TOP OF COAT HOOK

FINISH FLOOR

COAT HOOK

3'-4" A.F.F. MAX.
TO TOP OF OPERABLE PART TYP.

SANITARY NAPKIN DISPENSER

SEAT COVER DISPENSER

SOAP DISPENSER

MIRROR

35" MAX. OVERSEASIDE FROM REFLECTIVE SURFACE

HAND DRYER

PAPER TOWEL DISPENSER

ELECTRIC HAND DRYER REFER TO ELECTRICAL DWGS. FOR POWER

NOTES:

1. REFER TO SPECIFICATIONS AND TOILET ACCESSORY SCHEDULE FOR MORE INFORMATION.
2. FOR TOILET PARTITIONS REFER TO SPECIFICATIONS AND DETAIL.
3. REFER TO PLUMBING DRAWINGS FOR MORE INFORMATION ON SPECIFIC FIXTURES.
4. ALL ACCESSORIES SHALL HAVE A MAXIMUM PROJECTION OF 4 INCHES FROM THE FINISHED WALL SURFACE.

(INDICATES REQUIRED CLEARANCES OF FIXTURES &
ACCESSORIES TO FINISHED SURFACE, PER AGE GROUP CHART)

Diagram illustrating the installation of a toilet tissue dispenser on a wall. The dispenser is mounted 19 inches from the floor and 7 to 9 inches from the front edge of the toilet seat. The dispenser is 2.5 inches high and 12 inches wide. The diagram also shows the location of the grab bars and the toilet seat.

Diagram illustrating the placement of a toilet tissue dispenser in a toilet stall. The dispenser is mounted on the wall above the toilet seat. The diagram shows the dispenser's location relative to the floor and the toilet seat. Key dimensions and labels include:

- TOILET TISSUE DISPENSER** (located on the wall)
- GRAB BAR FOR MOUNTING SEE DET.**
- FACE OF WALL**
- TOILET SEAT COVER DISPENSER**
- MIN. 10" CLEAR** (clearance from the toilet seat to the dispenser)
- PER AGE GROUP** (indicated for both the toilet and the dispenser)
- 40" MAX. TO TOP OF GRINDING** (dimension from the floor to the top of the dispenser)
- FIN. FLOOR**
- 7'-0"** (dimension from the front edge of the toilet seat to the dispenser)
- TOILET TISSUE DISPENSER LOCATED WITHIN 7'-0" OF THE FRONT EDGE OF THE TOILET SEAT**

$$3/8'' = 1'-0''$$

3

[illegible]

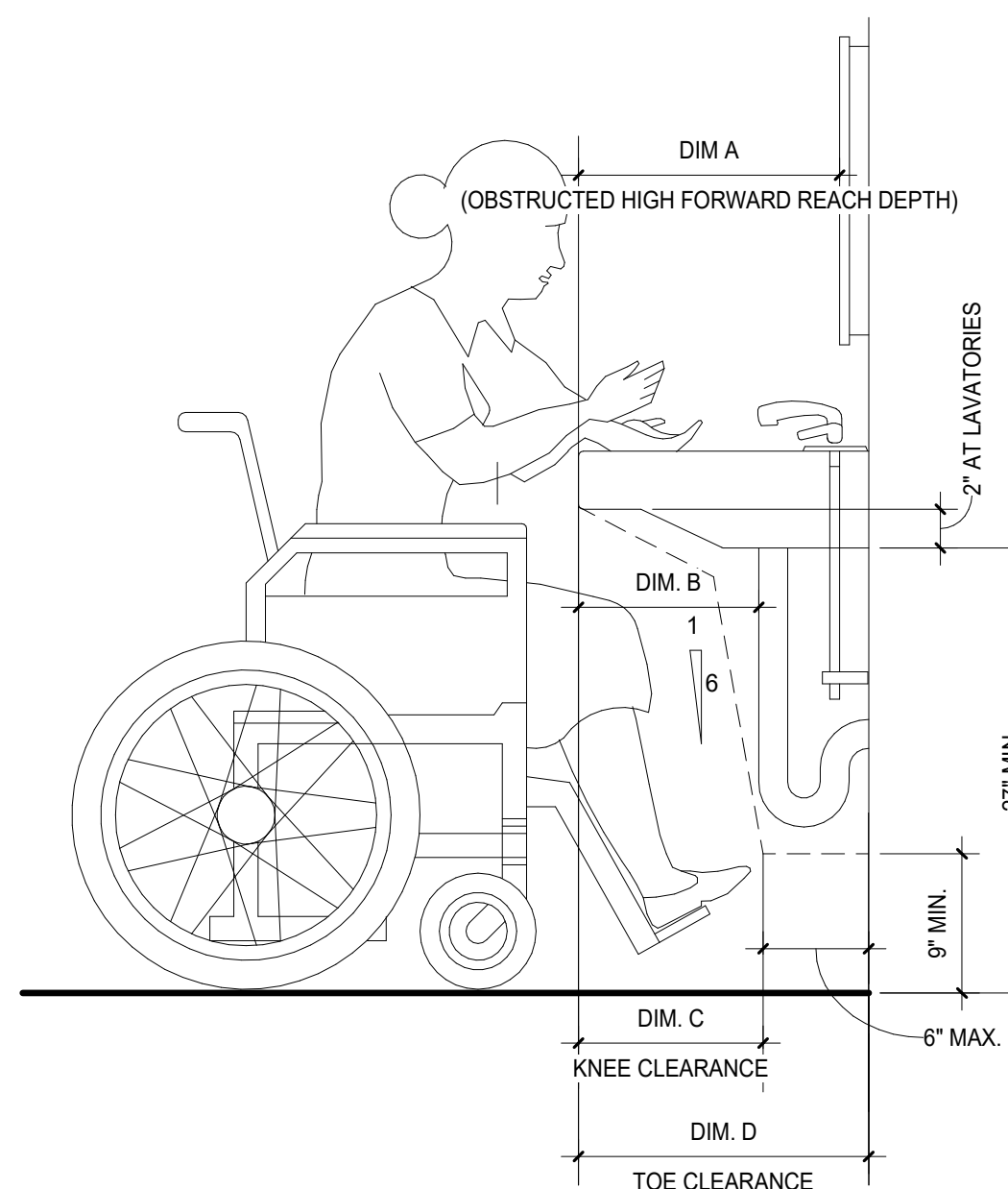
SCALE: As indicated

ACCESSIBILITY DETAILS

NOTES:
1. REFER TO FLOOR PLANS FOR FIXTURE LOCATIONS.
2. REFERENCE ELECTRICAL AND PLUMBING FOR ADDITIONAL INFORMATION

$$1/2^\circ = 1'-0''$$

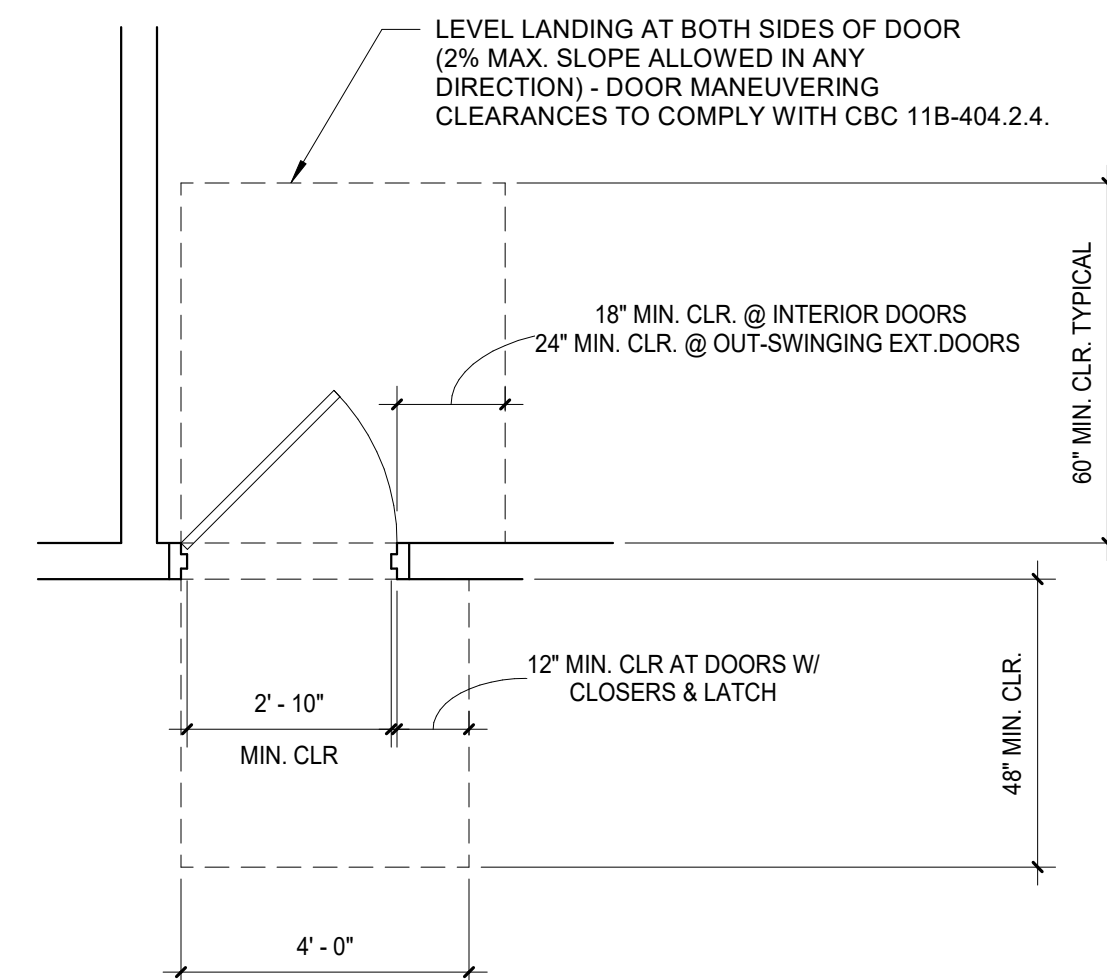
13



	DIM. A	DIM. B	DIM. C	DIM. D
LAV.	17"	8"	11"	17"
	18"	9"	12"	18"
SINK	19"	10"	13"	19"
	20"	11"	14"	20"
	21"	12"	15"	21"
	22"	13"	16"	22"
	23"	14"	17"	23"
	24"	15"	18"	24"
	25"	16"	19"	25"

$$1'' = 1' - 0''$$

2



@ METAL STUD. PROVIDE (3) #14 x 2-1/2" —
LONG STAINLESS STEEL SHEET MTL.
SCREWS AT EA. GRAB BAR MOUNTING
FLANGE TO WALL BACKING PLATE
@ CMU WALL PROVIDE (3) 3/8" DIA. HILTI KWI
BOLT T22 SS304 STAINLESS STEEL MASONRY
ANCHORS 2 1/2" MIN. EMBED (ICC ESR-4561)

WALL AND FINISH PER FLOOR PLAN ———

NOTES:
1. REFER TO GRAB BAR MANUFACTURER'S INSTRUCTIONS FOR INSTALLATION AT TOILET PARTITIONS.
2. GRAB BAR AND ITS INSTALLATION SHALL RESIST A 250 LB POINT VERTICAL LOAD.

$$3/8" = 1'-0"$$

5

5

1'-0"	1
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1



REDONDO BEACH UNIFIED
SCHOOL DISTRICT

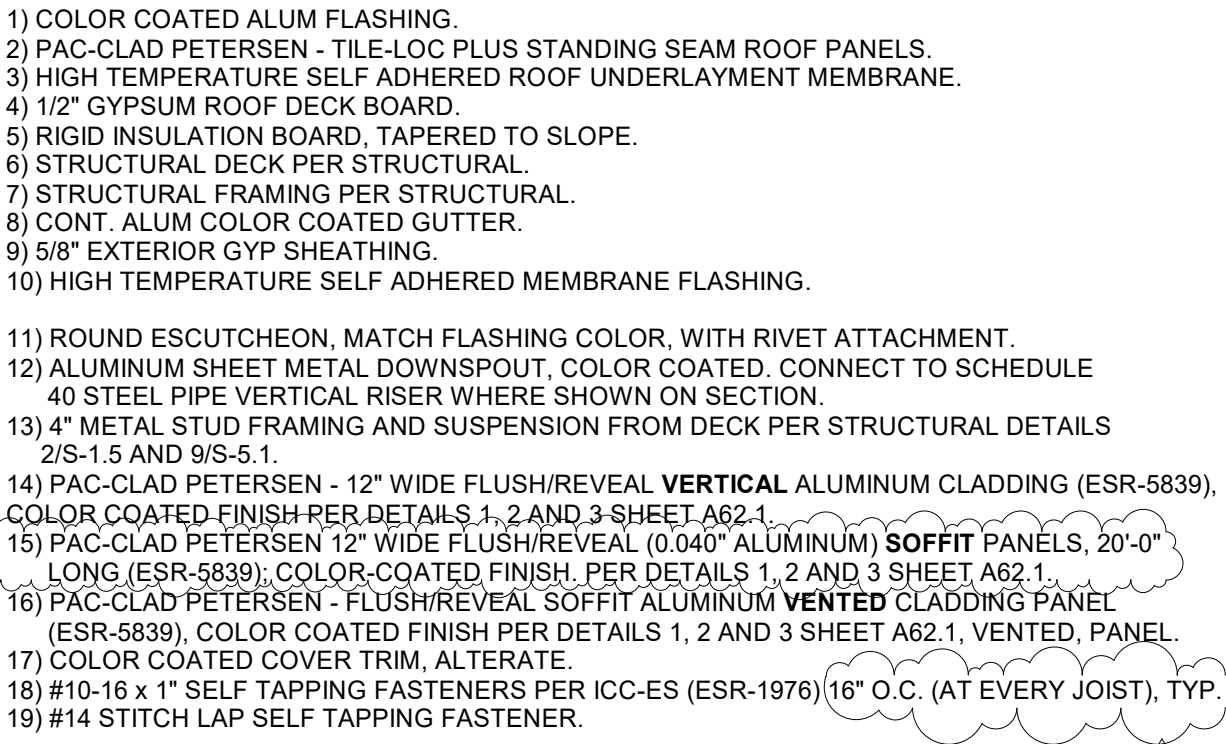
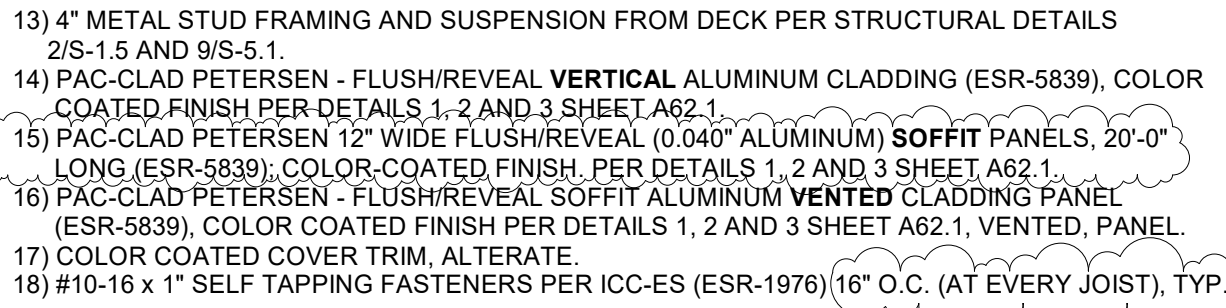
REDONDO UNION HIGH SCHOOL
SEA HAWK BOWL IMPROVEMENTS

1 SEA HAWK WAY, REDONDO BEACH, CA 90277

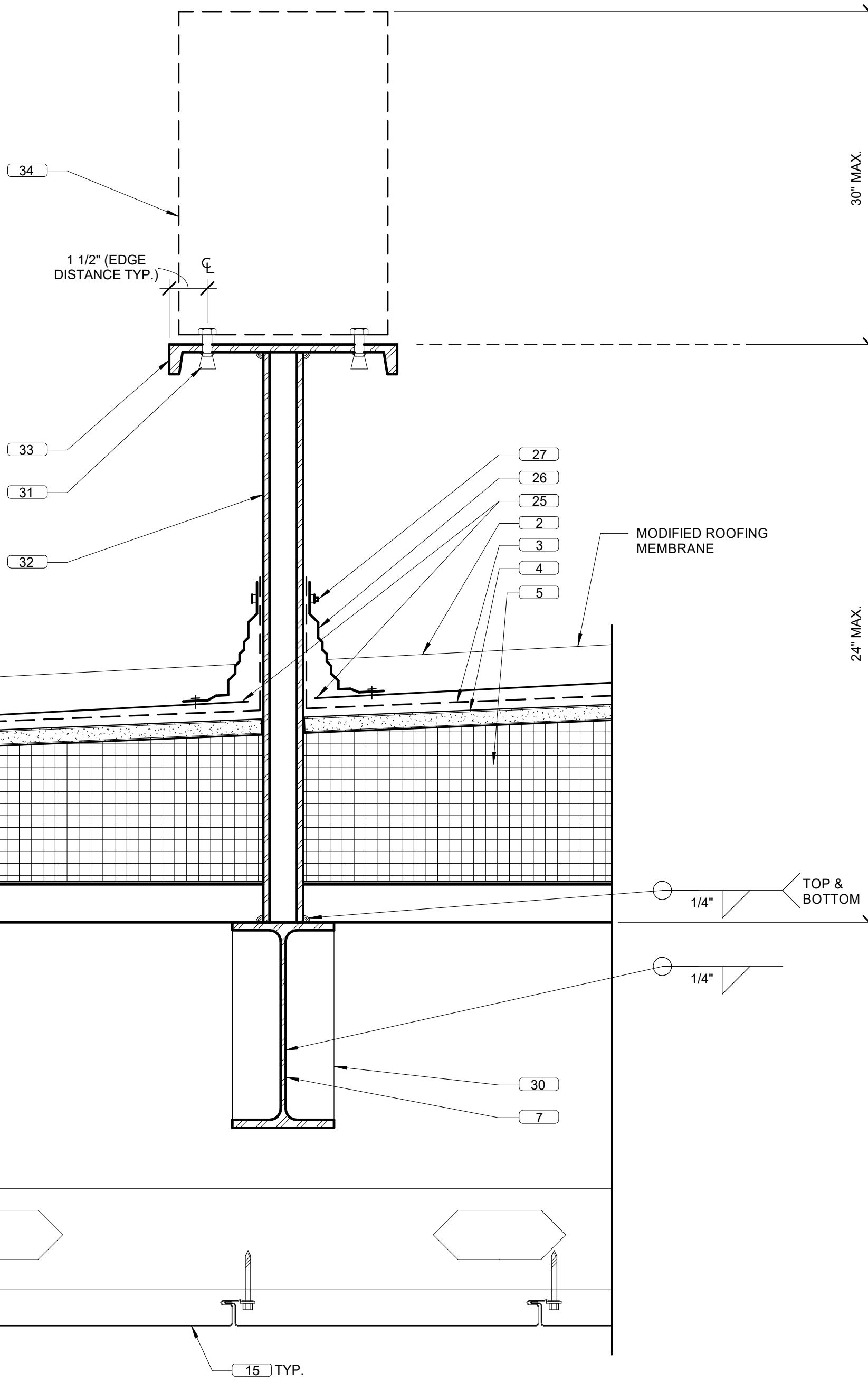
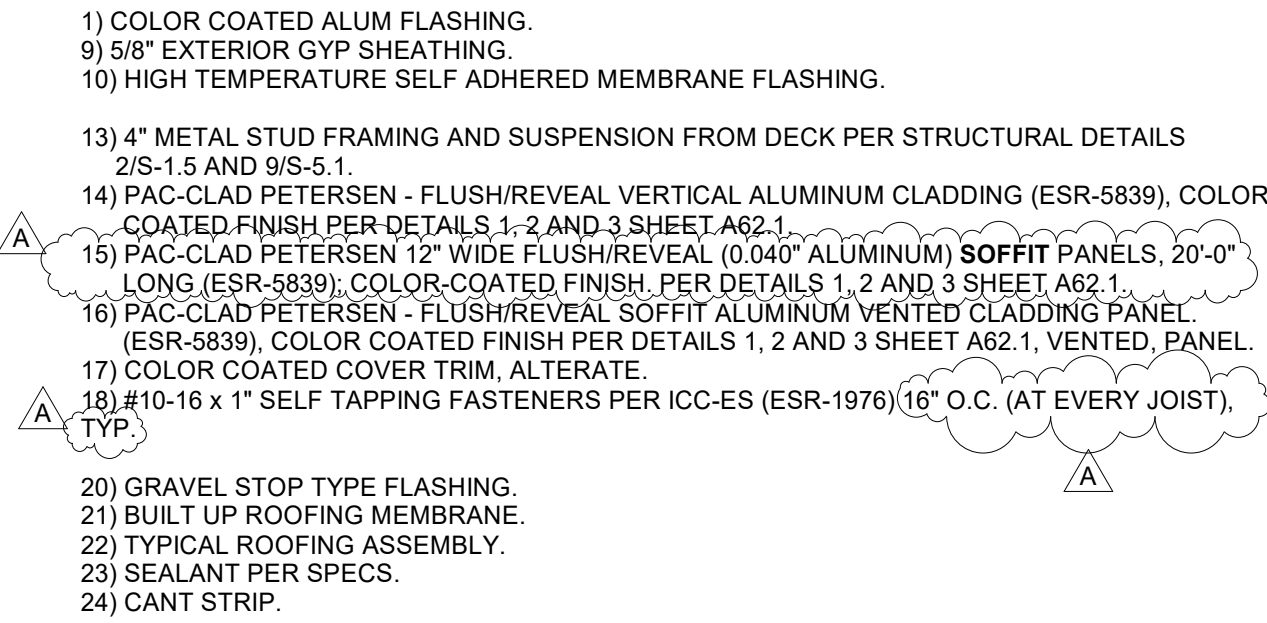


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$$3^{\circ} = 1^{\circ} - 0^{\circ}$$


3" = 1'-0" 3


$$3'' = 1'-0''$$


3" = 1'-0" | **1**

1 SEA HAWK WAY, REDONDO BEACH, CA 90277

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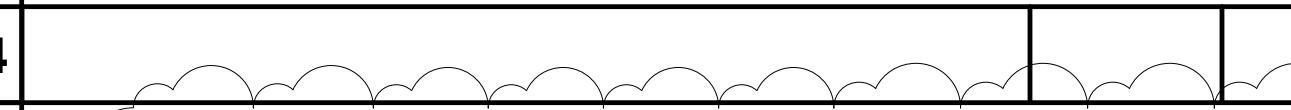
SCALE: 3" = 1'-0"

ROOF ASSEMBLY DETAILS

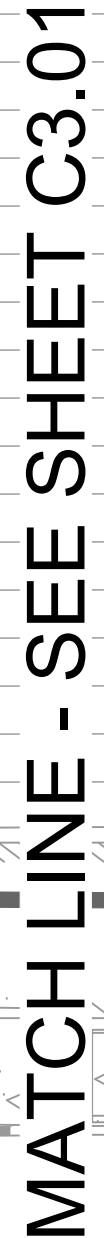


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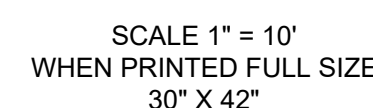
	CIVIL LIMIT OF WORK LINE
	ACCESSIBLE ROUTE (LOCATION PURPOSES ONLY, DO NOT PAINT)
	LIMITS OF SAWCUT
	PROPOSED OVERHANG
	PROPERTY LINE
	CENTER LINE
	PROPOSED LANDSCAPE PER LANDSCAPE PLANS
	PROPOSED HEAVY DUTY CONCRETE PAVEMENT
	PROPOSED BUILDING
	PROPOSED HEAVY DUTY ASPHALT PAVEMENT

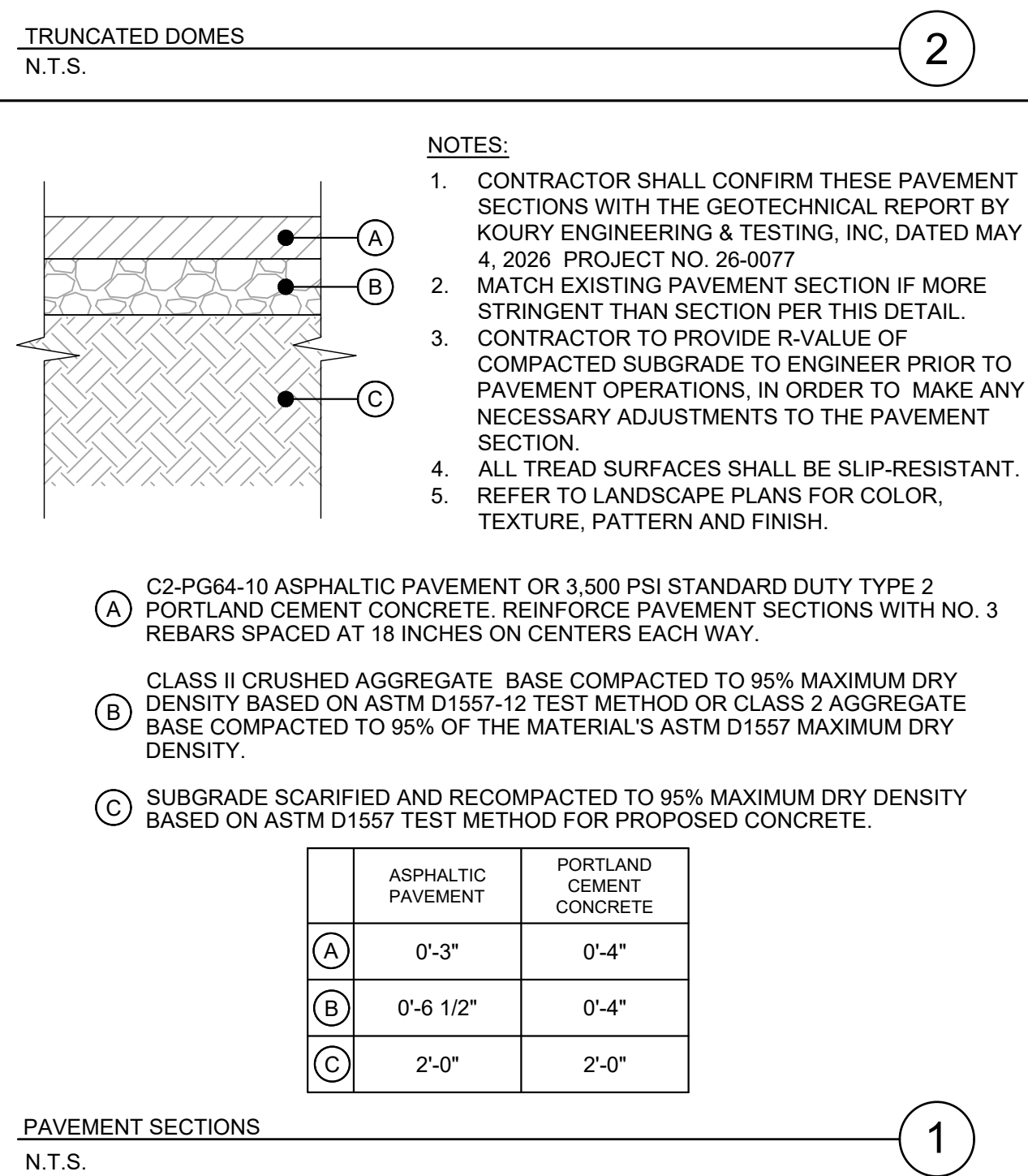
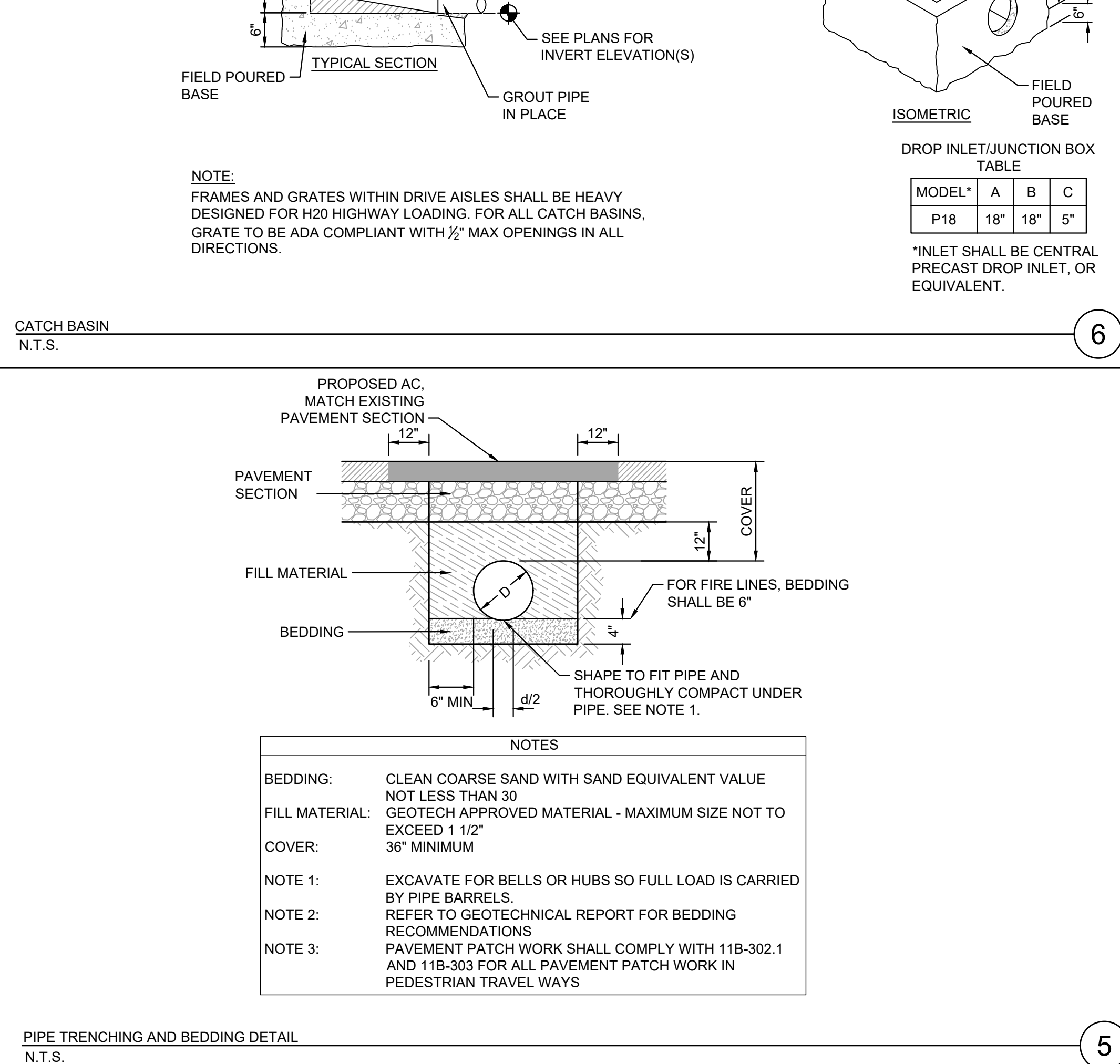
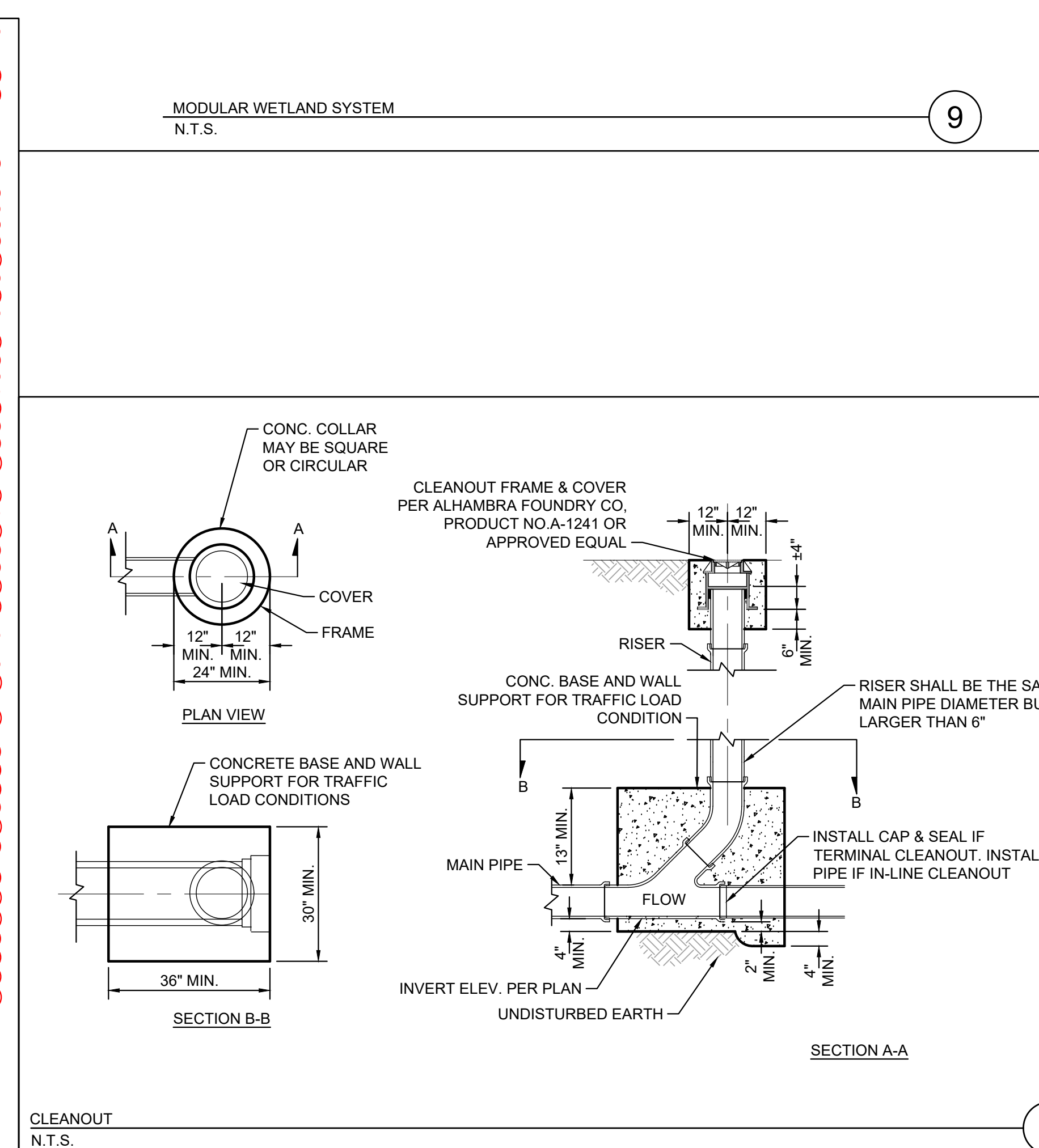
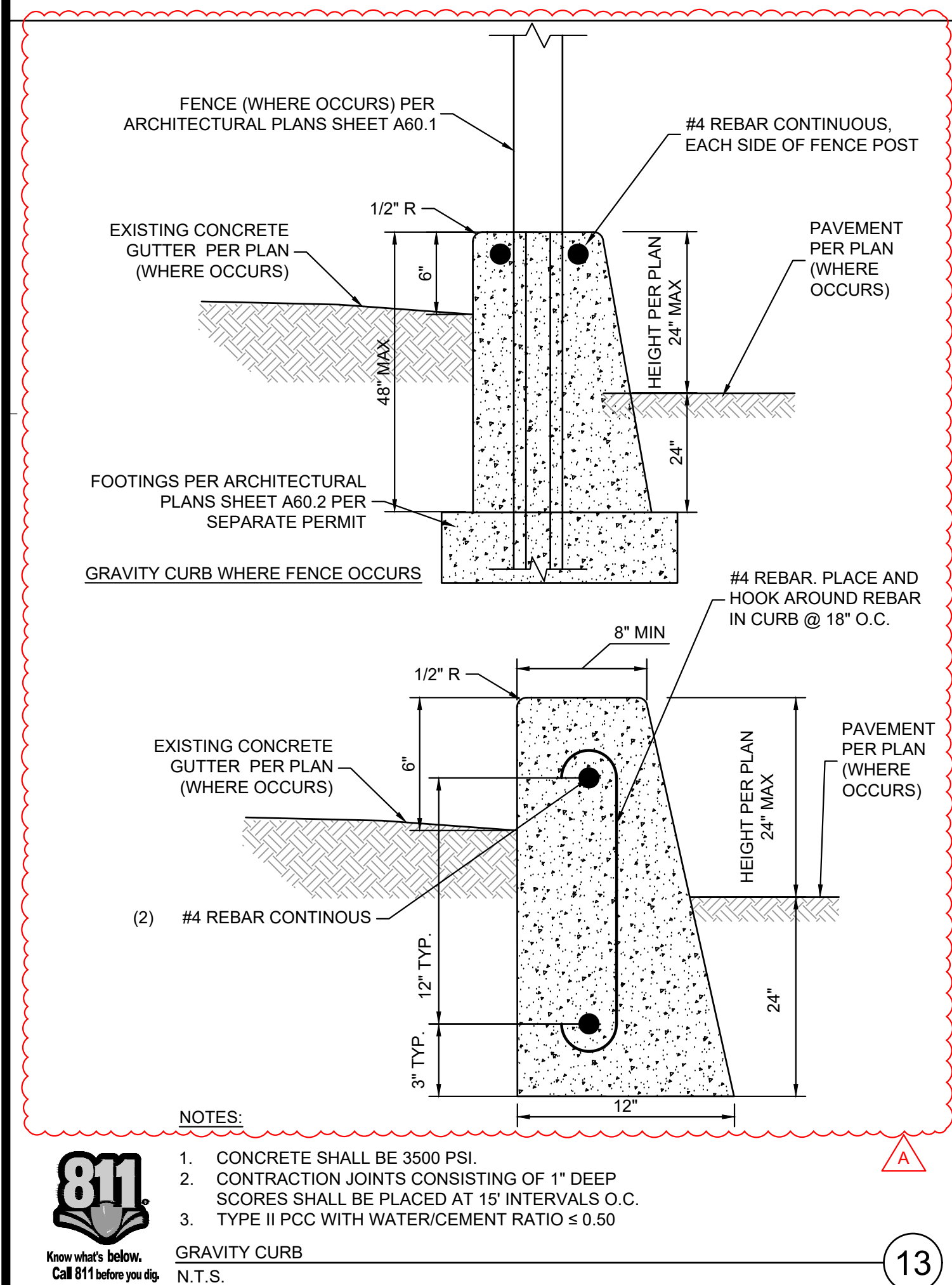
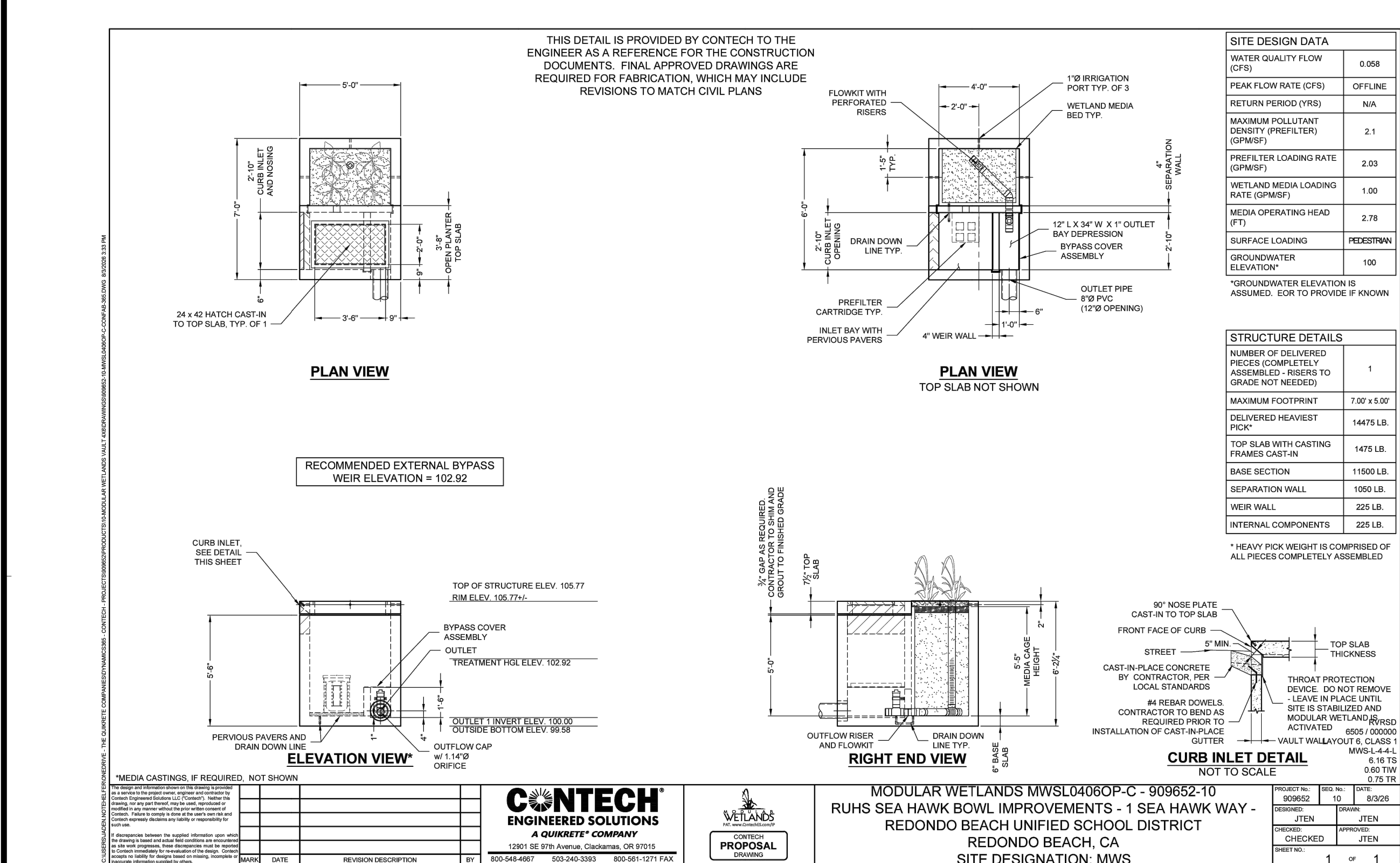
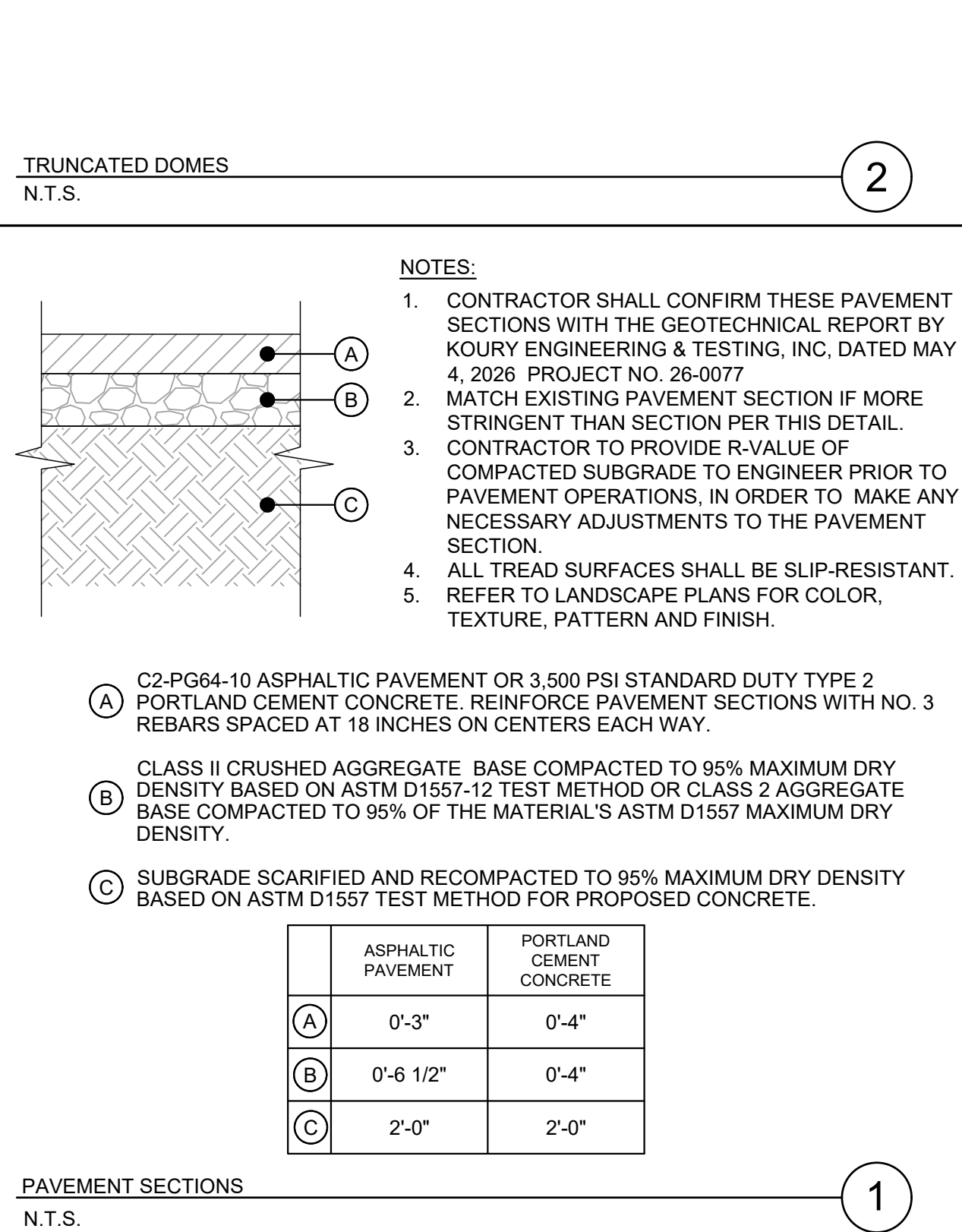
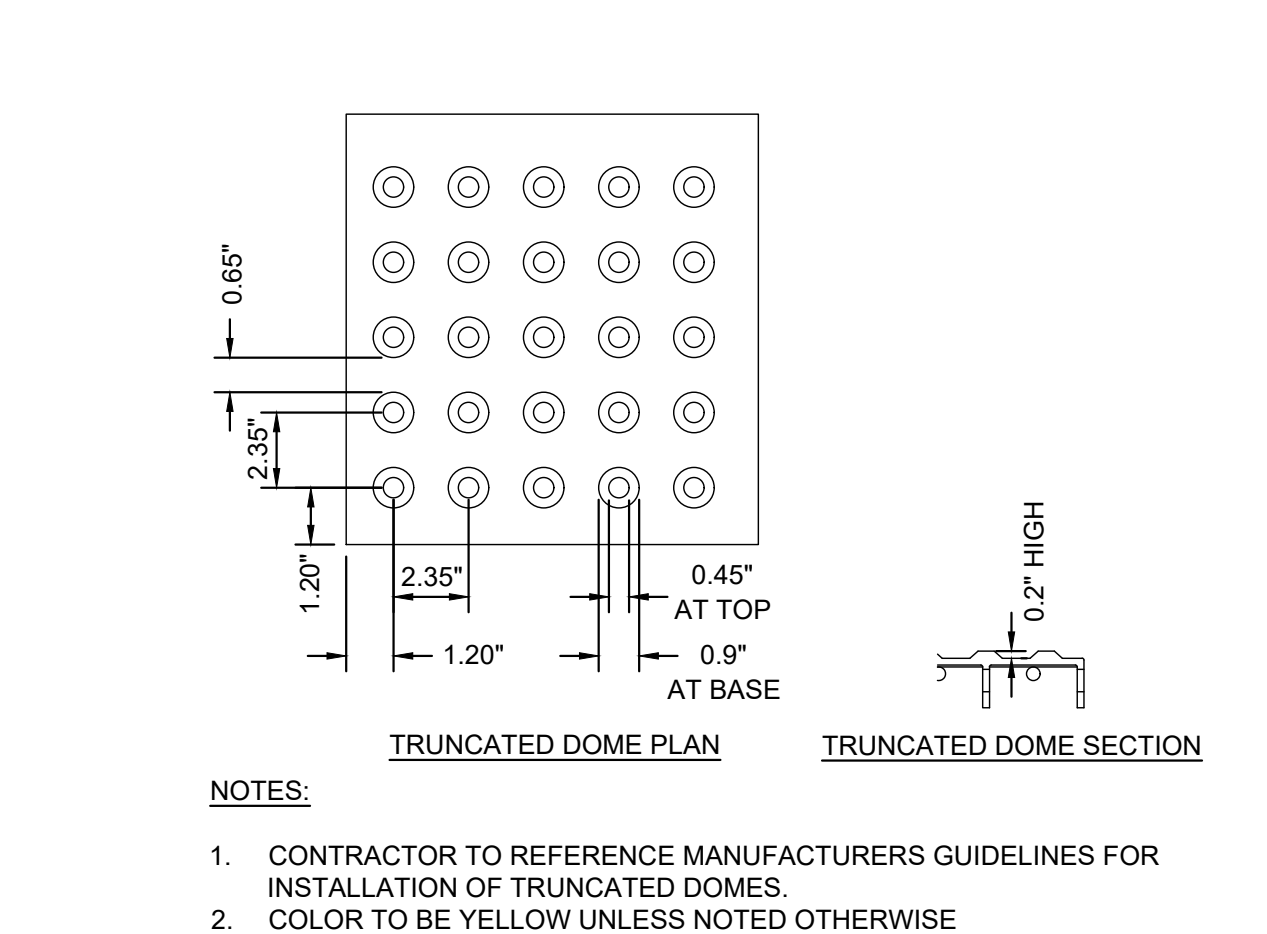
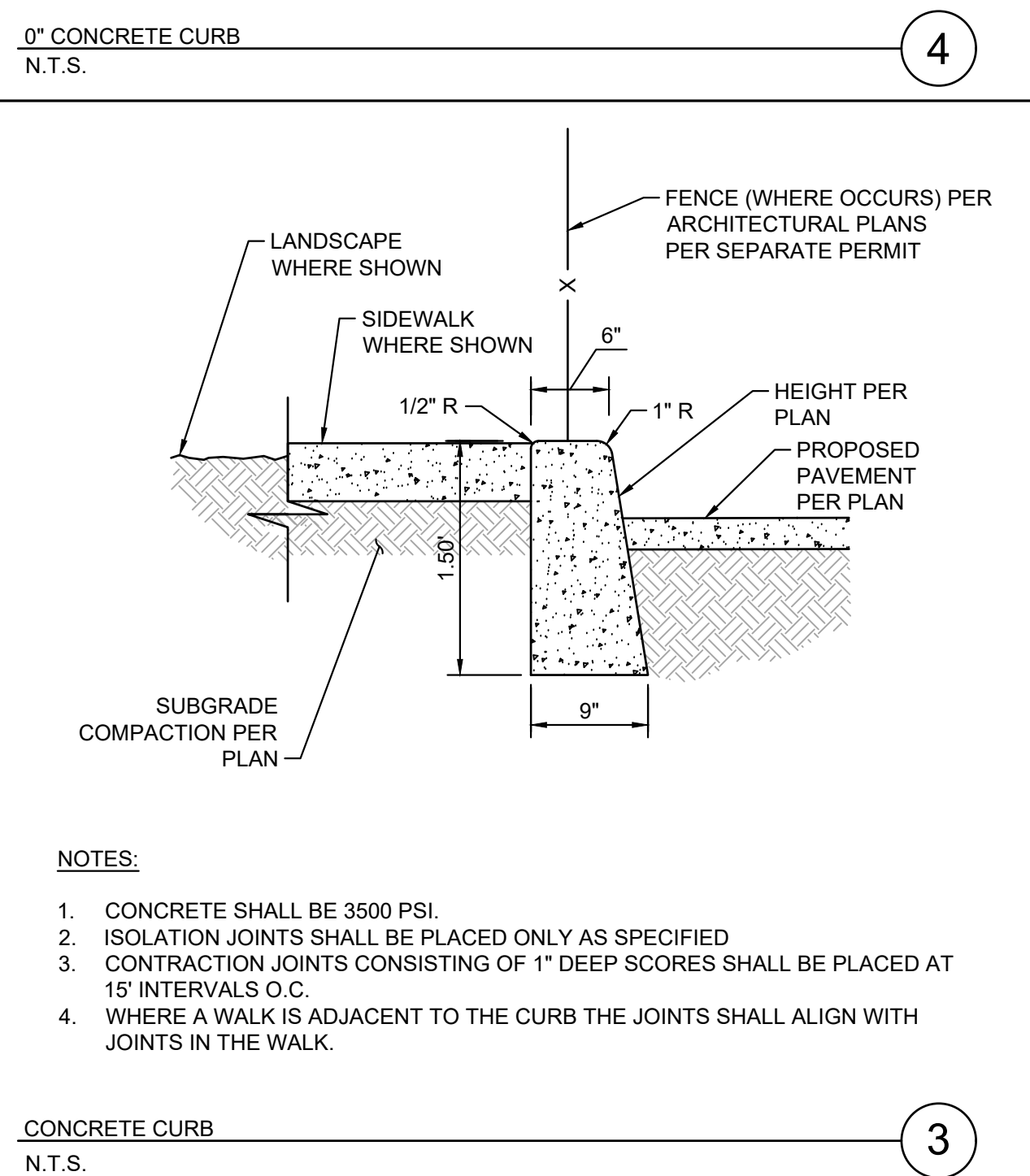
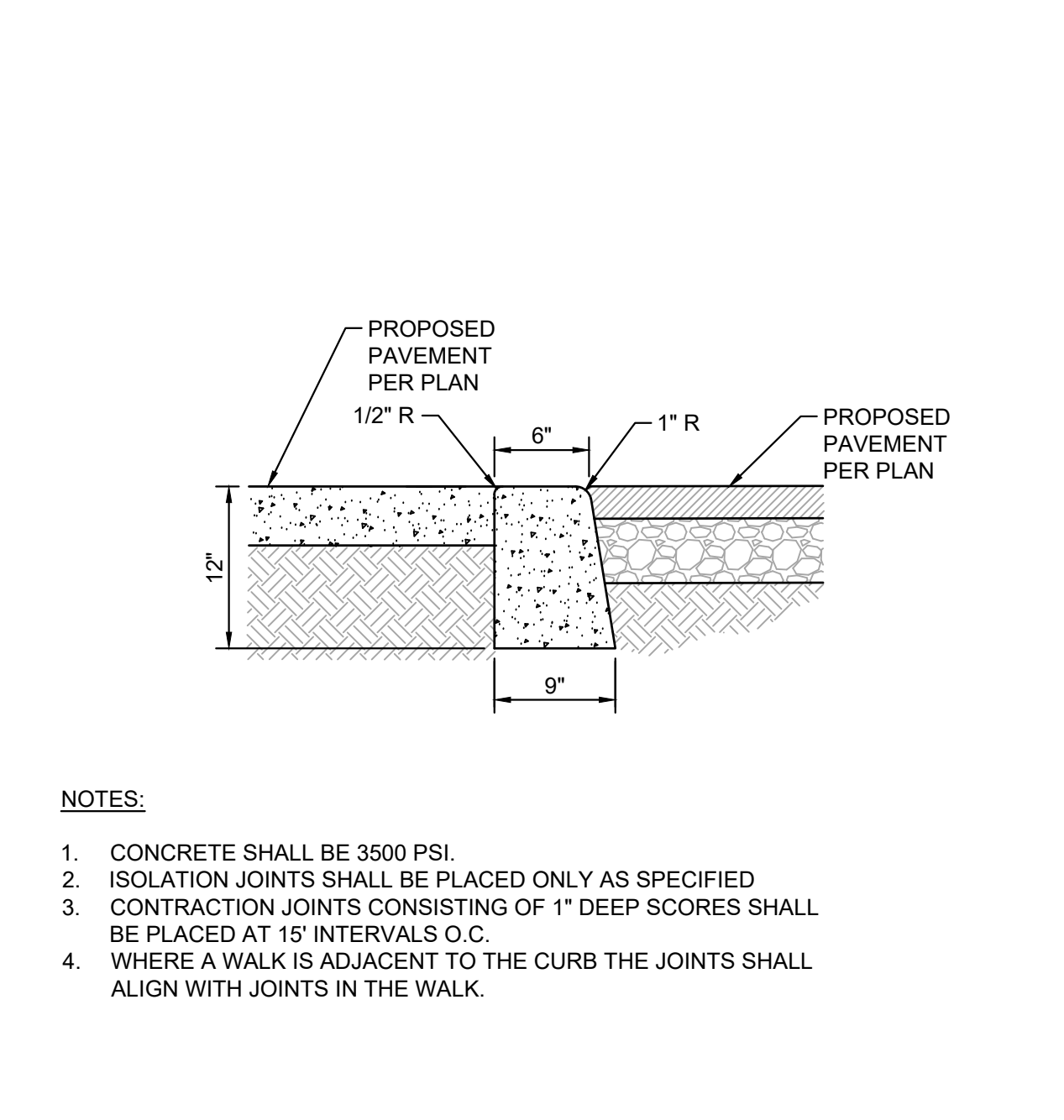
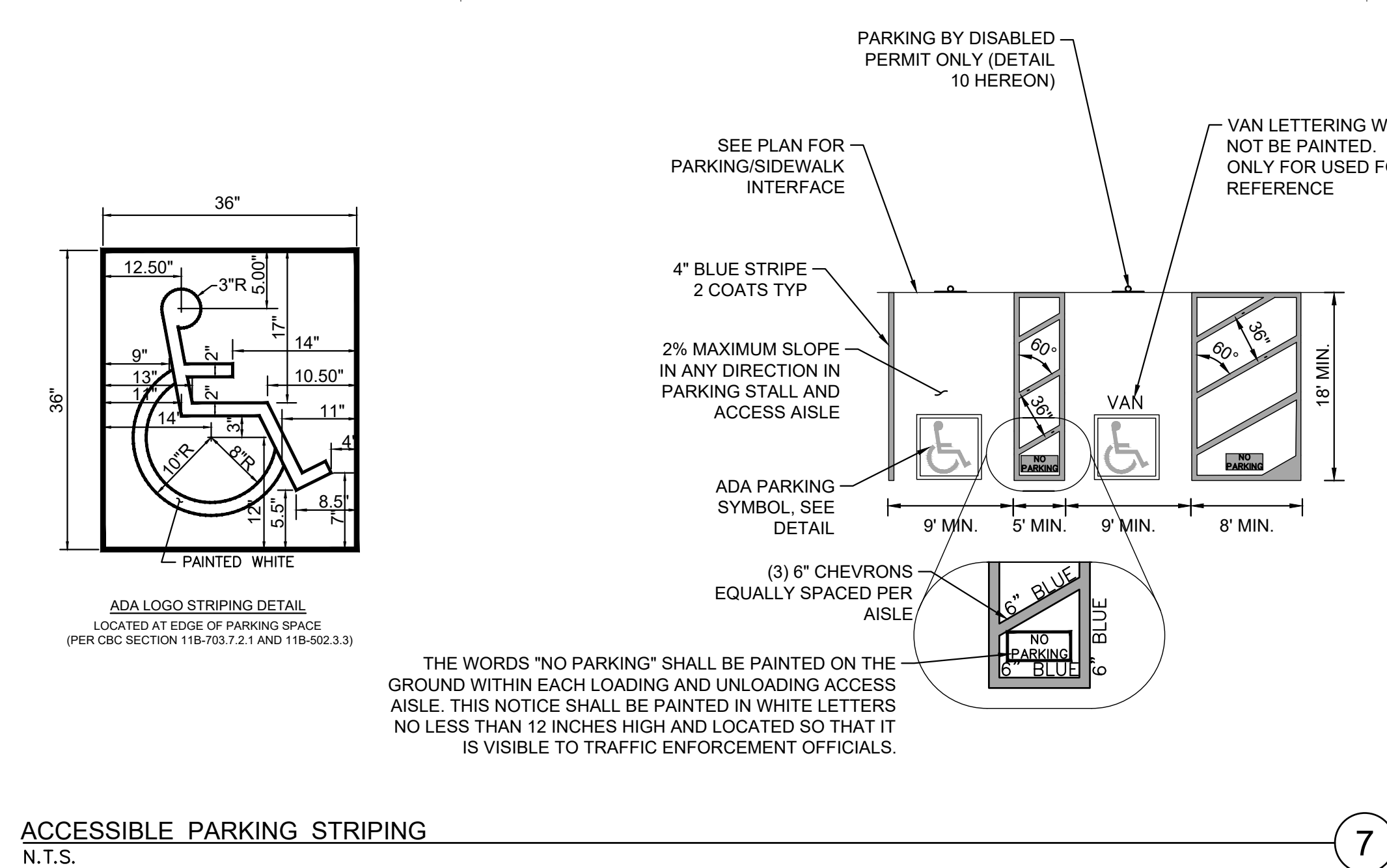
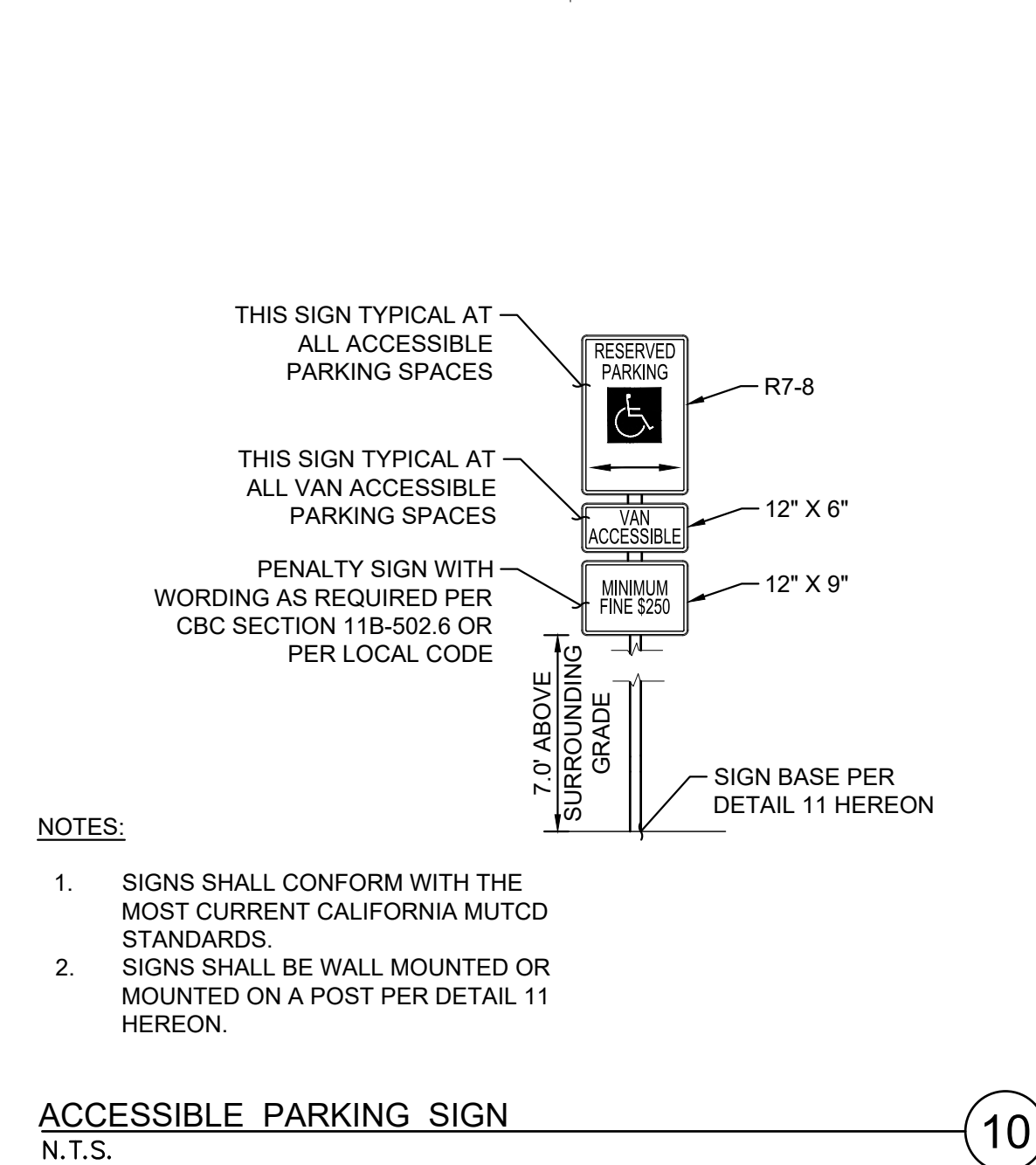
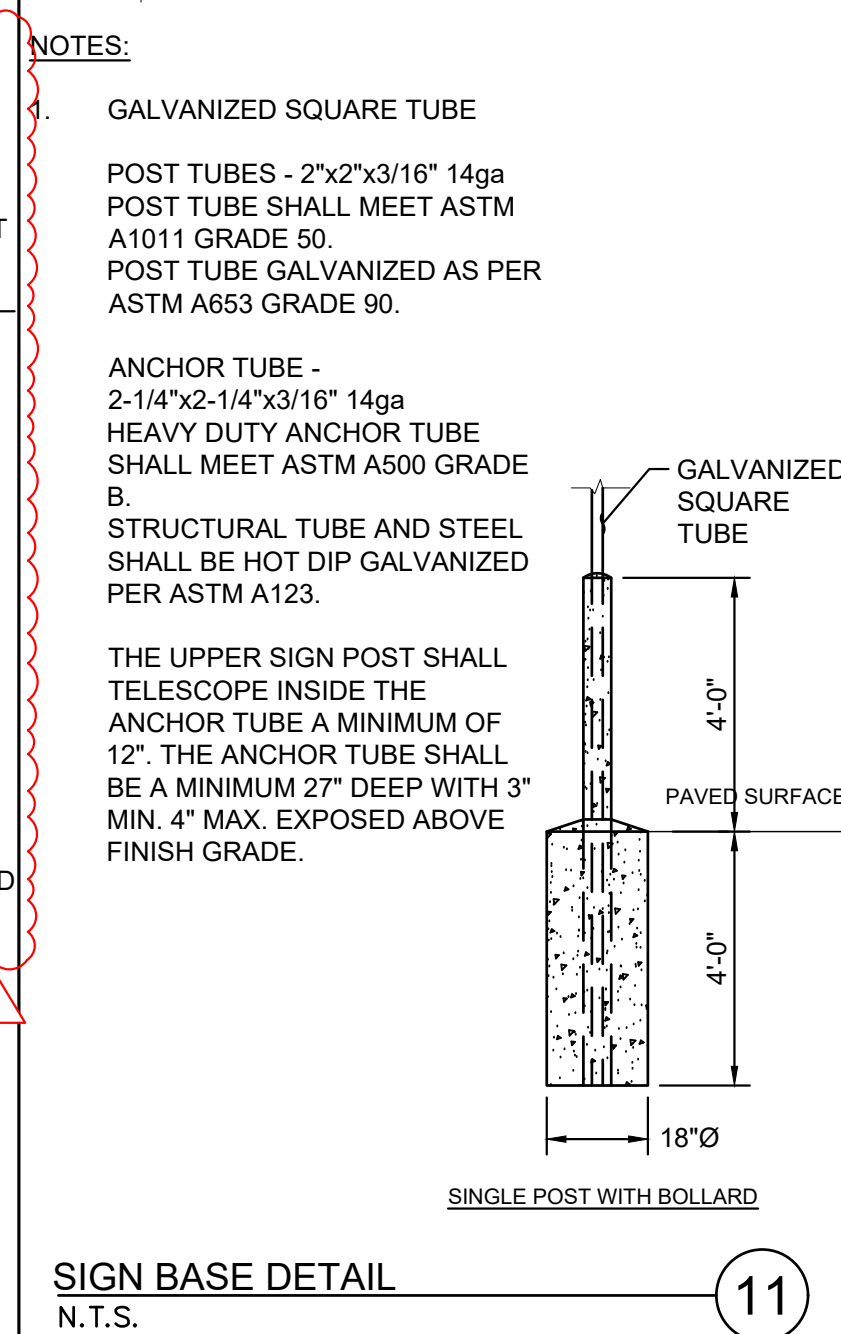
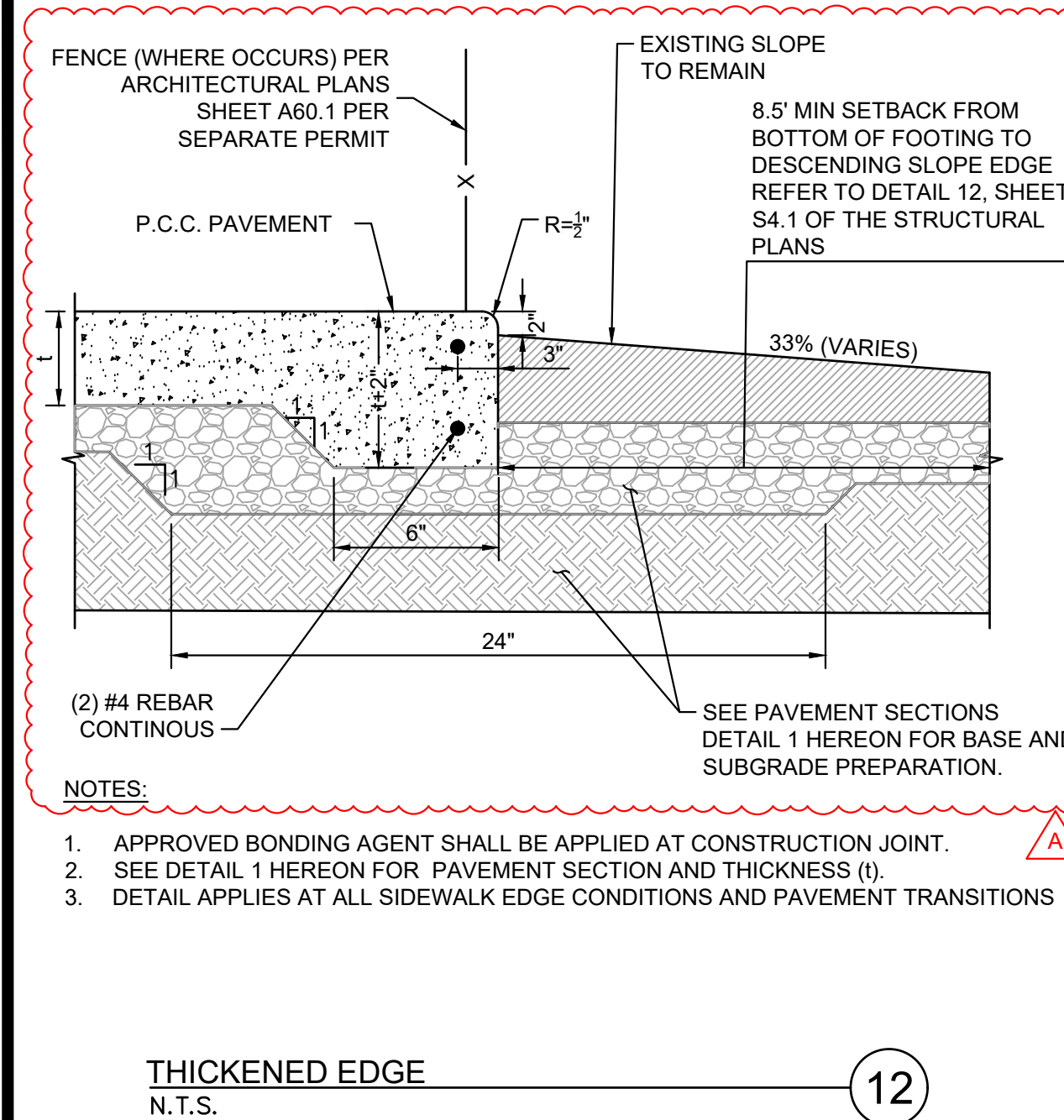
- 1 PROPOSED BUILDING PER ARCHITECTURAL PLAN
- 2 PROPOSED FENCE PER ARCHITECTURAL PLAN
- 3 PROPOSED GATE PER ARCHITECTURAL PLAN
- 4 PROPOSED STANDARD DUTY CONCRETE PAVEMENT PER DETAIL 1, SHEET C6.00
- 5 PROPOSED 6" CONCRETE CURB PER DETAIL 3, SHEET C6.00
- 6 PROPOSED 0" CONCRETE CURB PER DETAIL 4, SHEET C6.00
- 7 PROPOSED SAWCUT LINE
- 8 PROPOSED TRUNCATED DOMES PER DETAIL 2, SHEET C6.00
- 9 PROPOSED RETAINING WALL PER STRUCTURAL PLANS
- 10 PROPOSED COLUMN PER ARCHITECTURAL AND STRUCTURAL PLANS
- 11 PROPOSED OVERHANG PER ARCHITECTURAL PLAN
- 12 PROPOSED GRAVITY CURB PER DETAIL 13, SHEET C6.00
- 13 PROPOSED ACCESSIBLE PARKING STALL, DIMENSIONS PER PLAN
- 14 PROPOSED 45° STRIPING, SPACED AT 36" O.C. PER DETAIL 7, SHEET C6.00
- 15 PROPOSED STANDARD PARKING STALL, DIMENSIONS PER PLAN
- 16 PROPOSED WHEEL STOP PER ARCHITECTURAL PLANS
- 17 PROPOSED ACCESSIBLE PARKING SIGN POST PER ARCHITECTURAL PLANS
- 18 PROPOSED THICKENED EDGE PER DETAIL 12, SHEET C6.00
- 19 PROPOSED DRINKING FOUNTAIN PER ARCHITECTURAL PLAN

PROJECT NO: 2025-40137
ISSUANCE: A#03-126194
DATE ISSUED: 06/12/2026
SCALE:

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1. DETAILS OF CONSTRUCTION NOT SHOWN SHALL BE OF SAME NATURE AS THOSE SHOWN FOR SIMILAR CONDITIONS. REFER TO THE TYPICAL DETAIL SHEETS FOR TYPICAL DETAILS OF CONSTRUCTION. TYPICAL DETAILS APPLY TO ALL CONSTRUCTION UNLESS SPECIFICALLY NOTED OR SHOWN OTHERWISE. WHERE CONDITIONS REQUIRE MODIFICATIONS OF A TYPICAL DETAIL, THE CONTRACTOR SHALL SUBMIT MODIFIED DETAIL FOR APPROVAL BY THE ENGINEER OF RECORD PRIOR TO FABRICATION AND INSTALLATION.

- ### STRUCTURAL DESIGN CRITERIA:

-
- ROOF ZONES
(PLAN VIEW)**

1. FIELD VERIFY ALL CONDITIONS & DIMENSIONS PRIOR TO SHOP DRAWING PRODUCTION AND FABRICATION OF STRUCTURAL ELEMENTS.
2. WHERE ALL OTHER EXISTING CONDITIONS VARY SIGNIFICANTLY FROM THE SHOWN ON THESE DRAWINGS, THE STRUCTURAL ENGINEER SHALL BE NOTIFIED PRIOR TO ANY CONSTRUCTION RELATED TO SUBJECT CONDITIONS.
3. SHORE ALL EXISTING CONSTRUCTION AS REQUIRED.
4. ALL EXISTING (E) CONNECTIONS AT ELEMENTS TO BE REPLACED SHALL BE REPLACED OR RE-ATTACHED TO MATCH EXISTING CONDITIONS.
5. USING NON-DESTRUCTIVE MEANS, VERIFY LOCATION OF EXISTING REBAR IN CONCRETE/MASONRY WALLS, RAISED/ELEVATED CONCRETE SLABS/BEAMS/FOUNDATIONS PRIOR TO ANY DEMOLITION / SAWCUTTING / FABRICATION. S.O. ON GRADE AND CONCRETE FILLED METAL DECKS ARE EXEMPT UNLESS NOT OTHERWISE.
6. SPECIAL INSPECTION IS REQUIRED FOR ALL WORK.
7. SEE "AS BUILT" DRAWINGS FOR EXISTING BUILDING DESIGN FOR ITEMS NOT SHOWN OR NOTED.
8. ALL EXISTING (E) WOOD ELEMENTS TO REMAIN SHALL BE FIELD INSPECTED DURING CONSTRUCTION AND TREATED FOR DRYROT REMOVAL / CONTROL WHERE EXISTING GLBS TO REMAIN ARE FOUND TO HAVE EXTENSIVE DRYROT DEEPER THAN THE TOP TWO LAMINATIONS (3"). THE STRUCTURAL ENGINEER SHALL BE NOTIFIED PRIOR TO CONTINUED CONSTRUCTION RELATED TO SUBJECT GLBS.
9. CORE DRILLS REQUIRED SHALL NOT CUT ANY REINFORCING. THE CONTRACTOR IS TO COORDINATE WORK OF ALL TRADES TO ENSURE QUALITY. ALL CORE DRILLS ARE TO BE PRESENTED TO THE IOR FOR VERIFICATION. THE IOR IS TO DOCUMENT CORES EXAMINED INDICATING AN ABSENCE OF REINFORCING.

1. THE ARCHITECT AND ENGINEERS ARE NOT RESPONSIBLE FOR THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES WHETHER OR NOT SHOWN ON THE DRAWINGS. THE LOCATION OF ANY EXISTING UNDERGROUND UTILITIES SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE OWNER SHOULD ANY SUCH UNIDENTIFIED CONDITIONS BE DISCOVERED.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGES WHICH MAY RESULT FROM HIS FAILURE TO EXACTLY LOCATE AND PRESERVE ALL EXISTING UNDERGROUND UTILITIES.

1. SEE SOIL REPORT BY: KOURY ENGINEERING & TESTING, INC.
REPORT NO.: 26-0077 DATED: MAY 4, 2026

2. FOUNDATION DESIGN PARAMETERS:

- a. BEARING PRESSURE, STATIC = 2,000 PSF
- b. BEARING PRESSURE, CYCLIC + SHORT TERM = $2,000 \times 1.33 = 2,600$ PSF
- c. MINIMUM FOOTING SIZE:
 - CONTINUOUS FOOTINGS: MINIMUM WIDTH = 18 INCHES, MINIMUM DEPTH BELOW LOWEST ADJACENT GRADE = 18 INCHES
 - PAD FOOTINGS: MINIMUM WIDTH = 24 INCHES (SQUARE), MINIMUM DEPTH BELOW LOWEST ADJACENT GRADE = 18 INCHES
- d. LATERAL CAPACITY:
 - ALLOWABLE PASSIVE PRESSURE, PASSIVE ONLY = 250 PSF/FT (2,500 PSF MAX).
 - WHEN INCLUDING SEISMIC USE $250 \times 1.33 = 325$ PSF/FT
 - COEFFICIENT OF FRICTION = 0.3
 - (WHEN COMBINED WITH FRICTION, ALLOWABLE PASSIVE PRESSURE TO BE REDUCED BY ONE THIRD)
- e. RETAINING WALL PRESSURES:
 - ACTIVE = 40 PSF/FT (DRAINED)
 - AT-REST = 80 PSF/FT (DRAINED)
 - SEISMIC = 144 LBS (LBS PER FT OF WALL LENGTH) APPLIED AT 0.6 H ABOVE TOE OF WALL
 - BEARING PRESSURE = 2,000 PSF MAXIMUM

13. PRIOR TO THE CONTRACTOR REQUESTING A BUILDING DEPARTMENT FOUNDATION INSPECTION, THE SOILS ENGINEER SHALL ADVISE THE BUILDING OFFICIAL IN WRITING THAT:
- A. THE BUILDING PAD WAS PREPARED IN ACCORDANCE WITH THE SOILS REPORT.
 - B. THE UTILITY TRENCHES HAVE BEEN PROPERLY BACKFILLED AND COMPACTED.
 - C. THE FOUNDATION EXCAVATIONS COMPLY WITH THE INTENT OF THE SOILS REPORT.

1. THE CONTRACTOR SHALL PROVIDE FOR THE DESIGN, APPROVALS, PERMITS, INSTALLATION AND MONITORING OF ALL TEMPORARY SHORING AND BRACING AS REQUIRED TO SUPPORT EXISTING FRAMING WHERE SUPPORT ELEMENTS (BEAMS, COLUMNS, AND BEARING WALLS) ARE TO BE REMOVED.
2. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF THE CALIFORNIA CONSTRUCTION SAFETY ORDERS (CAL-OSHA).
3. TEMPORARY CUTS SHALL NOT EXCEED SLOPES RECOMMENDED IN THE SOIL REPORT, NOR THOSE SHOWN ON THE SHORING DRAWINGS FOR CONSTRUCTION OF FOUNDATIONS.
4. THE INSTALLATION OF SHORING AND EXCAVATIONS SHALL BE PERFORMED UNDER THE CONTINUOUS INSPECTION AND APPROVAL OF THE GEOTECHNICAL ENGINEER.
5. THE DESIGN OF THE SHORING SYSTEM SHALL BE BASED UPON RECOMMENDATIONS CONTAINED IN THE SOIL REPORT. THE SHORING CONTRACTOR SHALL FAMILIARIZE HIMSELF WITH ALL DATA THEREIN AND SHALL KEEP A COPY OF THE REPORT AT THE JOB SITE AT ALL TIMES.
6. THE STRUCTURAL ENGINEER RESPONSIBLE FOR THE SHORING DESIGN (HEREAFTER CALLED THE SHORING ENGINEER) SHALL MAKE PERIODIC VISITS TO THE JOB SITE FOR THE PURPOSE OF OBSERVING THE INSTALLATION OF THE SHORING SYSTEM. OBSERVATIONS SHALL INCLUDE, BUT SHALL NOT BE LIMITED TO, THE FOLLOWING:
 - A. PRIOR TO THE START OF INSTALLATION, MEET WITH THE GEOTECHNICAL ENGINEER AND SHORING CONTRACTOR TO REVIEW ALL ASPECTS OF THE DESIGN AND INSTALLATION OF THE SHORING.
 - B. REVIEW OF CONDITIONS AT COMPLETION OF EXCAVATION. THE CONTRACTOR SHALL NOTIFY THE SHORING ENGINEER AT LEAST 48 HOURS PRIOR TO EACH OF THE ABOVE REQUIRED OBSERVATIONS. CHECKPOINTS SHALL BE ESTABLISHED BY A LICENSED SURVEYOR TO MONITOR ANY HORIZONTAL AND VERTICAL MOVEMENTS OF THE SHORING. INITIAL READINGS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL, AND WEEKLY DATA SHALL BE SUBMITTED AS EXCAVATION PROGRESSES AND THROUGHOUT THE CONSTRUCTION PERIOD.
 - C. ADDITIONAL READINGS OF THE SHORING WHEN REQUESTED BY THE BUILDING OFFICIAL, SHORING ENGINEER OR GEOTECHNICAL ENGINEER. IF ANY HORIZONTAL OR VERTICAL MOVEMENT OCCURS, THE GEOTECHNICAL ENGINEER, THE SHORING ENGINEER AND THE GEOTECHNICAL ENGINEER SHALL EVALUATE SUCH MOVEMENT AND RECOMMEND CORRECTIVE MEASURES, IF NECESSARY BEFORE EXCAVATION IS CONTINUED.

1.	REINFORCING GRADES FOR CONCRETE OR MASONRY: • ALL BARS EXCEPT THOSE TO BE WELDED ASTM A615, GRADE 60 • TIES AND STIRRUPS ASTM A615, GRADE 60 • WELDED WIRE FABRIC ASTM A1064 • ALL BARS TO BE WELDED ASTM A706, GRADE 60 • ALL BARS IN THE CONCRETE SHEARWALL INDICATED ON WALL ELEVATION • INCLUDING SUPPORTING WALL, FOOTING, LONGITUDINAL BARS (WF), AND • ALL GRADE BEAMS (GB) A706, GRADE 60																							
2.	MAINTAIN MINIMUM CONCRETE COVER FROM FACE OF CONCRETE TO EDGE OF ALL REINFORCEMENT AS FOLLOWS (IN O.I.) AND PROVIDE THE LARGEST COVER REQUIRED FOR ALL APPLICABLE CONDITIONS. WHERE #3 STIRRUPS OR TIES ARE USED, ENSURE THAT THE COVER FOR LONGITUDINAL BARS IS ADEQUATE (SEE PLAN SECTION FOR CONCRETE W/ FLOOR FINISH) <table border="0"> <tr> <td>CONDITION</td> <td>COVER</td> </tr> <tr> <td>CONCRETE POURED AGAINST EARTH</td> <td>3"</td> </tr> <tr> <td>CONCRETE POURED IN FORMS AND EXPOSED TO WEATHER OR EARTH</td> <td></td> </tr> <tr> <td> #6 BARS AND LARGER</td> <td>2"</td> </tr> <tr> <td> #6 BARS AND SMALLER</td> <td>1 1/2"</td> </tr> <tr> <td>INTERIOR COLUMNS AND BEAMS</td> <td>1 1/2"</td> </tr> <tr> <td>INTERIOR WALL FACES AND RAISED SLABS</td> <td>1"</td> </tr> <tr> <td>STRUCTURAL SLABS ON GRADE</td> <td></td> </tr> <tr> <td> FROM BOTTOM OF SLAB</td> <td>2"</td> </tr> <tr> <td> FROM TOP OF SLAB</td> <td>1 1/2"</td> </tr> <tr> <td>OTHER CONCRETE NOT EXPOSED TO WEATHER OR EARTH FOR #11 BARS AND SMALLER</td> <td>1"</td> </tr> </table>	CONDITION	COVER	CONCRETE POURED AGAINST EARTH	3"	CONCRETE POURED IN FORMS AND EXPOSED TO WEATHER OR EARTH		#6 BARS AND LARGER	2"	#6 BARS AND SMALLER	1 1/2"	INTERIOR COLUMNS AND BEAMS	1 1/2"	INTERIOR WALL FACES AND RAISED SLABS	1"	STRUCTURAL SLABS ON GRADE		FROM BOTTOM OF SLAB	2"	FROM TOP OF SLAB	1 1/2"	OTHER CONCRETE NOT EXPOSED TO WEATHER OR EARTH FOR #11 BARS AND SMALLER	1"	
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3. REINFORCEMENT SHALL BE PLACED IN ACCORDANCE WITH THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI) "MANUAL OF STANDARD PRACTICE". EACH REINFORCING BAR SHALL BE WIRED TO A CROSS BAR AT A MAXIMUM SPACING OF 24" O.C. PROVIDE ALL ACCESSORIES NECESSARY TO SUPPORT REINFORCING IN POSITIONS SHOWN ON THE PLANS.
4. SPLICES IN CONTINUOUS REINFORCEMENT SHALL BE USED IN WALLS, FOOTINGS, SLABS, ETC., SHALL HAVE A CLASS "B" LAP (1'- 6" MIN) AND THE SPLICES IN ADJACENT BARS SHALL BE NOT LESS THAN 5'- 0" APART. VERTICAL WALL BARS SHALL BE SPACED AT 12" ON CENTER LINES. BARS MAY BE WIRED TOGETHER AT SPLICES OR LAPS EXCEPT FOR TOP REINFORCING OF BEAMS AND SLABS OR WHERE SPECIFICALLY DETAILED TO BE SEPARATED. WELDED WIRE FABRIC SHALL BE SPACED 12" MINIMUM.
5. ALL DOWELS, ANCHOR BOLTS AND OTHER HARDWARE TO BE SET IN CONCRETE SHALL BE PLACED PRIOR TO PLACEMENT OF CONCRETE. NO SAW CUTTING, STABBING, RODDING OR OTHER MOVEMENT OF EMBEDDED ITEMS SHALL BE PERFORMED DURING PLACEMENT OF CONCRETE.
6. BEND REINFORCING BARS COLD.
7. STEEL SHALL BE KEPT CLEAN AND FREE OF RUST.
8. DOWELS BETWEEN FOOTING AND WALLS OR COLUMNS SHALL BE THE SAME GRADE, SIZE AND SPACING AS THE MAIN REINFORCING U.N.O.
9. ALL BARS SHALL BE MARKED SO THEIR IDENTIFICATION CAN BE MADE WHEN THE FINAL IN PLACE INSPECTION IS MADE.
10. CHAIRS OR SPACERS FOR REINFORCING SHALL BE NON-FERROUS OR PLASTIC COATED WHEN RESTING ON EXPOSED SURFACES.

CONCRETE SHALL BE MIXED, PLACED AND CURED IN ACCORDANCE WITH ACI 308, 2019 EDITION, AND PROJECT SPECIFICATIONS.

2. CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL (AS IN WALLS) SO AS TO CAUSE SEPARATION OF AGGREGATES. IN SUCH CASES, HOPPERS AND VERTICAL CHUTES OR TRUNKS SHALL BE USED. CHUTES OR TRUNKS SHALL BE OF VARIABLE LENGTHS SO THAT FREE UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED SIX FEET. A SUFFICIENT NUMBER OF CHUTES OR TRUNKS SHALL BE USED TO ENSURE THE CONCRETE IS KEPT LEVEL AT ALL TIMES.

3. CONSTRUCTION JOINTS SHALL BE INTENTIONALLY ROUGHENED TO A FULL AMPLITUDE OF 1/4 INCH. JOINTS SHALL THEN BE CLEANED AN LAITANCE REMOVED BEFORE NEW CONCRETE IS PLACED. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, JOINTS SHALL BE PREWETTED AND STANDING WATER REMOVED. SEE PLANS AND DETAILS FOR LOCATION AND TYPE OF CONSTRUCTION JOINT. LOCATIONS OF ADDITIONAL CONSTRUCTION JOINTS NOT SHOWN ON THESE PLANS SHALL BE SUBMITTED FOR APPROVAL BY THE SEOR PRIOR TO PLACING ANY CONCRETE.

4. STRUCTURAL CONCRETE SHALL MEET THE FOLLOWING DESIGN CRITERIA:

LOCATION	fc	CONC TYPE	MAX AGGR. SIZE	MAX W/C RATIO	SLAB THICK. (IN)	CEMENT TYPE
FOUNDATIONS, 4000 psi	NWC	1 1/2"	0.45	4"	I/IV	
CONCRETE WALLS						
SLAB ON GRADE	4000 psi	NWC	1"	0.45	4"	I/IV
ALL OTHER STRUCTURAL CONC. NOT NOTED	4000 psi	NWC	1"	0.50	6"	II / V

- MAXIMUM DRY WEIGHT OF LIGHTWEIGHT CONCRETE SHALL BE 110 PCF.
- SLUMP MEASURED PRIOR TO SUPERPLASTICIZER, WHERE OCCURS.

5. CONCRETE MIX DESIGN AND TESTING SHALL MEET THE REQUIREMENTS OF THE BUILDING CODE, AND SPECIFICATIONS. ALL CONCRETE MIXES SHALL BE DESIGNED BY A RECOGNIZED TESTING LAB STAMPED AND SEALED BY A LICENSED CALIFORNIA CIVIL ENGINEER AND SUBMITTED TO THE SEOR FOR REVIEW PRIOR TO CONCRETE PLACEMENT. STRUCTURAL CONCRETE MIXES SHALL CONSIST OF 5 SACK MINIMUM U.N.O.

6. AGGREGATES IN NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C-33 (HARDROCK). AGGREGATES IN LIGHT WEIGHT CONCRETE SHALL CONFORM TO ASTM C-330.

7. COMPRESSIVE STRENGTH TEST REPORTS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND THE SEOR.

8. PORTLAND CEMENT SHALL AS BE NOTED ABOVE FOR ALL CONCRETE CONFORMING TO ASTM C150, LOW ALKALI. MILL TESTS WITH CERTIFICATES OF COMPLIANCE SHALL BE SUBMITTED.

9. FLY ASH OR OTHER POZZOLANS CONFORMING TO ASTM C618 CLASS N OR F MAY USED AS PARTIAL SUBSTITUTION FOR PORTLAND CEMENT UP TO A MAXIMUM OF 15% TOTAL CEMENTITIOUS MATERIALS BY WEIGHT IF THE MIX DESIGN IS PROPORTIONED PER ACI 318, SECTION 26.4.3.

10. CONCRETE MIXING OPERATIONS, ETC. SHALL CONFORM TO ASTM C94.

11. LEAN CONCRETE, WHERE SPECIFICALLY INDICATED, SHALL CONTAIN 2 SACKS OF CEMENT PER CUBIC YARD OF CONCRETE.

12. DRYPACK OR NONSHRINK GROUT SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2 TIMES THE SUPPORTING CONCRETE STRENGTH, AND SHALL BY Sika GROUT, OR APPROVED EQUAL WITH VALID COI'S ON FILE (FOR USE IN CONCRETE). FOR THICK GROUT LAYERS FOLLOW MANUFACTURER'S GUIDELINES TO ATTAIN THE REQUIRED STRENGTH, WHICH MAY INCLUDE THE ADDITION OF PEA GRAVEL.

13. DO NOT USE ANY CONCRETE OR GROUT CONTAINING CHLORIDES. WATER USED IN MIX SHALL BE CLEAN AND POTABLE.

14. PRIOR TO ERECTING ANY ELEMENTS THAT LOAD THE FOUNDATION, CONCRETE MUST REACH AN UNCONFINED COMPRESSION STRENGTH OF 2000 PSI MINIMUM AS DETERMINED BY TESTING OR PREVIOUSLY DOCUMENTED DATA. PRIOR TO THE MIXING OF CONCRETE UNDER SIMILAR CONDITIONS, AND MUST BE ALLOWED TO CURE FOR A MINIMUM OF 5 DAYS.

15. FOR INTERIOR SLABS-ON-GRADE AND ALL OTHER SLABS RECEIVING ADHERED COVERS OR NON-STRUCTURAL PARTITIONS AND OTHER EMBEDDED ITEMS NOT EXCEED 4.5". CURING COMPOUNDS USED ON CONCRETE THAT IS TO RECEIVE FINISHES SHALL BE COMPATIBLE WITH TILE AND ADHESIVES OR GROUTS IN ACCORDANCE WITH MANUFACTURER'S DATA AND BE APPROVED BEFORE USE.

16. MAINTAIN CONCRETE ABOVE 50 DEGREES FAHRENHEIT AND IN A MOIST CONDITION FOR A MINIMUM OF 7 DAYS AFTER PLACEMENT UNLESS OTHERWISE ACCEPTED BY SEOR.

17. SEE ARCHITECTURAL DRAWINGS FOR WALL OPENINGS, WALL OFFSETS, CHAMFERS, KEERS, DRIPS AND FOR EXTENT OF DEPRESSIONS, RAMPS, ETC. PROVIDE SLOPES FOR ALL PIPES THROUGH CONCRETE WALLS AND COVERS WITHIN CONCRETE. SEE ARCHITECTURAL DRAWINGS. CORING IS NOT PERMITTED WITHOUT PRIOR APPROVAL BY THE SEOR.

18. SEE ARCHITECT'S PLANS FOR LOCATIONS OF SLAB SLOPES, DEPRESSIONS, CHAMFERS, KEERS, NON-STRUCTURAL PARTITIONS AND OTHER EMBEDDED ITEMS NOT SHOWN ON THE STRUCTURAL PLANS.

19. EXPOSED CORNERS OF SLABS, BEAMS, WALLS, COLUMNS, ETC. SHALL BE FORMED WITH 3/4" CHAMFER, U.N.O.

20. CONSTRUCTION JOINTS (CJ) AND SAWCUT (SC) JOINTS IN SLABS SHALL OCCUR WHERE LOCATED ON PLANS AND DETAILS. CJS SHALL HAVE FORMED PIER CLOSERS. CONSTRUCTION JOINTS IN WALLS AND FOOTINGS NEED NOT OCCUR AT THE SAME LOCATION, U.N.O.

A.B.	ANCHOR BOLT	KLF	KIPS PER LINEAR FOOT
ABV	ABOVE	KSL	KIPS PER SQUARE FOOT
ADDTL	ADDITIONAL	KSI	KIPS PER SQUARE INCH
ADJ	ADJACENT	L	ANGLE
A.F.F.	ABOVE FINISH FLOOR	LBS	POUND
ALT	ALTERNATE	LFRS	LATERAL FORCE
ARCH	ARCHITECT(URAL)		RESISTING SYSTEM
A.T.R.	ALL THREADED ROD	LLBB	LONG LEG BACK TO BACK
BLDG	BUILDING	LLH	LONG LEG HORIZONTAL
BLKG	BLOCKING	LLV	LONG LEG VERTICAL
BLW	BELOW	LONG.	LONGITUDINAL
BM	BEAM	LP	LOW POINT
CN	BOUNDARY NAILING	LWC	LIGHTWEIGHT CONCRETE
B.O.	BOTTOM OF	MAX	MAXIMUM
BOT	BOTTOM	M.B.	MACHINE BOLT
BRG	BEARING	MECH	MECHANICAL
B.S.	BOTH SIDES	MFR	MANUFACTURER
BTWN	BOUNDARY SCREW	MIN	MINIMUM
	BETWEEN	MTL	METAL
	CAMBER OR	(N)	NEW
	C-CHANNEL	NOM.	NOMINAL
C.I.P.	CAST IN PLACE	NS	NEAR SIDE OR NON-SHRINK
CJ	CONTROL	NTS	NOT TO SCALE
CJ	CONSTRUCTION JOINT	NWC	NORMAL WEIGHT CONCRETE
CL	CENTERLINE	OD	OUTSIDE DIAMETER
CLR	CLEAR	O.C.	ON CENTER
CMU	CONCRETE MASONRY UNIT	O.F.	OUTSIDE FACE
COL	COLUMN	OH	OPPOSITE HAND
CONC	CONCRETE		OSHPD PRE APPROVAL OF
CONN	CONNECTION	OPM	MANUFACTURER'S
CONT	CONTINUOUS		CERTIFICATION
CP	COMPLETE PENETRATION	OPNG	OPENING
CSK	COUNTERSINK	PDF	POWDER DRIVEN FASTENER
CR(D)	CENTERED	PENE	PENETRATION
	BAR OR BOLT DIAMETER	PERF	PERFORATED
DBL	DOUBLE	PJ	PANEL JOIST
DC	DEMAND CRITICAL	PL	PLATE
DEMO	DEMOLITION	PLC(S)	PLATE(S)
DET	DETAIL	PLF	POUND PER LINEAR FOOT
DIA	DIAMETER	PLYWD	PLYWOOD
DIAG	DIAGONAL	PREFAB	PREFABRICATED
DIM	DIMENSION	PSF	POUND PER SQUARE FOOT
DIST	DISTANCE	PSI	POUND PER SQUARE INCH
DO	DITTO	PT	PRESSURE TREATED OR
DWG	DRAWING		POST TENSION
(E)	EXISTING	QTY	QUANTITY
E.A.	EACH	RAD, R	RADIUS
EJ	EACH FACE	REF	REFERENCE
EX	EXPANSION JOIST	REIN.F	REINFORCING
EMBED	EMBEDMENT	REQ'D	REQUIRED
ELEC	ELECTRICAL	RJ	ROOF JOIST
ELEV	ELEVATION	R.O.	ROUGH OPENING
EN	EDGE NAILING	S.A.D.	SEE ARCHITECTURAL DRAWINGS
E.O.	EDGE OF	SB	SILL BOLT
ENG	ENGINEER OF RECORD	SC	STANDARD OR SLIP CRITICAL
EQ	EQUAL	SCHED	SCHEDULE
EQUIP	EQUIPMENT		STRUCTURAL ENGINEER
E.S.	EACH SIDE/ EDGE SCREW	SEOR	ON RECORD
E.W.	EACH WAY	SF	SPREAD FOOTING
EXP	EXPANSION	SHT'G	SHEATHING
EXT	EXTERIOR	SIM	SIMILAR
FIN	FINISH	SLRS	SEISMIC LOAD
FJ	FLOOR JOIST		RESISTING SYSTEM
FLR	FLOOR	SMS	SHEET METAL SCREW
FN	FIELD NAILING	SN	SILL NAIL
FN HT	FINISH HEIGHT	S.O.G.	SLAB ON GRADE
FND	FOUNDATION	SS	SQUARE
F.O.	FACE OF	SS	STAINLESS STEEL
F.S.	FAR SIDE OR FIELD SCREW	STD	STANDARD
FS	FINISH SURFACE	STAGG.	STAGGERED
FRMG	FRAMING	STIFF	STIFFENER
FT	FOOT OR FEET	STL	STEEL
FTG	FOOTING	STRUCT	STRUCTURAL
G	GIRDER	T&B	TOP AND BOTTOM
GA	GAUGE	THK	THICK
GALV	GALVANIZED	T.O.	TOP OF
GB	GRADE BEAM	T.O.M.	TOP OF MASONRY
H.A.B.	HIDDEN ANCHOR BOLT	T.O.S.	TOP OF STEEL
HD	HOLDUPN	TRANS.	TRANSVERSE
HDR	HEADER	TYP	TYPICAL
HGR	HANGER	U.N.O.	UNLESS NOTED OTHERWISE
HK	HOOK	VERT	VERTICAL
HORIZ	HORIZONTAL	W	WITH
HP	HIGH POINT	W/O	WITHOUT
HS	HIGH STRENGTH	W/O	WIDE FLANGE/WALL FOOTING
HSS	HOLLOW STRUCTURAL	WLD	WELDED
HT	HEIGHT	WO	WHERE OCCURS
IN	INCH	WP	WORK POINT
INT	INTERIOR	W.S.	WELDED STUD
INT	INTERIOR	WT	WEIGHT
JST	JOIST	WWF	WELDED WIRE FABRIC
	REFERENCE ELEVATION OR	WWM	WELDED WIRE MESH
	WORK POINT	ø	DIAMETER

<u>SHEET LIST</u>	
S-0.1	GENERAL NOTES
S-0.2	GENERAL NOTES
S-1.1	TYP REBAR & S.O.G
S-1.2	TYP FOOTING DETAILS
S-1.3	TYPICAL DETAILS - METAL STUD
S-1.4	TYPICAL DETAILS - METAL STUD
S-1.5	TYPICAL DETAILS - METAL STUD
S-1.6	TYPICAL DETAILS CMU
S-1.7	TYPICAL DETAILS CMU
S-1.8	TYPICAL DETAILS
S-1.9	TYPICAL DETAILS
S-1.10	TYPICAL DETAILS- MEP
S-2.1	FOUNDATION PLAN
S-3.1	ROOF FRAMING PLAN
S-4.1	FOUNDATION DETAILS
S-5.1	FRAMING DETAILS
S-6.1	WALL ELEVATIONS

REDONDO BEACH UNIFIED
SCHOOL DISTRICT

REDONDO UNION HIGH SCHOOL
SEA HAWK BOWL IMPROVEMENTS



VCA ENGINEERS INC
CIVIL • STRUCTURAL • MECH • ELEC • S&E
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[illegible]

PROJECT NO: 2025-40137

ISSUANCE: A#03-126194

DATE ISSUED: 06/22/2026

SCALE: 12" = 1'-0"

S-0.1

GENERAL NOTES



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DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE SPECIFICATIONS AND STANDARD OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC), AS CONTAINED IN THE 15TH EDITION OF "AISC MANUAL OF STEEL CONSTRUCTION".

2. ALL STRUCTURAL STEEL SHALL BE ERECTED PLUMB AND TRUE TO LINE. TEMPORARY BRACING SHALL BE INSTALLED AND SHALL BE LEFT IN PLACE UNTIL OTHER MEANS IS PROVIDED TO ADEQUATELY BRACE THE STRUCTURE.
3. PROVIDE THE FOLLOWING MATERIALS FOR STRUCTURAL STEEL U.N.O.:
STRUCTURAL STEEL GRADES:
SECTION: _____ ASTM/GRADE: _____
ALL WIDE FLANGE SECTIONS, TEES _____ ASTM A992
SQUARE OR RECT. ANGLES _____ ASTM A500, GRADE C (Fy= 50 KSI)
HOLLOW STRUCTURAL SECTIONS (HSS) _____
ROUND HOLLOW STRUCTURAL _____ ASTM A500, GRADE C (Fy= 46KSI)
SECTION (HSS) _____
PIPES _____ ASTM A53 TYPE E OR S, GRADE B (Fy=35 KSI)
PLATES, ANGLES, CHANNELS _____ ASTM A36
& TEES _____
ALL PLATES PART OF SLRS _____ ASTM A572, GRADE 50
MACHINE BOLTS (MB) _____ ASTM A307
HIGH STRENGTH BOLTS (HSB) _____ ASTM F3125 GRADE A325
_____ GRADE A490
WELDED HEADED STUDS _____ ASTM A29
THREADED RODS FOR ANCHOR BOLTS _____ ASTM F1554, GRADE 36
4. 1/8" THICK PLATES AND THICKER SHALL BE GAS CUT OR SAW CUT EXCEPT AS OTHERWISE NOTED. ALL BOLTS SHALL BE HIGH STRENGTH BOLTS EXCEPT OTHERWISE NOTED. ALL BOLT HOLES SHALL BE STANDARD HOLES.
5. ALL CONNECTIONS NOT SHOWN SHALL CONFORM TO THE "AISC MANUAL OF STEEL CONSTRUCTION" AND SHALL BE SUBMITTED ON SHOP DRAWINGS FOR REVIEW BY SEOR PRIOR TO FABRICATION.
6. ALL WELDED HEADED STUDS, THREADED STUDS, AND DEFORMED BARS SHALL BE BELT SLOTTED OR OTHERWISE NOTED. ALL WELDED (IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS BY CERTIFIED WELDERS) SO AS TO FULLY DEVELOP THE TENSILE CAPACITY OF THE CONNECTOR.
7. BOLTS WITH UPSET THREADS ARE NOT ALLOWED. USE THE APPROPRIATE NUT AND WASHER TYPE FOR THE SPECIFIED BOLT.
8. ALL STEEL FABRICATION SHALL BE PERFORMED BY A AISC OR CITY OF LOS ANGELES CERTIFIED FABRICATOR.
9. ALL STRUCTURAL STEEL AND MISCELLANEOUS STEEL PERMANENTLY EXPOSED TO THE ELEMENTS SHALL BE HOT DIP GALVANIZED AFTER FABRICATION UNLESS A WEATHER PROOF COATING IS SPECIFIED BY THE ARCHITECT U.N.O. STAINLESS AND WEATHERING STEELS ARE EXCEPTED WHERE SPECIFIED.
10. SEE ARCHITECTURAL DRAWINGS FOR NAILER HOLES, WELDED STUDS OR OTHER ITEMS NOT SHOWN IN THESE DRAWINGS. WHERE STEEL IS EMBEDDED IN CONCRETE OR MASONRY, PROVIDE HOLES AS REQUIRED FOR PASSAGE OF CONTINUOUS REINFORCING BARS WHERE INDICATED ON DRAWINGS. DO NOT CUT HOLES IN STRUCTURAL STEEL WITHOUT PRIOR APPROVAL OF SEOR.
11. ALL ARCHITECTUALLY EXPOSED STRUCTURAL STEEL (AESS) SHALL COMPLY WITH AISC CODE OF STANDARD PRACTICE, SECTION 10.
12. PLACE NON-SHRINK OR DRYPACK GROUT UNDER ALL BASE PLATES AND ALLOW TO CURE BEFORE APPLYING LOADS.
13. ALL OPEN HSS ENDS SHALL BE CAPPED. MIN 1/4" STEEL CAP. PROVIDE SQUARE WELD ALL AROUND, CAP PLATE TO HSS.
14. FOR STRUCTURAL STEEL, IN ADDITION TO THE REQUIREMENTS OF SPECIFICATION SECTION A3.1c, HOT ROLLED SHAPES WITH FLANGES 1 1/2" THICK AND THICKER SHALL HAVE MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20FT-LB. SHAPES 1 1/2" THICK AND THICKER SHALL HAVE A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20FT-LB AT 70°F.

HIGH-STRENGTH BOLT NOTES:

1. SEE STRUCTURAL STEEL THIS SHEET FOR ADDITIONAL INFORMATION.
2. ALL HIGH-STRENGTH BOLTS SHALL CONFORM TO ASTM F-3125 G. A-325 OR U.G.-400, NUTS SHALL CONFORM TO ASTM A-563 AND WASHERS SHALL CONFORM TO ASTM F-436.
3. HIGH STRENGTH BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH THE CURRENT EDITION OF THE "AISC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS". SLIP CRITICAL BOLTS (SC) SHALL BE USED FOR ALL "SEISMIC LOAD RESISTING SYSTEM" (SLRS) MEMBER STEEL-TO-STEEL CONNECTIONS.
4. PAINT SHALL NOT BE PERMITTED ON CONTACT SURFACES UNLESS NOTED OTHERWISE. CONTACT SURFACES OF BOLTED PARTS SHALL BE DESCALED AND FREE OF DIRT, OIL, BURRS, PITS, AND OTHER DEFECTS WHICH PREVENT SOLID SEATING OF PARTS.
5. FAYING SURFACE SHALL BE "CLASS A" FOR SLIP CRITICAL BOLTS (SC).
6. SLIP-CRITICAL JOINT ASSEMBLIES SHALL BE FULLY PRE-TENSIONED BY TURN-OF-NUT TIGHTENING, CALIBRATED WRENCH TIGHTENING, INSTALLATION OF ALTERNATE DESIGN BOLTS OR BY DIRECT TENSION INDICATOR TIGHTENING.

METAL DECK NOTES (UNFILLED):

1. MATERIAL FOR METAL DECK SHALL HAVE A MIN VILY STRENGTH OF 50 KSI AND CONFORM TO ASTM A653-SS GRADE 33 WITH GALVANIZED G60 COATING COMPLYING WITH ASTM A525.
2. SEE TYPICAL DETAILS FOR REINFORCING OF DECK AROUND OPENINGS. CONTRACTOR SHALL COORDINATE SIZE AND LOCATIONS OF OPENINGS WITH ALL MECHANICAL TRADES. NO LOADS SHALL BE HUNG FROM DECK WITHOUT APPROVAL OF SEOR.
3. FLOOR AND ROOF DECK IS DESIGNED FOR UNSHORED CONSTRUCTION. UNO. MAINTAIN 3" MIN. CLEARANCE WITH OVERHEADS WHERE POSSIBLE (2' SPAN MIN.) EXCEPT AT STAIR LANDING AND WHERE NOTED OTHERWISE ON PLANS.
4. PROVIDE 2" MINIMUM BEARING AT ALL SUPPORTS. END LAPS OF METAL DECK SHALL BE A MINIMUM OF 2' AND SHALL BE OVERLAPPED ON TOP OF SUPPORTS. DECK SHALL BE LAID OUT SO THAT A LOW FLUTE FALLS ON EACH PARALLEL SUPPORT.
5. INSTALL DECK BY WELDING. USE 3/4" DIAMETER PUDDLE WELDS OR WELDED STUDS TO SUPPORTS SPACED AS SHOWN ON CONSTRUCTION DRAWINGS. SPACING FOR TOP SEAM, SIDE SEAM, BUTTON PUNCH, OR PUNCHLOCK CONNECTION SHALL BE IN ACCORDANCE WITH DRAWINGS. SEE TYPICAL METAL DECK DETAILS.
6. SUBMIT SHOP DRAWINGS FOR METAL DECK TO THE SEOR FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW TYPE OF DECK, LAYOUT OF METAL DECK, THE SIZE AND LOCATION OF ANY OPENINGS OF WIDTH GREATER THAN 1'-0", AND ATTACHMENT METHOD.
7. ALTERNATES TO TYPE OF DECK AND FASTENING MAY BE USED WITH THE APPROVAL OF THE SEOR. DECK PROPERTIES SHALL BE EQUAL TO OR GREATER THAN THOSE SHOWN ON THE PLANS. ANY DECK OR METHOD OF FASTENING SHALL HAVE LATEST EVALUATION REPORT PER CURRENT CODE APPROVING THE DECK FOR THE APPLICATION.
8. METAL DECK WITH CONCRETE FILL SHALL HAVE POSITIVE VENTING. DO NOT EMBED PIPES, SLEEVES, CONDUIT, ETC IN CONCRETE TOPPING UNO.
9. CONCRETE FILL OVER METAL DECK SHALL NOT BE OVER-POURED TO ACHIEVE LEVEL FLOOR.

1. DESIGN, FABRICATION AND ERECTION OF COLD-FORMED STEEL FRAMING SHALL CONFORM TO THE SPECIFICATIONS AND STANDARD OF THE AMERICAN IRON AND STEEL INSTITUTE (AISI), AS CONTAINED IN THE "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS"; LATEST EDITION, INCLUDING ALL APPLICABLE AMENDMENTS.
2. ALL COLD-FORMED STEEL FRAMING SHALL BE ERECTED PLUMB AND TRUE TO LINE. TEMPORARY BRACING SHALL BE INSTALLED AND LEFT IN PLACE UNTIL OTHER MEANS IS PROVIDED TO ADEQUATELY BRACE THE STRUCTURE.
3. COLD-FORMED STEEL GRADES:
A. 18 GA (43 MILS) OR THINNER ASTM A1003 GRADE 33 (FY = 33 KSI)
B. 16 GA (54 MILS) AND THICKER ASTM A1003 GRADE 50 (FY = 50 KSI)
4. ALL COLD-FORMED STEEL FRAMING SHALL BE BRACED AS REQUIRED BY SECTION D3 OF THE AISI SPECIFICATION.
5. SUBMIT COLD-FORMED STEEL FRAMING SHOP DRAWINGS AND SPECIFICATIONS TO THE SEOR FOR REVIEW PRIOR TO FABRICATION.
6. COLD-FORMED STEEL STUDS AND TRACKS ARE TO BE ATTACHED WITH SHEET METAL SCREWS (SMS) WITH SIZES CALLED OUT ON THE DETAILS. PENETRATION OF SCREWS THROUGH JOINED MATERIAL SHOULD NOT BE MORE THAN 3 EXPANSION THROUGHS. SCREWS ARE TO BE INSTALLED AND TIGHTENED IN ACCORDANCE WITH SCREW MANUFACTURER'S RECOMMENDATIONS.
7. ALL HOLES FOR BOLTS SHOULD BE SHALL BE STANDARD HOLES.
8. GALVANIZED COATING SHALL COMPLY WITH ASTM A924.
9. COLD-FORMED STEEL FRAMING SHALL BE PER STEEL STUD MANUFACTURER'S ASSOCIATION (SSMA), ICC E-306AP.

1. WELDING PROCEDURES, ELECTRODES AND WELDER QUALIFICATIONS SHALL CONFORM TO THE "CODE FOR WELDING IN BUILDING CONSTRUCTION" (AMERICAN WELDING SOCIETY (AWS), D1.1, D1.8 AND THE AISC "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS").
2. ALL WELDERS SHALL HAVE EVIDENCE OF PASSING THE AWS STANDARD QUALIFICATION TESTS, AND SHALL BE CERTIFIED FOR THE WORK THEY ARE PERFORMING.
3. PROJECT WELDING SHALL BE PERFORMED ONLY IN ACCORDANCE WITH WELDING PROCEDURE SPECIFICATIONS (WPS) SUBMITTED BY THE CONTRACTOR AND REVIEWED BY THE SEOR AND PROJECT WELDING INSPECTOR. THE WPS SHALL BE IN ACCORDANCE WITH AWS D1.1-D1.4 & D1.8 CURRENT EDITION.
4. ALL WELDS WITHIN MEMBERS DESIGNATED AS PART OF THE SEISMIC LOAD RESISTING SYSTEM (SLRS) SHALL CONFORM TO THE DETAILING, MATERIALS, WORKMANSHIP, TESTING, AND INSPECTION REQUIREMENTS PER AWS D1.8 AND MUST HAVE A MIN. CVM TOUGHNESS OF 20 FT-LB @ 0°F PER AISC 341 A3.4A.
5. NOT USED.
6. WELDING OF STRUCTURAL STEEL SHALL BE PERFORMED PER AWS D1.1 & D1.8 USING E70XX ELECTRODES UNLESS OTHERWISE NOTED.
7. WELDING OF REINFORCING BARS SHALL BE PERFORMED PER AWS D1.4 USING E60XX ELECTRODES.
8. WELDING OF METAL DECK AND LIGHT GAUGE STEEL SHALL BE IN ACCORDANCE WITH AWS D1.3.
9. ALL GROOVE OR BUTT WELDS SHALL BE COMPLETE PENETRATION WELDS. ALL EXPOSED BUTT WELDS SHALL BE GROUND SMOOTH.
10. ALL EXPOSED WELDS ON ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS) SHALL COMPLY WITH AISC CODE OF STANDARD PRACTICE, SECTION 10.
11. FIELD WELDS HAVE BEEN INDICATED WHERE THEY ARE EXPECTED TO OCCUR. THE CONTRACTOR SHALL DETERMINE THE ACTUAL FIELD WELDING NECESSARY TO COMPLETE THE PROJECT AND INCLUDE ALL ASSOCIATED COSTS WITHIN THE BASE BID.
12. ALL WELDS (SHOP AND FIELD) REQUIRE SPECIAL INSPECTION.
13. FULL PENETRATION WELDS SHALL BE ULTRA-SONIC TESTED PER AWS D1.1 & AISC 341. IF 2.5% ULTRASONIC TESTING SHALL BE REQUIRED ON 100% OF COMPLETE-JOINT-PENETRATION IN MATERIALS 5/16 INCH THICK OR GREATER. UT IN MATERIALS LESS THAN 5/16 IN THICK IS NOT REQUIRED.

1. MATERIAL FOR METAL DECK SHALL HAVE A MIN VIELD STRENGTH OF 50 KSI AND CONFORM TO ASTM A663-SS GRADE 33 WITH GALVANIZED 60 COATING COMPLYING WITH ASTM A525.
2. SEE TYPICAL DETAILS FOR REINFORCING OF DECK AROUND OPENINGS. CONTRACTOR SHALL COORDINATE SIZE AND LOCATIONS OF OPENINGS WITH THE VARIOUS TRADES. NO LOADS SHALL BE HUNG FROM DECK WITHOUT APPROVAL OF SEOR.
3. FLOOR AND ROOF DECK IS DESIGNED FOR UNSHORED CONSTRUCTION, UNO. MAINTAIN 3 SPAN CONDITION WHEREVER POSSIBLE (2 SPAN MIN) EXCEPT AT STAIR LANDING AND WHERE NOTED OTHERWISE ON PLANS.
4. PROVIDE 2" MINIMUM BEARING AT ALL SUPPORTS. END LAPS OF METAL DECK SHALL BE A MINIMUM OF 2" AND SHALL OCCUR ONLY OVER SUPPORTS. DECK SHALL BE LAID OUT SO THAT A LOW FLUTE FALLS ON EACH PARALLEL SUPPORT.
5. INSTALL DECK BY WELDING. USE 3/4" DIAMETER PUDDLE WELDS OR WELDED STUDS TO SUPPORTS SPACED AS SHOWN ON CONSTRUCTION DRAWINGS. SPACING FOR TOP SEAM, SIDE SEAM, BUTT JUNCTION, OR PUNCHLOK CONNECTION SHALL BE IN ACCORDANCE WITH DRAWINGS. SEE TYPICAL METAL DECK DETAILS.
6. SUBMIT SHOP DRAWINGS FOR METAL DECK TO THE SEOR FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW TYPE OF DECK, LAYOUT OF DECK, THE SIZE AND LOCATION OF ANY OPENINGS OF WIDTH GREATER THAN 1'-0", AND ATTACHMENT METHOD.
7. ALTERNATES TO TYPE OF DECK AND FASTENING MAY BE USED WITH THE APPROVAL OF THE SEOR. DECK PROPERTIES SHALL BE EQUAL TO OR GREATER THAN THOSE SHOWN ON THE PLANS. ANY DECK OR METHOD OF FASTENING SHALL HAVE LATEST EVALUATION REPORT PER CURRENT CODE APPROVING THE DECK FOR THE APPLICATION.
8. METAL DECK WITH CONCRETE FILL SHALL HAVE POSITIVE VENTING. DO NOT EMBED PIPES, SLEEVES, CONDUIT, ETC IN CONCRETE TOPPING UNO.
9. CONCRETE FILL OVER METAL DECK SHALL NOT BE OVER-POURED TO ACHIEVE LEVEL FLOOR.

MASONRY UNITS SHALL DEVELOP THE FOLLOWING MINIMUM 28 DAY PRISM COMPRESSIVE STRENGTHS IN ACCORDANCE WITH THE BUILDING CODE:

LOCATION:	ftm	MORTAR PER ASTM C270 (NOTE #4)	GROUT STRENGTH PER ASTM C476
BUILDING WALLS	2000 PSI	TYPE M OR S	2000 PSI
SITE WALLS	2000 PSI	TYPE M OR S	2000 PSI

CONCRETE BLOCK SHALL CONFORM TO ASTM C90 MEDIUM WEIGHT.

GROUT AGGREGATES SHALL CONFORM TO ASTM C404 AND AGGREGATE SIZE SHALL NOT EXCEED 3/8".

MORTAR MIN STRENGTH TO CONFORM TO ASTM C270. MINIMUM STRENGTH MORTAR: TYPE S = 1800PSI, TYPE M = 2500 PSI.

VERTICAL REINFORCING SHALL BE FULL HEIGHT OF WALL AND SHALL BE BRACED AT 6" - 8" MAXIMUM TO PREVENT MOVEMENT WHILE GROUTING.

HORIZONTAL REINFORCING SHALL BE IN BOND BEAM UNITS AND TIED SECURELY TO VERTICAL REINFORCING.

DOWELS, ANCHORS, AND OTHER EMBEDDED ITEMS SHALL BE TIED SECURELY IN PLACE TO PREVENT MOVEMENT WHILE GROUTING. WET SETTING OR STABBING IS NOT ALLOWED.

GROUTING NOTES:

A. PROVIDE CLEANS OUTS IN THE BOTTOM COURSE OF MASONRY FOR EACH GROUT POUR WHEN GROUT POUR EXCEEDS 4 FEET FOR 8-INCH NOMINAL OR 5 FEET 4 INCHES FOR 10-INCH NOMINAL OR WIDER HOLLOW UNIT MASONRY AS FOLLOWS:

a. CLEANOUT OPENINGS IN HOLLOW UNIT MASONRY SHALL BE PROVIDED IN EVERY CELL AT THE BOTTOM OF EACH POUR OF GROUT. ALTERNATIVELY, IF THE COURSE AT THE BOTTOM OF THE POUR IS CONSTRUCTED ENTIRELY OF INVERTED DOUBLE OPEN-END BOND BEAM UNITS, CLEANOUT OPENINGS NEED ONLY BE PROVIDED FOR ACCESS TO EVERY REINFORCED CELL AT THE BOTTOM OF EACH POUR OF GROUT.

b. CLEANOUTS IN MULTI-WYTHE MASONRY SHALL BE PROVIDED FOR EACH POUR BY LEAVING OUT EVERY OTHER UNIT IN THE BOTTOM WYTHE OF THE SECTION BEING POURED OR BY CLEANOUT OPENINGS IN THE FOUNDATION.

c. THE FOUNDATION OR OTHER HORIZONTAL CONSTRUCTION JOINTS AT CLEANS SHALL BE REBAR OR ENDED OF ALL REINFORCING MATERIAL AND MORTAR DROPPINGS BEFORE EACH POUR. THE CLEANOUTS SHALL BE SEALED AFTER INSPECTION AND BEFORE GROUTING.

B. FOR GROUT POURS NOT GREATER THAN 4 FEET FOR 8-INCH NOMINAL OR 5 FEET 4 INCHES FOR 10-INCH NOMINAL OR WIDER HOLLOW UNIT MASONRY, THE TOP OF GROUT POUR SHALL BE AT THE TOP OF CONSTRUCTED MASONRY, OR WITHIN 8 INCHES OF THE TOP OF THE CONSTRUCTED MASONRY. GROUT POURS SHALL NOT TERMINATED AT THE TOP OF CONSTRUCTED MASONRY SHALL COMPLY WITH TMS 602, ARTICLES 3.5 C.3.A THROUGH 3.5 C.3.

GROUT POURS IN EXCESS OF 4 FEET FOR 8-INCH NOMINAL OR 5 FEET 4 INCHES FOR 10-INCH NOMINAL OR WIDER HOLLOW UNIT MASONRY SHALL BE SUBJECT TO THE FOLLOWING:

a. GROUTING SHALL BE DONE IN A CONTINUOUS POUR IN LIFTS NOT EXCEEDING 4 FEET FOR 8-INCH NOMINAL OR 5 FEET 4 INCHES FOR 10-INCH NOMINAL OR WIDER HOLLOW UNIT MASONRY (FOR HIGH LIFT GROUTING SEE DSA IR 21-2).

b. AN APPROVED ADMIXTURE OF A TYPE THAT REDUCES EARLY WATER LOSS AND PRODUCES AN EXPANSIVE ACTION SHALL BE USED.

c. THE GROUTING OF ANY SECTION OF WALL SHALL BE COMPLETED IN ONE DAY WITH NO INTERRUPTIONS GREATER THAN ONE HOUR.

d. FOR MULTIPLE GROUT LIFTS WITHIN A GROUT POUR, EACH GROUT LIFT HEIGHT OF WALL, COLUMN, PIER OR BEAM SHALL BE INSPECTED BEFORE PLACEMENT OF ADDITIONAL UNITS.

e. CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF EACH POUR OF GROUT.

D. WHERE GROUTING HAS BEEN STOPPED FOR MORE THAN AN HOUR, GROUT SHALL BE STOPPED 1 1/2 INCH BELOW A MORTAR JOINT. WHERE A BOND BEAMS OCCUR, THE GROUT POUR SHALL BE STOPPED A MINIMUM OF 1/2 INCH BELOW THE TOP OF THE MASONRY WITH A MINIMUM GROUT COVER ABOVE THE HORIZONTAL REBAR OF 1 INCH.

E. ALL GROUT SHALL BE THOROUGHLY CONSOLIDATED BY VIBRATING IMMEDIATELY AFTER PLACING. SHAKING OR RODDING REBAR IS NOT ALLOWED. CONSOLIDATION SHALL BE BY MECHANICAL VIBRATION, AND RECONSOLIDATE BY MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED, BUT BEFORE PLASTICITY IS LOST.

F. THE MAXIMUM GROUT POUR HEIGHT WILL BE 12'-8" FOR WALLS WITH A NOMINAL THICKNESS OF LESS THAN 12 INCHES, AND 16'-0" FOR WALLS WITH A 12" OR LARGER NOMINAL THICKNESS.

9. BLOCK SHALL BE PLACED IN RUNNING BOND UNLESS NOTED OTHERWISE IN THE ARCHITECTURAL DRAWINGS. WHERE 8-INCH WALL OCCURS THAT HAS REINFORCEMENT ON EACH FACE, BLOCKS SHALL BE OPEN-ENDED H-BLOCKS WITH SINGLE-OPEN ENDED AT DOORS AND CORNERS/INTERSECTIONS.

10. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE SAFETY OF LIFT HEIGHT FOR OPEN ENDED OR FIELD MODIFIED BLOCKS.

11. LAYOUT OF MASONRY BLOCK UNITS SHALL BE RUNNING BOND. U.N.O. BLOCK MODULES/MORTAR JOINTS SHOWN ON THESE DRAWINGS ARE FOR PRESENTATION PURPOSES ONLY, AND NOT INTENDED TO SUPERSEDE ARCHITECTURAL DESIGN REQUIREMENTS.

12. WALL REINFORCEMENT SEE PLANS & ELEVATIONS

13. NOT USED.

14. WHEN THE AMBIENT TEMPERATURE FALLS BELOW 40°F, OR EXCEEDS 100°F, PROVISIONS OF TMS602 / ACI 530 / ASCE/ ARTICLE 1.8C OR ARTICLE 1.8D SHALL BE IMPLEMENTED.

1. FOR ALL ITEMS ON THE CONSTRUCTION DOCUMENTS NOT NOTED WITH A SPECIFIC PRODUCT TYPE OR MANUFACTURER, THE CONTRACTOR SHALL PROVIDE THE PRODUCT SPECIFIED IN THE TABLE BELOW.
2. THE FOLLOWING PRODUCTS SHALL BE INSTALLED PER THE REQUIREMENTS OF THE REFERENCED PRODUCT APPROVALS BELOW, U.N.O.
3. AT CONTRACTOR'S OPTION, PRODUCTS MAY BE SUBSTITUTED FOR LIKE PRODUCTS PER THE SCHEDULE BELOW IF APPROVED BY SEQR.

ITEM	APPROVED PRODUCTS	ICC#	IAPMO ER
SCREW ANCHOR TO MASONRY	SIMPSON TITEN HD SCREW ANCHOR	ESR-1056	-
	HILTI KH-EZ	ESR-3056	-
	DEWALT SCREW-BOLT+	ESR-4042	-
SHOTPIN	HILTI LOW-VELOCITY X-U UNIVERSAL POWER-DRIVEN	ESR-2269	-
	SIMPSON POWER-DRIVEN	ESR-2138	-
	RAMSET POWER-DRIVEN	ESR-1799	-
SHEET METAL SCREW	HILTI KWIK-PRO SDS	ESR-2196	-
	DRIL-FLEX SDS	ESR-3332	-
	PRO-TWIST SELF-DRILL SCREW	ESR-1408	-
WELD STUD /ANCHORS	NELSON BAR ANCHORS/STUD	ESR-2907	-

WELDING ELECTRODES

A. SMAW	E70xx*
B. FCAW	E70xx*

* WELD METAL SHALL BE CAPABLE OF DELIVERING A MINIMUM OF 20 FT-LBS AT -0°F AND 40FT-LBS @ 70°F AS MEASURED BY A CHARPY V-NOTCH IMPACT TEST PER AWS D1.1 CLAUSE 4 PART-D.

* BASE METAL FOR STRUCTURAL SHAPES IN SIZE GROUP 4 & 5 W/ U/I ≥ 11/2" SHALL BE CAPABLE OF DELIVERING A MINIMUM OF 20 FT-LBS AT 70° F. AS MEASURED BY A CHARPY V-NOTCH IMPACT TEST PER A.I.S.C. MANUAL OF STEEL CONSTRUCTION, LATEST EDITION.

1. WELDING SHALL BE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY (AWS) STRUCTURAL WELDING CODE FOR STEEL AWS D1.1 AND D1.8.
2. WELDING PROCEDURE SPECIFICATIONS (WPS) SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL. PRIOR THE ENGINEER FOR APPROVAL, PRIOR TO THE BEGINNING OF WELDING OPERATIONS. THE WPS SHALL INCLUDE THE WELDING PARAMETERS RECOMMENDED BY THE ELECTRODE MANUFACTURER. SEE AWS D1.1 ANNEX M, JOINT DETAILS TO SUIT EXISTING CONDITIONS. THE WELDING ELECTRODE MANUFACTURER'S SPECIFICATIONS SHALL BE ATTACHED TO THE WPS. ALL WELDERS AND INSPECTORS SHALL BE INFORMED OF THE WPS AND SHALL RETAIN A COPY.
3. SEE AWS FOR PREQUALIFIED SMAW WPS'S.
4. SEE AWS FOR PREQUALIFIED FCAW WPS'S.
5. WELDING TO BE DONE BY WELDERS CERTIFIED BY AWS.
6. ONLY SHIELDED METAL ARC WELDING (SMAW) WITH LOW HYDROGEN ELECTRODES OR FLUX CORE ARC WELDING (FCAW), EITHER GAS METAL ARC WELDING OR SELF SHIELDED, SHALL BE USED. FOR SMAW, THE TYPICAL REQUIREMENTS SHALL BE ACCORDING TO NOTE #3 ABOVE. WELDING PROCEDURES SHALL MEET THE REQUIREMENTS OF AWS D1.1 CLAUSE 3.2
7. INDIVIDUAL WELDS SHALL BE CARRIED CONTINUOUSLY TO COMPLETION BEFORE THE JOINT IS ALLOWED TO COOL BELOW THE MINIMUM SPECIFIED PREHEAT AND INTERPASS TEMPERATURE.
8. AT ALL COMPLETE PENETRATION WELDS, WELD BEADS SHALL BE PEENED AFTER EACH PASS, WITH THE EXCEPTION OF THE ROOT PASS AND THE SURFACES PASSES, AS FOLLOWS: TURN SLAG GUN 90 DEG. AND MAKE A MINIMUM OF 4 PASSES WITH DULL CHISEL.
9. TACK WELDS SHALL COMPLY WITH AWS D1.1 CLAUSE 5.17 AND AWS D1.8 CLAUSE 6.12.
10. FOR BEAM FLANGE REINFORCEMENT PLATES, THE ROLLING DIRECTION OF GRAIN SHALL BE INDICATED ON THE PLATE AND INSTALLED SO THAT THE GRAIN DIRECTION IS PARALLEL TO THE LONGITUDINAL AXIS OF THE MEMBER.
11. FULL TIME VISUAL INSPECTION BY AN AWS QC-1 QUALIFIED INSPECTOR IS REQUIRED FOR ALL WELDING.
12. FULL PENETRATION WELDS SHALL BE ULTRA-SONIC TESTED PER AWS D1.1 & D1.8 J6.2. ULTRASONIC TESTING SHALL BE PERFORMED ON 10% OF COMPLETE JOINT PENETRATION IN MATERIALS 5/16 INCH THICK OR GREATER. UT IN MATERIALS LESS THAN 5/16 IN THICK IS NOT REQUIRED.
13. AMPEREAGE, VOLTAGE, POLARITY AND ELECTRODE STICK OUT SHALL BE VERIFIED TO BE IN COMPLIANCE WITH THE ELECTRODE MANUFACTURER'S RECOMMENDATIONS.
14. REMOVE B.U. BARS AND WELD TABS AT ALL FLANGE COMPLETE JOINT PENETRATION (CJP) WELDS AT DUCTILE MOMENT FRAME CONNECTIONS. TYPICAL UNLESS NOTED OTHERWISE.

1. THE WALL SHALL BE WATERPROOFED WITH A HOT MOPPED ASPHALT COATING INCORPORATING FIBERGLASS "YELLOW JACKET" MESH. WATERPROOFING SHALL BE "FLINTKOTE E-M-W" OR EQUAL.

2. A DRAINAGE BLANKET OF GRANULAR SOIL PER GEOTECHNICAL REPORT SHALL BE INSTALLED BEHIND THE WALL TO WITHIN 1 FOOT OF THE TOP OF THE WALL AND EXTEND A HORIZONTAL DISTANCE EQUAL TO ONE FOOT MINIMUM AT THE BASE, A PERFORATED 4 INCH DIAMETER PIPE EMBEDDED IN A 1 FOOT SQUARE, 3/4 INCH DRAINAGE ROCK MATERIAL WHICH IS WRAPPED IN FILTER FABRIC SHALL BE INSTALLED AT THE BASE OF THE WALL. AT LEAST 3 INCHES OF GRAVEL SHOULD BE PLACED BENEATH THE PIPE. DRAIN PIPE SHOULD HAVE A MINIMUM GRADIENT OF ONE PERCENT TOWARD THE DRAIN OUTLET. PLACE PERFORATIONS DOWNWARD. DRAINAGE SYSTEM TO BE FREE-DRAINAGE AND UNOBSTRUCTED AND DISCHARGE THRU WALL AT INTERVALS NOT TO EXCEED 50 FT.

BACKFILL SHALL BE LEVEL
UNLESS NOTED OTHERWISE IN
THESE STRUCTURAL DRAWINGS

The diagram illustrates a cross-section of a retaining wall assembly. On the left, a vertical section of the wall is shown with a height dimension 'H' and a thickness dimension '4'-12\"/>

1. VISUAL OBSERVATIONS WILL BE PERFORMED AT THE DISCRETION OF THE OWNER, ARCHITECT, SEOR, AND AS REQUIRED BY THE BUILDING OFFICIAL IN ACCORDANCE WITH THE BUILDING CODE.
2. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO NOTIFY THE STRUCTURAL ENGINEER AS TO WHEN EACH MAJOR PHASE OF CONSTRUCTION IS READY FOR OBSERVATION A MINIMUM OF FIVE (5) WORKING DAYS IN ADVANCE.
3. THE FOLLOWING MAJOR PHASES OF CONSTRUCTION REQUIRE A SITE VISIT AND STRUCTURAL OBSERVATION REPORT FROM THE SEOR:
 - FOUNDATION REBAR AND ANCHORS - PRIOR TO POUR OF CONCRETE
 - STRUCTURAL FRAMING - AFTER ERECTION AND PRIOR TO CLOSING IN
 - COMPLETION OF THE STRUCTURAL SYSTEM
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THAT NAILING, REINFORCEMENT, WELDS, CONNECTIONS, ETC. ARE VISIBLE FOR OBSERVATION WHEN THE SEOR IS ON SITE AND FOR ANY SCHEDULING DELAYS DUE TO NONCOMPLIANT ITEMS FOUND DURING THE OBSERVATION.

1. THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
2. SPECIAL INSPECTIONS ARE NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVED FABRICATORS MUST SUBMIT A STATEMENT OF WORK AND LIST OF APPROVED FABRICATORS SUCH AS STRUCTURAL STEEL, PRECAST CONCRETE, GLUED LAMINATED TIMBER, ETC.
3. ALL INSPECTIONS SHALL BE PERFORMED BY INDEPENDENT SPECIAL INSPECTORS. JOBSITE TESTS BY THE STRUCTURAL ENGINEER OR BUILDING OFFICIAL DO NOT CONSTITUTE AND ARE NOT A SUBSTITUTE FOR INSPECTIONS BY A SPECIAL INSPECTOR.
4. ALL INSPECTION REPORTS SHALL BE SUBMITTED TO DSA AND SEOR THE FINAL REPORTS BY THE SPECIAL INSPECTOR(S) MUST CERTIFY THAT THE ENTIRE STRUCTURAL SYSTEM COMPLIES WITH THE APPROVED PLANS AND SPECIFICATIONS.
5. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO SEE THAT THESE INSPECTIONS ARE PERFORMED.
6. WORK REQUIRING SPECIAL INSPECTION SHALL BE INSPECTED BY THE SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS PERFORMED AND AT THE COMPLETION OF WORK. CONTINUOUS INSPECTION CONSISTS OF FULL-TIME INSPECTION; PERIODIC INSPECTION CONSISTS OF PART-TIME OR INTERMITTENT INSPECTION.
7. REFER TO DSA 103 SPECIAL INSPECTION FORM FOR ALL TEST AND INSPECTION REQUIRED, AS WELL AS CALIFORNIA BUILDING CODE CHAPTER 17A.

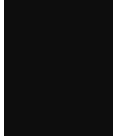
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PROJECT NO:	2025-40137
ISSUANCE:	A#03-126194
DATE ISSUED:	06/22/2026
SCALE:	As indicated

S-0.2

GENERAL NOTES



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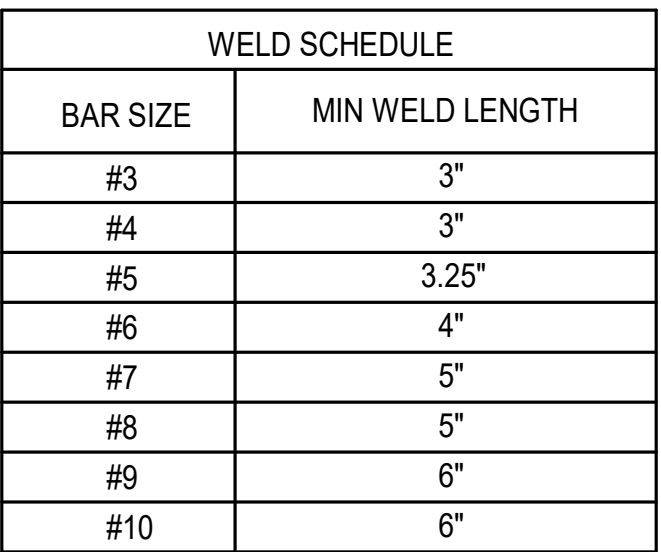
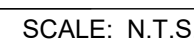
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NOTES:

1. Ldh = HOOK DEVELOPMENT LENGTH.
2. Ld = DEVELOPMENT LENGTH.
3. db = REINFORCING BAR DIAMETER
4. SEE BUILDING CODE AND ACI 318-19 VERSION FOR ALL REQUIREMENTS NOT NOTED.
5. FOR LIGHTWEIGHT CONCRETE MULTIPLY Ldh AND Ld VALUES SHOWN BY 1.33.
6. TOP BARS ARE HORIZONTAL WITH MORE THAN 12" OF FRESH CONCRETE CAST BELOW BARS. ALL OTHER BARS ARE "OTHER" BARS.
7. STANDARD HOOK Ldh DOES NOT ACCOUNT FOR THE EFFECT OF TIES. CONFINED HOOK Ldh MAY BE USED WHERE ACI 318-19 25.4.3 APPLIES.
8. VALUES SHOWN ABOVE ARE FOR GRADE 60 (FY=60 KSI) REINFORCEMENT.
9. SPLICE LENGTHS SHOWN ARE FOR REINFORCING BARS NOT LESS THAN 2db. CONCRETE COVER NOT LESS THAN db.
10. IF EPOXY COATED REINFORCEMENT IS USED, THE DEVELOPMENT LAP SPLICE LENGTHS HEREIN SHALL BE MULTIPLIED BY APPLICABLE FACTORS IN ACI 318-19 SECTIONS 25.4.2.5 OR 25.4.3.2.
11. IN LIEU OF LAP SPLICES, PROVIDE TYPE 2 COUPLERS PER IAPMO ER 0129. STAGGER COUPLERS SO THAT COUPLERS DO NOT OCCUR IN ROW.

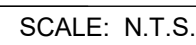
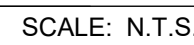
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NOTES:

1. WELDING SHALL BE WITH LOW HYDROGEN ELECTRODES E80XX AND SHALL CONFORM TO AWS-D1.4.
2. ALL WELDING SHALL BE SUBJECT TO CONTINUOUS INSPECTION.
3. REINFORCING BARS TO BE WELDED SHALL MEET THE REQUIREMENT OF ASTM A706.
4. RADIUS OF REINFORCING BAR = S.
5. THESE ARE SECTIONAL VIEWS. BAR DEFORMATIONS ARE SHOWN ONLY FOR ILLUSTRATIVE PURPOSES.

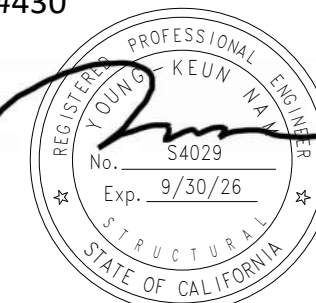
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Project No.: M1202



	DESCRIPTION	DATE
	PREAPP. BID ADD.	10/14/2026

PROJECT NO: 2025-40137

ISSUANCE: A#03-126194

DATE ISSUED: 06/22/2026

SCALE: N.T.S

S-1.1

TYP REBAR & S.O.G

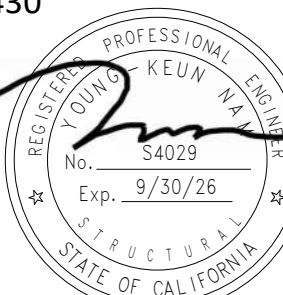


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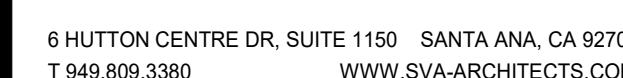


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FOUNDATION DETAILS



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EXHIBIT B-3: PROJECT SPECIFIC SCOPE OF WORK
REDONDO UNION HIGH SCHOOL SEA HAWK BOWL IMPROVEMENTS PROJECT –
PHASE 1 DEMOLITION AND ABATEMENT

GENERAL TERMS AND SPECIAL CONDITIONS

Work items listed in this section are for clarification only and are not intended to be a comprehensive list of inclusions. Your role as a Contractor is crucial, and it is your responsibility to include and or provide all costs for labor, supervision, material, equipment, detailing, hoisting, scaffolding, tools, rigging, engineering, permits, fees, applicable taxes, insurance fees, warranties, guarantees, and any facilities necessary to complete the work.

Work includes, but is not limited to, other related Contract Documents, which are specific requirements for this scope of work, including drawings, specifications, Exhibits, Pre-Bid Request for Information (RFI) Log(s), Project Manual exhibits, specifications, bid documents, and revisions included in the addenda.

1. This Contract shall include all Work assigned to Phase 1 by this Exhibit and the applicable Contract Documents. Except as expressly modified herein, all remaining requirements remain unchanged.
2. Redondo Beach City work hours are Monday through Friday, 7:00 AM to 6:00 PM and Saturdays from 9:00 AM to 5:00 PM. No work is allowed on Sundays or Holidays.
3. Coordinate with the City of Redondo Beach for street / parking stall closures on Vincent Street to perform this scope of work. All costs for the permit shall be included in the Base Bid amount.
4. The contractor is responsible for repairing damage to adjacent finished surfaces that are scheduled, indicated or shown to remain and shall repair from control joint to control joint such damage to like new condition including but not limited to landscaping, concrete, asphalt, etc. at no cost to the owner.
5. The contractor shall take into consideration ongoing campus/student activity in the areas surrounding the work site when planning and performing all work activities for this contract. Any work that creates loud intrusive noise such as concrete cutting, jack hammering, grinding/cutting, of material, heavy equipment, etc. during school hours will need to be coordinated with Balfour Beatty and may need to be conducted in accordance with the schools needs as contractors cost.
6. The contractor shall provide temporary toilet and hand wash stations for their workers. District facilities are not to be used by the contractor. Restrooms shall be maintained and cleaned regularly.
7. Smoking or the use of any tobacco/vaping products are strictly prohibited on District property. Any contractor, worker, supplier or other person on the District property who is violating this law will be immediately removed from the project and will be unable to return.
8. On a monthly basis as specified above, submit updated Construction Schedule:
 - a. Indicate "actual" progress in percent completion for each activity.
 - b. Provide written narrative summary of revisions causing delays in the program. Explain corrective actions taken or proposed.
 - c. Provide on a weekly basis, a Three-Week Look Ahead Schedule in Excel format or as approved by the Construction Manager. The look-ahead schedule will be detailed enough to show all work occurring within the current week plus the following three weeks.

EXHIBIT B-3: PROJECT SPECIFIC SCOPE OF WORK
REDONDO UNION HIGH SCHOOL SEA HAWK BOWL IMPROVEMENTS PROJECT –
PHASE 1 DEMOLITION AND ABATEMENT

PHASE 1 PROJECT - DEMOLITION AND ABATEMENT

9. Perform all demolition and hazardous-material abatement indicated in the Contract Documents only within the school property lines and Phase 1 limits of Work. Furnish all labor, supervision, materials, equipment, permits, temporary facilities, protection and incidentals required for a complete Phase 1 scope.
10. Remove, legally transport, and dispose of demolished and abated materials. Provide required containment, notifications, monitoring, clearances, disposal manifests and abatement closeout records. Stop affected Work and notify the Construction Manager before disturbing suspected hazardous materials not identified in the Contract Documents.
11. Perform utility safe-off, shutdown coordination, disconnection, capping, termination, abandonment, removal, and demolition designated for Phase 1 within the school property. Coordinate shutdowns with the District, Construction Manager, and serving utility agencies. Protect utilities and facilities designated to remain.

BASE BID STORM WATER POLLUTION PREVENTION PLAN (SWPPP) AND BEST MANAGEMENT PRACTICE (BMP) REQUIREMENTS

12. Include required BMP installation, inspections, monitoring, documentation, maintenance, repair, replacement, rain-event preparation and response, sediment removal, and restoration through completion and documented turnover of Phase 1 Work. This obligation includes all required erosion and sediment controls within the Phase 1 limits of Work and is not contingent upon or limited by the availability or use of any allowance. No guaranteed minimum post-turnover service period applies.
13. Inspect controls and plastic coverings before and after each storm event and at least once during each twenty-four (24) hour period during extended storm events, consistent with the SWPPP, applicable permit requirements, and Contract Documents. Monitor weather, maintain replacement supplies, promptly restore damaged controls, and provide required inspection records. These Phase 1 obligations are included in the Base Bid; ordinary rain events do not establish entitlement to additional compensation.

POST-TURNOVER SWPPP / BMP RESPONSE AND COMPENSATION

14. Notwithstanding the post-construction maintenance provisions of General Conditions Section 13.12.4, following documented Phase 1 turnover and until the end of the availability period stated herein, Contractor shall remain the designated project-wide SWPPP/BMP response contractor for authorized inspection, maintenance, repair, replacement, modification, supplementation, and storm response. This requirement is a Project-specific modification applicable to the Phase 1 Work. Contractor shall remain available through April 30, 2027, or until final stabilization is confirmed in writing by the District, whichever occurs first, unless otherwise extended or released in writing by the District. Turnover does not automatically transfer this response role to Phase 2.

EXHIBIT B-3: PROJECT SPECIFIC SCOPE OF WORK
REDONDO UNION HIGH SCHOOL SEA HAWK BOWL IMPROVEMENTS PROJECT –
PHASE 1 DEMOLITION AND ABATEMENT

15. Maintain a current on-call contact and coordinate access and response with the Construction Manager. Respond promptly within applicable SWPPP, permit and Contract Document requirements. On-call availability alone is noncompensable; no separate standby, retainer or guaranteed minimum service charge applies. Compensate only authorized additional Work actually performed. Required inspections and controls remain applicable during dry weather.
16. Additional Work outside the Base Bid requires written authorization by the District representative empowered under the Contract Documents. Use Section 01 26 00 and applicable contract modification procedures, existing rates, unit prices, allowable markups, and supporting cost records. Where no agreed rate applies, submit a supported proposal for approval. Construction Manager review or direction does not authorize a Contract Price change unless expressly delegated. No separate post-turnover SWPPP/BMP allowance or unit-price schedule is established. The Temporary Plastic Sheeting Allowance identified herein is limited to its stated purpose and does not modify the Contractor's Base Bid or post-turnover obligations.
17. Maintain required daily labor, equipment, material and quantity records for directed time-and-material Work. Record verification does not constitute approval of compensation. No additional compensation is due for defective or inadequate Phase 1 Work, improper installation, failure to maintain included controls, Phase 1-caused damage or Work otherwise included in the Base Bid.

EXCLUSIONS AND TURNOVER

18. Exclude new buildings, permanent improvements and all pavement removal and replacement outside the school property lines. Off-property pavement demolition, removal, preparation and replacement are assigned to the separately bid Phase 2 Improvements Project. The replacement pavement material shall be established in the issued Phase 2 Contract Documents.
19. Coordinate documented turnover and future contractor access without assuming Phase 2 award or start dates. Record existing BMPs, coverings, drainage conditions, incomplete items and responsibility allocation. Phase 2 and other contractors shall be responsible for pollutant controls associated with their respective Work and for documented damage they cause to existing BMPs. Upon expiration or written release of the Phase 1 Contractor's post-turnover availability, coordinate with the District and other responsible parties to provide uninterrupted BMP protection and documented transfer of applicable responsibilities. Nothing herein relieves the Contractor of closeout, documentation, or other obligations required by Section 13.12 and the Contract Documents.

EXHIBIT B-3: PROJECT SPECIFIC SCOPE OF WORK
REDONDO UNION HIGH SCHOOL SEA HAWK BOWL IMPROVEMENTS PROJECT –
PHASE 1 DEMOLITION AND ABATEMENT

ALLOWANCE

MARK-UP ON ALLOWANCE ITEMS: Contractors will include all applicable mark-ups on all project-listed allowances in their Base Bid Price. Mark-ups will include overhead, profit, and bond premiums. During construction, mark-ups will not be allowed on change orders and/or expenditures that are to be applied to the allowances since mark-ups were included within the Base Bid. Allowances must be approved prior to use and are at the discretion of Balfour Beatty and/or the District. Contractor to include a line item in their Schedule of Values for allowances. Unused allowances will be deducted via change order from the Contractor's contract.

1. Include in your Base Bid amount the following allowances:

- A. **Temporary Plastic Sheeting Allowance of \$15,000** – For additional temporary plastic ground covering authorized by Balfour Beatty and/or the District beyond erosion-control and BMP measures otherwise required as part of the Base Bid. When authorized, install continuous temporary plastic ground cover over exposed soil and disturbed earth within the Phase 1 limits of Work, extending toward protected drainage facilities as required by the Contract Documents. Use minimum seven (7) mil sheeting unless a more stringent requirement applies. Secure and maintain coverings to preserve soil integrity and minimize erosion and runoff. Use of this allowance requires prior written authorization by Balfour Beatty and/or the District.
- B. **Unforeseen Allowance of \$40,000** – For unforeseen Work as authorized in writing by Balfour Beatty and/or the District