

ADDENDUM NO. 3

MAMMOTH YOSEMITE AIRPORT MAMMOTH LAKES, MONO COUNTY, CALIFORNIA

MULTIPURPOSE BUILDING –PHASE 2 AIP NO. 3-06-0146-____-2026

DESIGN ENGINEER: Brandley Engineering, Inc.
6125 King Road, Suite 201
Loomis, California 95650
Telephone - (916) 652-4725
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BID OPENING: ~~Tuesday, July 14, 2026, at 3:00 p.m.~~
Thursday July 16, 2026 at 3:00 p.m.

This Addendum No. 3 includes the following:

- Revise Bid Opening date from Tuesday, July 14, 2026 to Thursday July 16, 2026
- Revision to select bid document submission deadline.
- Replace Plan Sheet S1.1 with revisions to Design Criteria
- Replace Plan Sheet A50-03 – Revised fall protection and metal canopy detail
- Replace Plan Sheet M03-001A – Revised vehicle exhaust schedules

SPECIFICATIONS

Item 3-1 *Bid Opening Date*

Revise Bid Opening Date from Tuesday, July 14, 2026, at 3:00 p.m. to **Thursday, July 16, 2026, at 3:00 p.m.**

Item 3-2 *Bid Submission Deadline*

Revise Bid Submission Document Deadlines as follows: All documents and forms included in the Proposal and Contract Booklet must be submitted via upload to the opengov procurement portal except for the following items may be submitted 10 calendar days after bid opening by email to Colin Brownlee at cbrownlee@townofmammothlakesca.gov.

List of documents that may be submitted 10 days after bid opening include:

- CARB Certificate of Reported Compliance (CRC) for all subcontractors (note this applies to subcontractor CRCs only. Prime Contractor CRC must still be included in bid submission prior to bid deadline)
- Bidders List Collection Form
- Fostering Small Business Participation Form

PLANS

Item 3-3 *Sheet No. S1.1*

Delete Sheet No. S1.1

Replace with attached Sheet No. A50-03, Addendum No. 3 – Revised Design Criteria

Item 3-4 *Sheet No. A50-03*

Delete Sheet No. A50-03

Replace with attached Sheet No. A50-03, Addendum No. 3 – Revised fall protection and metal canopy detail

Item 3-5 *Sheet No. M03-001A*

Delete Sheet No. M03-001A

Replace with attached Sheet No. SM03-001A, Addendum No. 3 – Revised Vehicle Exhaust Schedules

***** END OF ADDENDUM NO. 3 *****

General

- Interpretation of drawings & specifications
 - For convenience, specifications have been prepared for this project and are arranged in several sections (Structural Steel Buildings), and AISC "Specifications for the Design of Cold-Formed Steel Structural Members".
 - In general, the working details will indicate dimensions, positions and kind of construction, and the specifications will indicate quantities and methods. Any work indicated on the working details mentioned but not in the specifications, or vice versa, shall be furnished as though fully set forth in both. Work not particularly detailed, marked, or specified, shall be the same as similar parts that are detailed, marked, or specified. If conflicts occur between drawings and specifications, the most expensive materials or methods will prevail.
 - Should an error appear in the working details or specifications or in work done by others affecting this work, the contractor shall notify the architect at once and in writing. If the contractor proceeds with the work so affected without having given such written notice and without receiving the necessary approval, decision or instruction in writing from the owner, then he shall have no valid claim against the owner, for the cost of so proceeding and shall make good any resulting damage or defect. No verbal approval, decision, or instruction shall be valid or be the basis for any claim against the owner, its officers, employees or agents. The foregoing includes typical errors in the specifications or notational errors in the working details where the interpretation is doubtful or where the error is sufficiently apparent as to place a reasonably prudent contractor on notice that, should he elect to proceed, he is doing so at his own risk.
- Construction shall conform to all applicable codes and regulations.
- Shop Drawing Note:
 - Shop drawings shall be submitted in the form of one reproducible and two copies of each sheet.
 - The purpose of shop drawing submittals by the Contractor is to demonstrate to the Structural Engineer that he understands the design concept by indicating which materials he intends to furnish and install, and by detailing the fabrication and installation methods he intends to use.
 - Prior to fabrication, shop drawings shall be submitted for review to the Structural Engineer. Shop drawing submittals shall include, but are not necessarily limited to, structural steel, reinforcement, steel joists, girders, beams, and pre-fabricated wood roof framing items such as I-joists and trusses.
 - Prior to submission the Contractor shall review all submittals for conformance with the contract documents and shall stamp submittals as being "Reviewed for conformance".
 - Shop drawing submittals processed by the Structural Engineer are not change orders.
 - Any detail on the shop drawing that deviates from the contract documents shall clearly be marked with the note "This is a change".
 - Shop drawings or calculations submitted for review that require a third resubmittal for re-review shall be billed hourly for such time to the General Contractor. Re-review will not proceed without written approval from the General Contractor for additional engineering review services.
- Safety Note:
 - It is the Contractors responsibility to comply with the pertinent statutes, as they apply to this project, of the "Construction Safety Orders" issued by the State of California latest edition, and all OSHA requirements.
 - The owner and the Structural Engineer do not accept any responsibility for the Contractor's failure to comply with these requirements.
 - The Contractor shall be responsible for adequate design and construction of all forms and shoring required.
- The Contractor shall notify the Architect and Structural Engineer where a conflict or a discrepancy occurs between the structural drawings and any other portion of the contract documents or existing field conditions. Such notification shall be given in due time so as not to affect the construction schedule. In case of a conflict between structural drawings and specifications, the more restrictive condition shall take precedence unless written approval has been given for the least restrictive. Contractor shall verify all dimensions with architectural and structural drawings prior to commencing any work.
- Where no specific detail is shown, the construction shall be identical or similar to that indicated for like cases of construction on this project. Should there be any question, contact the Architect and Structural Engineer for clarification.
- Any substitutions for structural members, hardware, or details shall be reviewed by the Architect and Structural Engineer. Such review will be billed on a time and materials basis to the General Contractor with no guarantee that the substitution will be allowed.
- Do not scale drawings. Contact the Architect or Structural Engineer for any dimensions not shown.
- These drawings are not complete until reviewed and accepted by the local building official and signed by the owner and the Structural Engineer.
- All drawings and written material appearing herein constitutes the original and unaltered work of the Structural Engineer and the same may not be duplicated, used or disclosed without written consent of the Structural Engineer.
- The structure shown on these drawings is structurally sound only in its completed form. The stability of this structure depends on the diaphragms and the bracing members shown. The contractor is to provide for the design and construction of shoring for all earth, forms, concrete, steel, wood, and masonry to resist gravity, earth, wind, seismic, and construction loads. Shoring shall remain in place until all diaphragms and lateral resisting elements are in place in their entirety. Construction materials shall be spread out if placed on framed floors or roofs. Load shall not exceed the design live load per square foot.

Design Criteria

- Code: 2025 California Building Code (CBC)
- Design Live Loads:

Area	Live Load	Remarks
Roof		
A) Flat to < 4:12	$L_r = 20 \text{ psf}$	Reducible per code
B) 4:12 to < 12:12	$L_r = 12-20 \text{ psf}$	Reducible per code
- Snow Design Parameters:

Ground Snow Load	$P_g = 100 \text{ psf}$
Flat-Roof Snow Load	$P_f = 83 \text{ psf}$
Snow Exposure Factor	$C_e = 1.0$
Thermal Factor	$C_t = 1.0$
- Wind Design Parameters:

Basic Design Wind Speed (3-sec gust)	$V = 105 \text{ mph}$
Nominal Design Wind speed (3-sec gust)	$V_{wd} = 82 \text{ mph}$
Risk Category	IV
Exposure Category	C
Internal Pressure Coefficient	10.18
Analysis Method	Directional Procedure
- Earthquake Design Parameters:

Seismic Importance Factor	$I_e = 1.5$
Risk Category	IV
Soil Site Classification	D'
- Seismic Design category
- Mapped Spectral Response Accel

A) Short period	$S_a = 1.740g$
B) 1-sec period	$S_1 = 0.610g$
- Design Spectral Response Accel

A) Short Period	$S_{ps} = 1.434g$
B) 1-sec period	$S_{p1} = 0.641g$
- Seismic Force Resisting System
- Seismic Base Shear
- Seismic Response Coefficient
- 5.10 Component Response Modification Factor
- Analysis Procedure
- Equivalent Lateral Force

Foundations

- Foundation design is based on the geotechnical report by Brandley Engineering, dated October 27th, 2025.
- All building pad preparation and foundation work shall be done in accordance with the requirements of the geotechnical report. Copies of the report may be obtained from the engineer upon request.
- The Geotechnical Engineer shall observe all footing excavations prior to placement of reinforcing steel and concrete.
- Foundation depths indicated on plans are for estimating purposes only. Actual depths are to be determined by the Geotechnical Engineer on the jobsite as required.
- When structural observation is required, structural engineer shall observe footing reinforcing steel prior to concrete placement. Provide 48 hours notice to structural engineer prior to concrete placement.
- The contractor shall be solely responsible for all excavation procedures including, but not limited to, lagging, shoring and protection of adjacent property, structures, streets, and utilities in accordance with the local building department.
- Foundation type: conventional spread footings.

Spread footing design values:	
Allowable Bearing Pressures	
DL + LL	2,000 psf
DL + LL + wind or seismic	2,666 psf
Lateral Resistance	
Passive Pressure	450 psf
Coefficient of friction	0.25
Minimum footing dimensions	
depth = 36"	
width = 36" (spread footings)	

Prefabricated Metal Building

- Design and fabrication shall conform to the 2025 California Building Code (CBC), and the latest editions of AISC "Specifications for the Design, Fabrication, and Erection of Structural Steel Buildings", and AISC "Specifications for the Design of Cold-Formed Steel Structural Members".
- Metal Building Manufacturer (MBM) shall be AISC category "MB" certified.
- Drawings, calculations and engineering data on structural sections for all components shall be submitted to the owner for review prior to fabrication. See Design Criteria Notes for loading information.
- Calculations and drawings shall be signed by a Civil or Structural Engineer registered in the state in which the project is located.
- Building manufacturer shall provide plan drawing showing column locations and anchor bolt locations prior to fabrication. Anchor bolt sizes, numbers, and locations are to be designed and detailed by MBM. MBM shall furnish required anchor bolts and setting templates.
- Contractor shall verify all dimensions with Architectural drawings and MBM column layout prior to foundation construction.
- All hardware for connecting building components shall be designed, detailed and provided by building manufacturer.
- Contractor shall provide temporary erection bracing as required.
- Building designer shall account for the weight of all mechanical equipment in the design of all building components which support such units.
- Foundation design is based on preliminary evaluation of metal building reactions. Final building reactions are to be submitted to the Owner for validation of foundations prior to construction. The foundation design may need to be revised to meet the final building reactions.

Structural Deferred Submittals

- Deferred submittals shall conform to the 2025 CBC.
- The following are structural deferred submittal items:
 - Prefabricated Metal Building
 - Sprinklers
 - Consoles
 - Foundation Design (Foundation Plan shown for bidding purposes only)
- The submittal shall include but shall not be limited to layout drawing, any necessary sections and/or details, and design calculations stamped and signed by a Professional Engineer licensed in the State of California.
- Submittal documents for deferred submittal items shall be submitted to the Architect or Engineer of Record for review prior to submission to the Building Official.
- Ten working days shall be allowed for the Architect or the Engineer to review each deferred submittal.
- The deferred submittal items shall not be installed until their design and submittal documents have been approved by the Building Official.
- Deferred submittals shall be made for enough in advance such that no delay in construction occurs.

Concrete

- Structural concrete shall attain 28-day compressive strength as required in note #30. Maximum slump shall not exceed 4".
- Concrete mix designs shall be prepared by a registered Civil Engineer, reviewed by Owner's testing laboratory and submitted to the Structural Engineer for review. Selection of concrete mix proportions shall be per ACI 318-14 Section 26.4.3, & 26.4-4.
- Concrete mix design shall conform to technical specification section P-610.
- Cementitious materials:
 - Cement shall conform to ASTM C-150 type I or II.
 - Fly ash shall conform to ASTM C-618. Max quantity of fly ash shall be as given in specs (15% max u.n.o.)
- Concrete aggregates shall conform to ASTM C-33 for normal weight concrete and to ASTM C-880 for light weight concrete.
- Water shall be clean and free from injurious amounts of oils, acids, alkalis, salts, organic materials or other substances deleterious to concrete or reinforcement.
- Non-shrink grout or drypack shall consist of a premixed nonmetallic formula. See note #27 for additional information.
- Reinforcing steel shall conform to ASTM A615-grade 60 for 4" and larger, and ASTM A615-grade 40 for 3" and smaller, except reinforcing steel to be welded shall conform to ASTM A706. Contractor shall submit rebar mill certificates.
- Welded wire fabric shall conform to ASTM A-1064.
- All preheating and welding of reinforcing bars shall be done in accordance with AWS D14 latest edition and shall be continuously inspected by a qualified laboratory. Contractor shall furnish WPS for all rebar welding to the laboratory.
- Reinforcing steel shall be fabricated according to "Manual of Standard Practice for Reinforced Concrete Construction".
- Dimensions shown for location of reinforcing are to the face of bars listed and denote clear coverage. Non-prestressed, cast-in-place concrete coverage shall be as follows, u.n.o.:

Cast against earth (except slabs)	3"
Cast in forms and exposed to earth or weather	
#4 & larger	2"
#5 & smaller	1 1/2"
Beams & columns (ties)	1 1/2"
Beams & columns (main rebar)	2"
Cast-in-place walls	
Exterior face & soil side	see above
Interior face - #1 & smaller	3/4"
Tilt-up walls	see details
Slabs (on forms)	3/4"
Slabs (on ground)	2" c/c from top u.n.o.
- Splices in continuous reinforcement shall be lapped u.n.o., lap bars per note 31 u.n.o.. Splices in adjacent bars shall be greater than 5'-0" apart.
- Continuous bars in soil-bearing grade beams, structural slabs on grade and mat foundations as follows u.n.o.: top bars at centerline of support; bottom bars at mid-span. Splice continuous bars in elevated slabs and beams, etc., as follows u.n.o.: lap bars at mid-span; bottom bars at centerline of support. All bars size #14 and larger shall be continuous for full length shown or spliced with mechanical couplers as noted in details. Splices in HWF shall overlap 2 squares minimum.
- The minimum clear spacing between parallel bars in a layer shall not be less than the larger of bar diameter, 1", or 33% greater than the maximum aggregate size (nominal), whichever is greatest. This requirement also applies to the clear spacing between different layers of parallel bars and to the clear distance between a contact lap splice and adjacent splices or bars.
- All hooks shall be standard hooks unless otherwise shown or noted. At walls, provide hooks at ends of all reinforcing ends, corners and intersections, u.n.o.
- Provide constructional joints & all slabs on grade as noted on plan.
- Proposed joint plan shall be submitted to the Structural Engineering for approval prior to construction. Concrete surface at construction joints shall be thoroughly cleaned and laitance removed. Where indicated on drawings, roughen concrete surface to 1/4" amplitude. Concrete may be roughened by chipping the entire surface, sand blasting, or raking the surface to provide 1/4" deep deformations.
- Remove all debris from forms before casting dry concrete.
- Reinforcing, dowels, bolts, ties, anchors, sleeves, etc., to be embedded in concrete shall be securely positioned in forms before placing concrete.
- Pipes and electrical conduits shall not be embedded in structural concrete or concrete fill over metal decking except where specifically approved by the Structural Engineer.
- Anchor bolts (ABs) cast in concrete or masonry for wall sill and ledger/ applications shall be headed bolts with cut threads conforming to ASTM A307 or F1554 u.n.o. Refer to "Wood notes" for additional requirements for bolts in contact with pressure treated or fire retardant material. Refer to "Structural steel" note for requirements for anchor rods cast in concrete for column base plate and steel embed applications.
- Walls shall be cast in horizontal layers of 2'-0" maximum depth.
- Concrete in walls, piers or columns shall set at least 2 hours before placing concrete in beams, spandrels, or slabs supported thereon.
- Consolidate concrete placed in forms by mechanical vibrating equipment supplemented by hand-spooling, rodding or tamping. Use equipment and procedures for consolidation of concrete in accordance with the recommended practices of ACI 304 to suit the type of concrete and project conditions. Concrete shall not be dropped through reinforcing steel (as in walls) so as to cause segregation of aggregates. In such cases hoppers and chutes or trunks of variable lengths shall be used so that the free unfettered fall of concrete shall not exceed 6 feet.
- Drill through steel columns, beams and plates to pass continuous reinforcing, u.n.o.
- No wood spreaders allowed. No wood stakes allowed in areas to be concreted.
- Additional reinforcing in precast or tilt-up panels required for lifting stresses shall be supplied by Contractor.
- Provide #5x4'-0" diagonal reinforcing at mid-depth of slab at all re-entrant corners typical. This applies to slab on grade, concrete over metal deck, and elevated structural slab conditions.
- Place non-shrink grout under base plates, sill plates, etc. as indicated on the drawings. Non-shrink grout shall be Masterflex #28 grout by Master Builders Technologies or approved equal with a minimum f_c of 7500 psi @ 28 days.
- All saw cutting shall be done after initial set has occurred to avoid tearing or damage by the saw blade, but before initial shrinkage has occurred.
- Notify Structural Engineer a minimum of 48 hours before placing any concrete.
- Concrete strength: (max slump = 4")

Use	f_c @ 28 days	Max Aggregate Size	Density (lbs/cu ft)	Max W/C Ratio
All Concrete	4000 psi	1"	145	0.45

32. Development lengths shall be provided per the table below unless noted otherwise.

Straight Bars				With Standard Hooks			
Bar	f_c		Bar	f_c			
	3000 psi	4000 psi		3000 psi	4000 psi		
#3	15"	21"	#3	6"	6"		
#4	24"	25"	#4	11"	10"		
#5	36"	31"	#5	14"	12"		
#6	43"	37"	#6	17"	15"		
#7	63"	54"	#7	20"	17"		
#8	72"	62"	#8	22"	19"		
#9	80"	70"	#9	25"	22"		
#10	84"	78"	#10	28"	24"		
#11	98"	85"	#11	31"	26"		

33. Concrete finish shall be as required by the airport. A hard trowel finish is desired inside the building. Contractor shall note that the concrete mix requires 2% ± 0.5% entrained air for interior hard trowel finish areas. Contractor shall exercise extreme caution so that the finishing operations do not cause surface delamination or other defects. Any defective surface finish shall be repaired or replaced by the contractor at no cost to the owner.
34. Areas of concrete located outside of the building shall have 5% ± 1.2% air entrainment and shall have a broom finish.
35. Concrete slab shall not be constructed and left exposed over the winter. The building shall be completed and closed in over the slab prior to winter and freezing temperatures to protect the slab from freezing.

Structural Steel

- Fabrication, erection and materials shall conform to the specifications and standards of the AISC, as contained in the "AISC 360-16 Specifications of Structural Steel Buildings" and the "AISC Manual of Steel Construction", 15th edition and California Building Code latest edition.
- Structural steel shall conform to the following specifications, u.n.o.:

Shapes	
Wide Flanges (W, WT)	ASTM A992
Wide Flanges (S, M), Angles (L)	ASTM A572
Channels (C), Misc. Channels (MC)	ASTM A36 (#B), ASTM A992 (#B)
Hollow Structural Steel (HSS)	ASTM A500, Gr. C ($F_y = 50 \text{ ksi}$)
Steel Circular Pipes (P)	ASTM A53, Type E or S, Gr. B

Plates & Bars	
Column Base Plates	ASTM A36
Brace Gusset Plates	ASTM A36
Beam Shear Connection Plates	ASTM A36
Column Continuity Plates	ASTM A572, Gr. 50
Beam Stiffener Plates	ASTM A36
Deck Closure Plates	ASTM A36
Stainless Steel Plates & Bars	ASTM A276
Other	ASTM A36

Nuts, Bolts, Rods, & Washers	
General Bolts	ASTM F3125, Gr. A325-N
Slip Critical Bolts (see note #4 below)	ASTM F3125, Gr. A325-SC
High Strength Bolts	ASTM F3125, Gr. A325-N or Gr. A440
Machine Bolts (general use)	ASTM A307
Bent & Headed Anchor Bolts	ASTM F1554, Gr. 36, 55, or 105
Partial & Fully Threaded Anchor Rods	ASTM F1554, Gr. 36, 55, or 105
Fully Threaded Rod (general use)	ASTM A36 (A307 Gr. A for 3/8")
Welded Shear Connectors	ASTM A108, Gr. 1015 thru 1020
Welded Threaded Studs	ASTM A108, Gr. 1015 thru 1020
Nuts for Bolts & Machine Bolts	ASTM A563
Hardened Washers	ASTM F436
Unhardened Washers	ASTM F844
Plain Washers	ASTM B16.22.1
Beveled Washers	ASTM B16.23.1

 - Welded connections shall consist of unfinished bolts per the table above unless noted otherwise. Anchor bolts cast in concrete or masonry shall be headed bolts with cut thread, full diameter body style conforming to ASTM F1554 u.n.o., unless noted otherwise, anchor bolts/rods shall be grade 36 except that welded anchor bolts shall be grade 55 per SI Supplement, any requirements. All welded connections and base plates shall have standard cut washers unless noted otherwise. Washers at base plates shall be placed at top and bottom of plate.
 - "Slip-critical" bolted connections:
 - "Slip-critical" connections (A325-SC design values with special inspection) are required at all braced frame connections, at all connections along chord lines and drag lines (as noted on plans), and u.n.o., at all bolts in oversized or slotted holes.
 - The special inspector must be present during installation and tightening operation of "slip-critical" connections.
 - All structural steel shall receive minimum of one shop coat of red primer with a minimum dry film thickness of 2.0 mils. Do not shop prime or paint areas to be field welded, reprotected, galvanized, to receive slip-critical high strength bolts, or to be embedded in concrete. Prior to priming or painting, clean structural steel in accordance with American Society for Protective Coatings (SSPC) recommendations & as required by the primer & paint manufacturer. Provide additional painting as noted in the specifications.
 - All structural steel shall be erected plumb and true to line. Temporary bracing shall be installed and shall be left in place until other means are provided to adequately brace the structure. Contractor responsible for reviewing all base plate and support conditions during erection and bracing as required. See AISC and OSHA requirements.
 - Place non-shrink grout under all base plates before adding vertical load. See Concrete Notes for non-shrink grout requirements.
 - Structural steel members and girders shall have 3" minimum of concrete cover.
 - Provide 1/2" stitch bolts and ring fills, space at not more than 24" cc for all double angle members.
 - At wood to steel parallel contact, attach with 1/2" welded threaded studs at maximum 32" cc, & 6" from ends of wood member, typical unless noted otherwise.
 - Holes for unfinished bolts shall be of the same nominal diameter of the bolt plus 1/8". Use standard AISC gage and pitch for bolts except as noted otherwise. Holes for anchor bolts embedded in concrete shall be of the same nominal bolt diameter plus 3/8" unless noted otherwise.
 - Welding shall be done by the electric arc process in accordance with American Welding Society standards, using only certified welders. All groove welds shall have complete penetration unless noted otherwise. All exposed welds shall be ground smooth. All welding to be done using E70xx electrodes. In addition, welding of ASTM A572 grade 50 steel and ASTM A912 steel shall be done with electrodes capable of depositing weld metal with a maximum diffusible hydrogen content of 16ml/100g (H16). Weld lengths called for on plans are the net effective lengths required.
 - Minimum fillet welds:

3/8" & t < 1/2"	3/16"
1/2" & t < 3/4"	1/4"
3/4" & t < 1"	5/16"
 - Welding Procedure Specifications (WPS) for shop and field pre-qualified weld joints and weld joints qualified by test shall be prepared for review prior to fabrication. All welding procedures that meet these requirements of AWS D11 Sec. 5.1 shall be considered as pre-qualified. Qualification testing is required when the depth of a partial penetration or complete penetration weld is 2" or greater.
 - Structural steel & fasteners that are permanently exposed to weather shall be either primed and painted or hot dipped galvanized in accordance with ASTM A123 & A155. Repair welding in accordance with ASTM A780.
 - When structural steel & connections will be exposed to view in the completed building, they shall be fabricated, erected & finished in compliance with Architecturally Exposed Structural Steel (AESS) guidelines & Section 10 of the AISC 325-22 "Code of Standard Practice for Steel Buildings and Bridges".

Expansion Anchors-Concrete:

- Use Hilti Kwik Bolt-T22 Expansion Anchors as manufactured by Hilti Inc., Tulsa Oklahoma, ICC-ES Report No. ESR-4266 reissued December 2023.
- Installation of anchors shall be in accordance with the manufacturer's recommendations, ICC-ES Report, and these notes.
- Special inspection is required in accordance with the 2025 CBC Sections 1705A.1.1.3 and 1904.5. Special inspector must verify product, expiration date, concrete type and strength, anchor diameter and steel grade, compliance of drill bit, hole diameter and location, cleanliness of hole and anchor, and anchor embedment.
- Each anchor type (loaded in either pullout or shear) shall be torque tested in accordance with CBC Section 1904.5 to the appropriate test load shown in the table. If any anchor fails testing, all anchors of the same type not previously tested shall be tested until 20 consecutive anchors pass, then initial testing frequency may be resumed.
- When installing anchors in existing concrete do not cut or damage existing reinforcing bars. Locate existing reinforcing bars with pachometer or x-ray if required.
- The testing of the anchors shall be done by the Testing Laboratory and a report of the test results shall be submitted to the Building Dept. and Architect/Structural Engineer.
- Anchors installed up into the bottom of metal deck with concrete fill shall be installed in the center of the low flute of the decking. The decking shall have a minimum thickness of 20 gauge. The minimum depth of embedment above the top of the deck shall be 16", the effective depth of embedment is considered to be one-third of the metal deck height plus the depth of embedment above the top of the deck. There shall be a minimum concrete cover of 1" between the top surface of the concrete and the end of the bolt.

Normal Weight Concrete Anchors			
$f_c = 3000 \text{ psi}$			
Hilti Kwik Bolt-T22 Expansion Anchors			
Anchor Diameter	Embedment Effective/Nominal u.n.o.	Carbon Steel Anchors Installation Torque Torque Test Load (ft.-lbs)	Stainless Steel Anchors Installation Torque Torque Test Load (ft.-lbs)
3/8"	2" / 2 1/2"	30	30
1/2"	3 1/4" / 3 3/4"	50	40
5/8"	4" / 4 1/2"	40	60
3/4"	4 3/4" / 5 1/2"	110	125

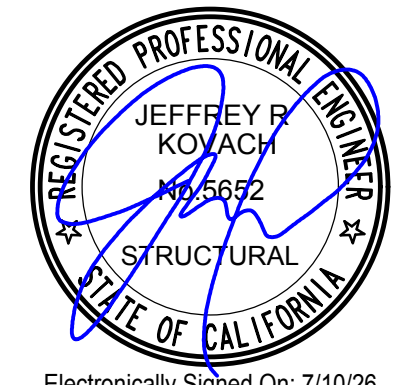
DATE	ISSUED FOR	REV
07/10/26	BID ADDENDUM 3	A

This drawing has been prepared solely for the use of MAMMOTH YOSEMITE AIRPORT and there are no representations of any kind made by NORR to any party with whom NORR has not entered into a contract.

This drawing shall not be used for construction purposes until the seal appearing hereon is signed and dated by the Architect or Engineer.

Project Component
Key Plan

Consultants
Survey: Brandley Engineering
Civil: Kinley-Horn
Architecture: NORR
Structural: Bevier Structural Eng
Mechanical: Seis
Electrical: NORR
Fire Sprinkler: Sacramento Engineering Consultants

Seal(s)

Electronically Signed On: 7/10/26

NORR
2020 I Street, Suite 220 Sacramento, CA, US 95811 norr.com
2479 Sunrise Blvd. Gold River, CA 95670 Tel: (916) 631-3030 Fax: (916) 631-6996 Web: www.bevier.net Bevier Job No 240424

Project Manager	Drawn
Project Leader	Checked
Client	JK
MAMMOTH YOSEMITE AIRPORT	
Project	
MAMMOTH SRE BLUIDING	
MAMMOTH, CALIFORNIA	
Drawing Title	
GENERAL NOTES	

Drawing Title	
GENERAL NOTES	
Scale	
Project No.	IN2024-0022
Drawing No.	S1.1

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LOUVER SCHEDULE				
TAG	MANUFACTURER	MODEL NUMBER	SIZE	
IL-1	RUSKIN	ELF375DX	PER PLAN	ALUMINUM DRAINABLE INTAKE LOUVER. PROVIDE WITH GRAVITY BACKDRAFT DAMPERS
EL-1	RUSKIN	ELF375DX	PER PLAN	ALUMINUM DRAINABLE EXHAUST LOUVER. PROVIDE WITH GRAVITY BACKDRAFT DAMPERS
FINISH TO MATCH ADJACENT EXTERIOR WALL SURFACE. PROVIDE INTGRAL 1-1/2" FLANGE.				

UNIT HEATER SCHEDULE										
TAG	MANUFACTURER	MODEL NUMBER	SERVICE ROOM	BTU/HR INPUT	BTU/HR OUTPUT	ELECTRICAL VOLT PHASE	AMPS	HP	CFM	NOTES
UH - 1,2,3,4	REZNOR	UDXC-100	WAREHOUSE	100,000	87,500	120V/1PH	4.35	1/8	1650	
PROVIDE WITH FIELD INSTALLED ACCESSORIES FROM MANUFACTURER: VERTICAL DEFLECTOR BLADES, NATURAL GAS TO PROPANE CONVERSION KIT, SINGLE STAGE ROOM THERMOSTAT, CONDENSATE PUMP, CONDENSATE PUMP SUSPENSION KIT, CONDENSATE PH NEUTRALIZING KIT, HORIZONTAL CONCENTRIC VENT KIT. PROVIDE AND INSTALL ON CONTRACTOR-SUPPLIED THREADED ROD SUSPENSION WITH DIAGONAL BRACING.										

GAS DETECTION MONITOR SCHEDULE						
EQUIPMENT TAG	AREA SERVED	ELECTRICAL VOLT	1	PART NUMBER	MODEL NUMBER	REMARKS
CO/NO2	VEHICLE BAYS	120	1	04652-0601-0000	GSM-60	
NOTE: 1. ALL MODEL NUMBERS ARE ENMET UNLESS OTHERWISE NOTED. 2. INCLUDES: INSTRUMENT WITH CO AND NO2 SENSORS, WALL MOUNTED ENCLOSURE, LCD DIGITAL DISPLAY, AUDIBLE AND VISUAL ALARMS, INTERNAL MOTORIZED SAMPLE PUMP WITH FLOW SWITCH AND ALARMS, AUXILIARY RELAYS, PARTICLE FILTER, INLET AND OUTLET PORTS, 4-20ma OUTPUT SENSOR.						

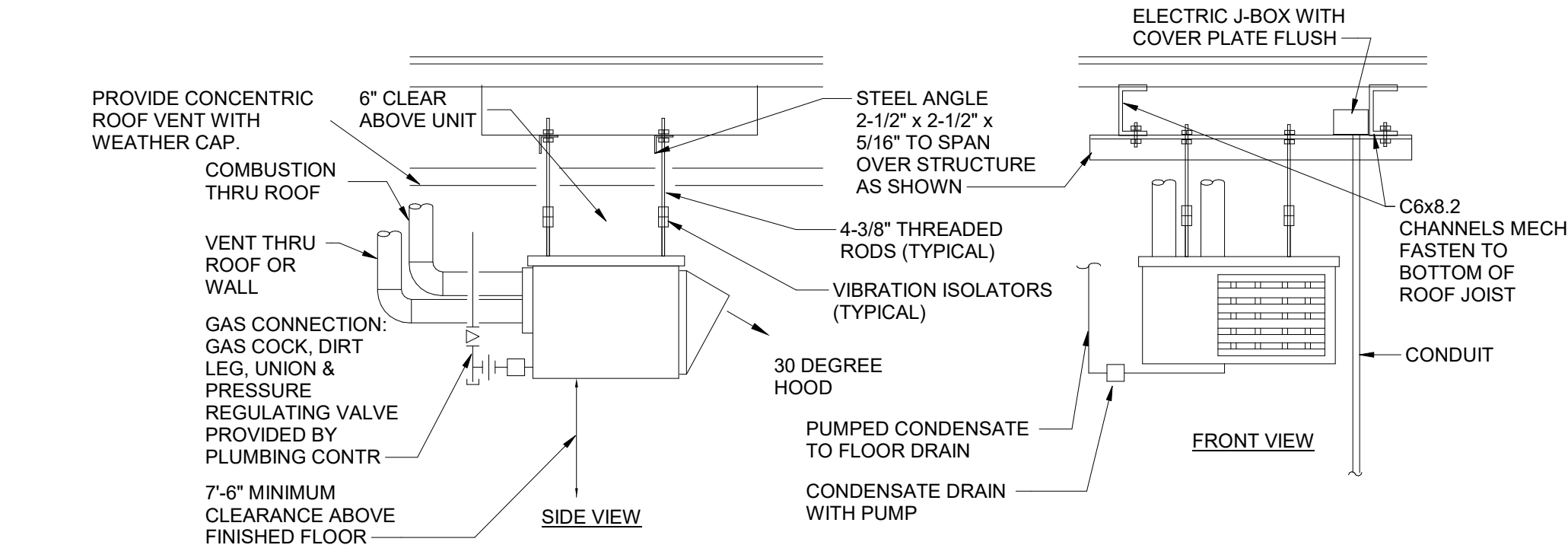
MECHANICAL VENTILATION TABLE REFERENCE FROM CENc TABLE 120.1-A				
ROOM NUMBER	ROOM NAME	AREA	OUTDOOR AIR RATE	REQUIRED OUTDOOR AIRFLOW
ROOM 06	SRE GARAGE	6428	0.15	(6428x 0.15) = 965

AIR BALANCE SCHEDULE	
EXHAUST AIR	
SRE STORAGE AREA	1480 CFM
EXHAUST AIR REQUIRED	
OUTSIDE AIR PROVIDED	
AIR BALANCE EF-1 (N)	1480

VEHICLE EXHAUST SNORKEL FAN SCHEDULE									
EQUIPMENT TAG	MANUFACTURER	MODEL #	CFM	ESP (IN.)	SERVICE ROOM	ELECTRICAL VOLT PHASE	HP	WEIGHT (LBS)	NOTES
EF-1	NEDERMAN	N24 #14510122	883	N/A	SRE GARAGE	208V/1PH	1	37	SEE VE SCHEDULE
EF-2	NEDERMAN	N24 #14510122	883	N/A	SRE GARAGE	208V/1PH	1	37	SEE VE SCHEDULE
PROVIDE WITH: GRAVITY BACKDRAFT DAMPER.									

VEHICLE EXHAUST EXTRACTION SCHEDULE									
TAG	EXHAUST RAIL		MAX EXHAUST TEMP	ELECTRICAL			MODEL NUMBER	WEIGHT (LBS)	ACCESSORIES
	LENGTH	CAPACITY # VEHICLES		VOLT	PHASE	AMP			
EF-1	66	2	300°F	208	3	16.7	MAGNARAIL	338.8	SUPPORT RAIL
EF-2	66	2	300°F	208	3	16.7	MAGNARAIL	338.8	SUPPORT RAIL
NOTE: 1. ALL MODEL NUMBERS ARE NEDERMAN UNLESS OTHERWISE NOTED. 2. INCLUDES COMPLETE SYSTEM INCLUDING: HB NOZZLES, OUTLET CONNECTION, RADIO RECEIVER AND TRANSMITTER, EXHAUST FAN WITH VFD.									

EXHAUST FAN SCHEDULE									
EQUIPMENT TAG	MANUFACTURER	MODEL #	CFM	ESP (IN.)	SERVICE ROOM	ELECTRICAL VOLT PHASE	HP	WEIGHT	NOTES
EF-3	GREENHECK	SBE-1H24	950	0.50	SRE GARAGE	208V/3PH	1/4	65	



2 GAS FIXED UNIT HEATER
M03-001A SCALE: 1/8" = 1'-0"

DATE	ISSUED FOR	REV
2026/05/28	PERMIT	1
2026/07/07	BID ADDENDUM 3	2

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Project Component

Key Plan

Consultants
Survey:
Civil:
Architecture:
Structural: NORR
Electrical:
Interiors:
Landscape:

Seal(s)

NORR

NORR OFFICE ADDRESS
norr.com

Project Manager M. NOVAK	Drawn JL
Project Leader J. PRICE	Checked BMS

Client
MAMMOTH YOSEMITE AIRPORT

Project
MAMMOTH SRE BUILDING

MAMMOTH, CA

Drawing Title
MECHANICAL SCHEDULES AND DETAILS

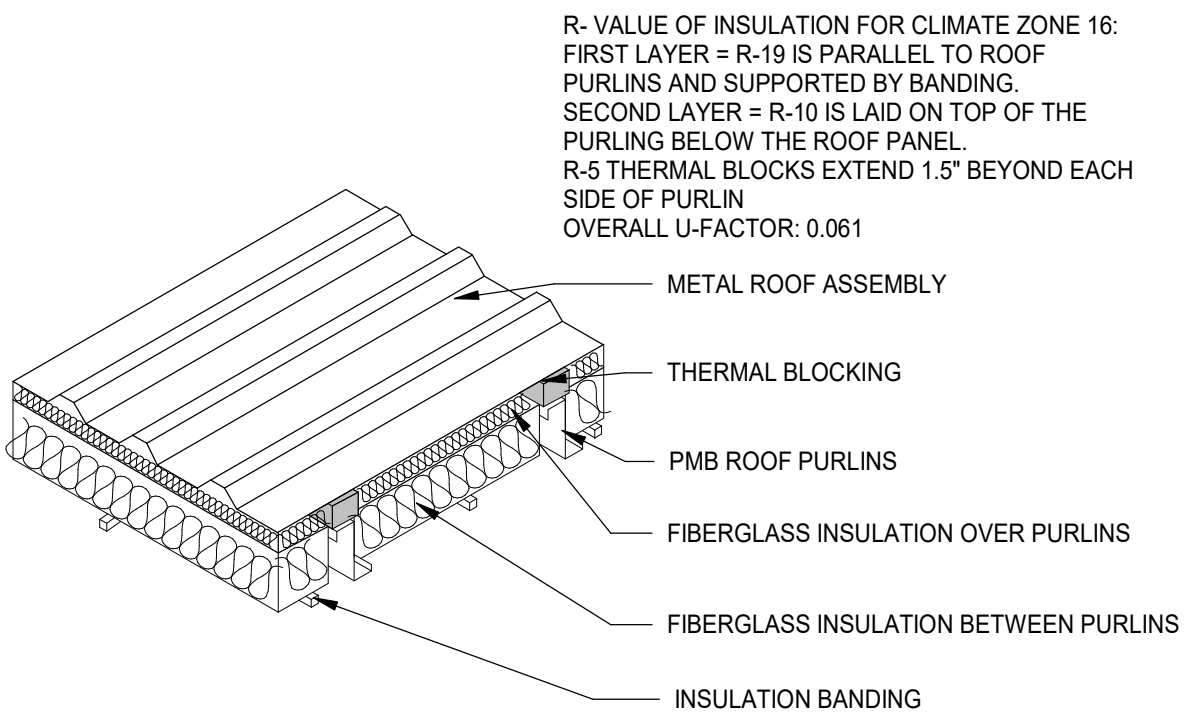
Scale
As indicated

Project No.
IN2024-0022

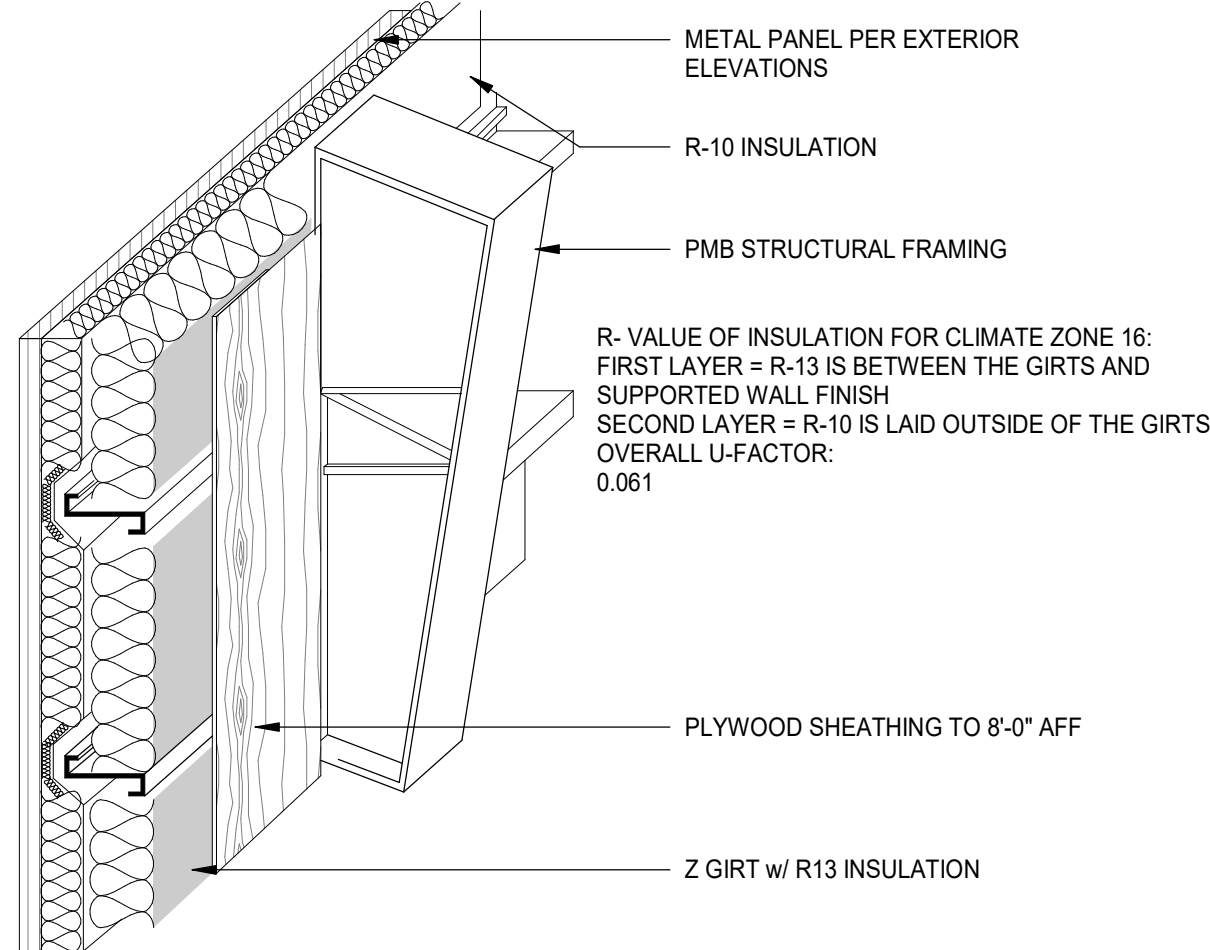
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M03-001A



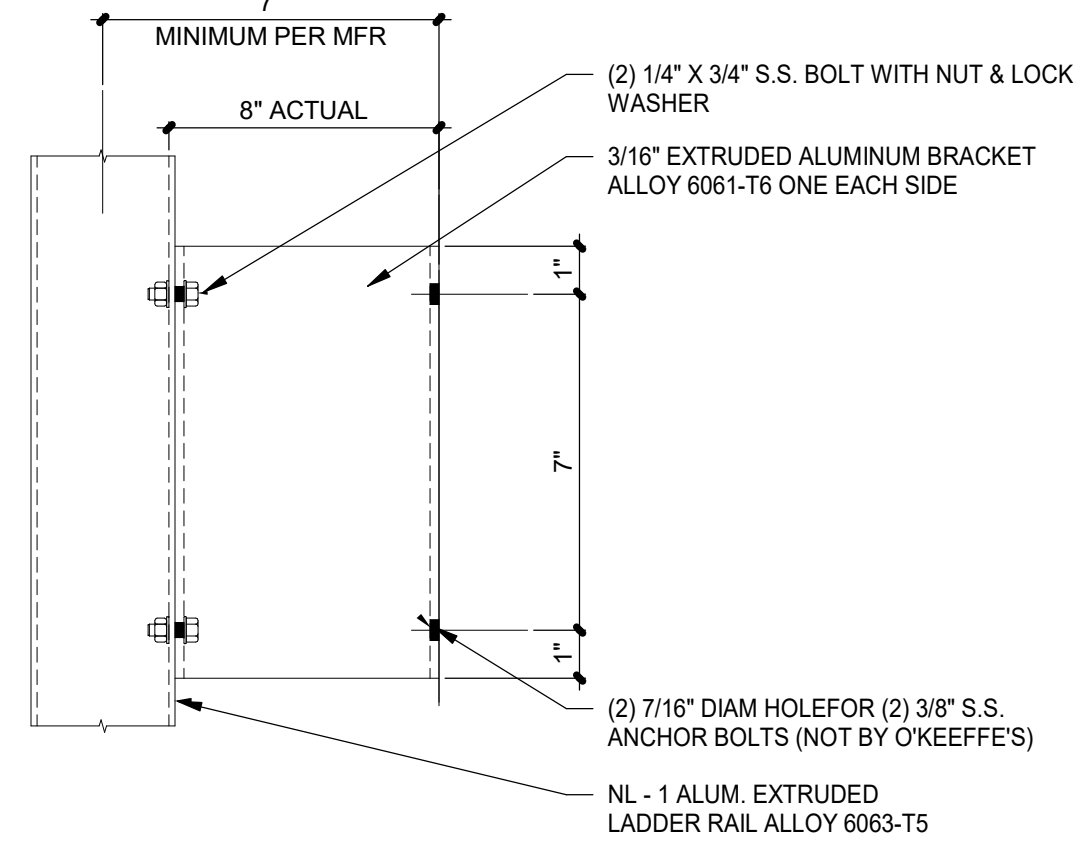
6 FALL PROTECTION TIE OFF ASSEMBLY
SCALE: 1" = 1'-0"



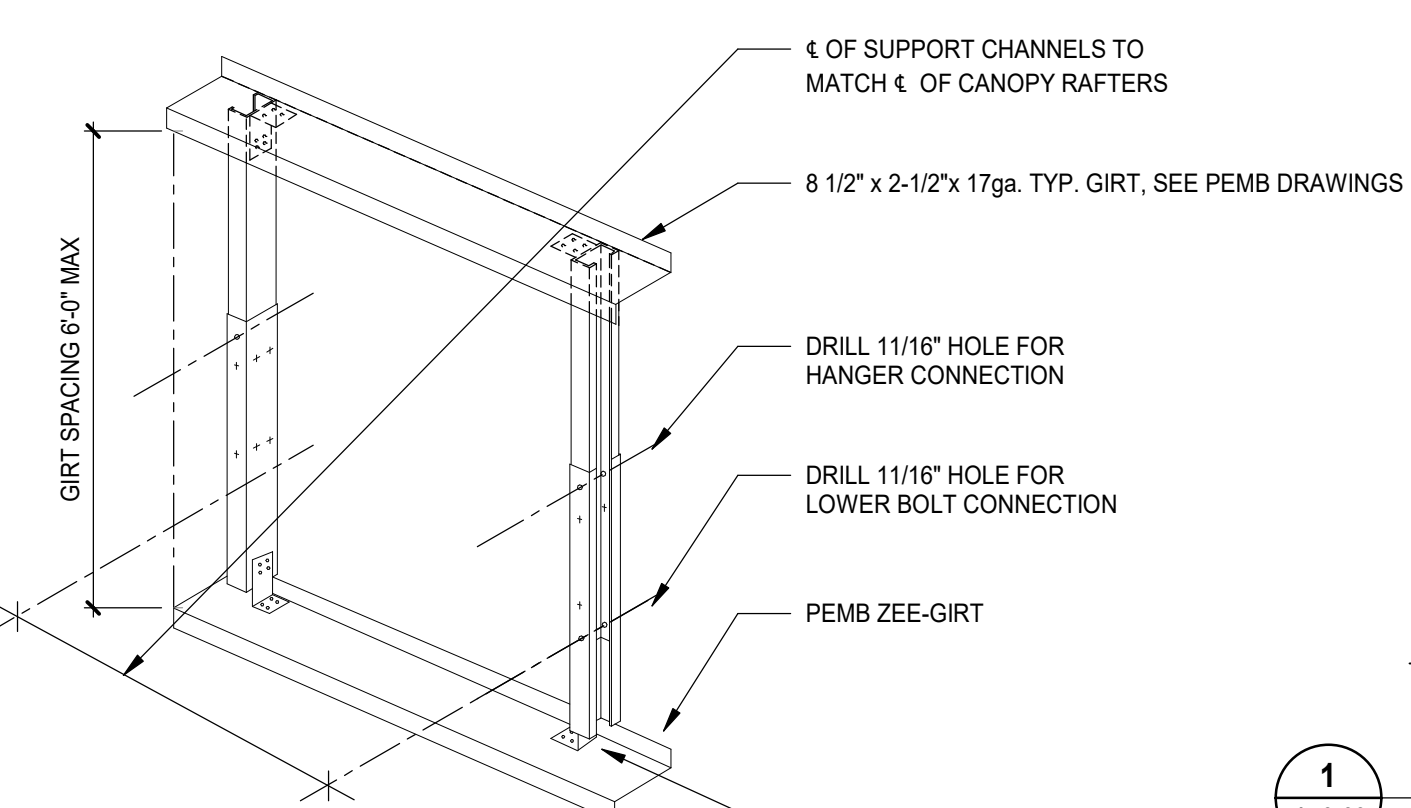
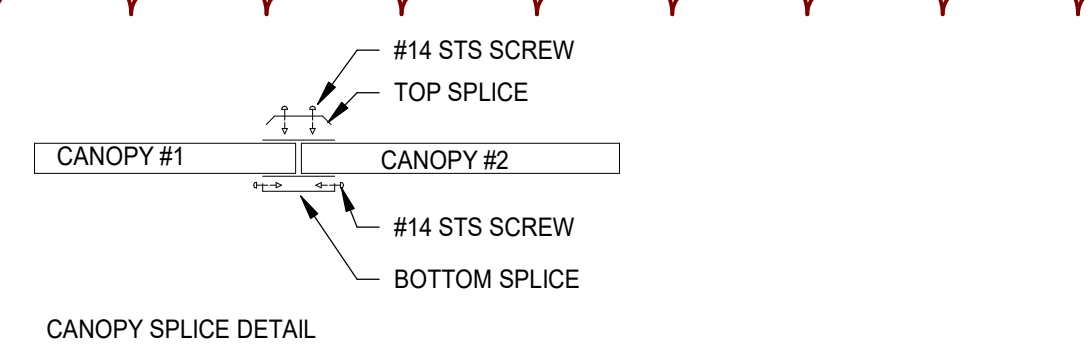
6 ROOF PRESCRIPTIVE ENVELOP REQUIREMENTS
SCALE: 1/2" = 1'-0"



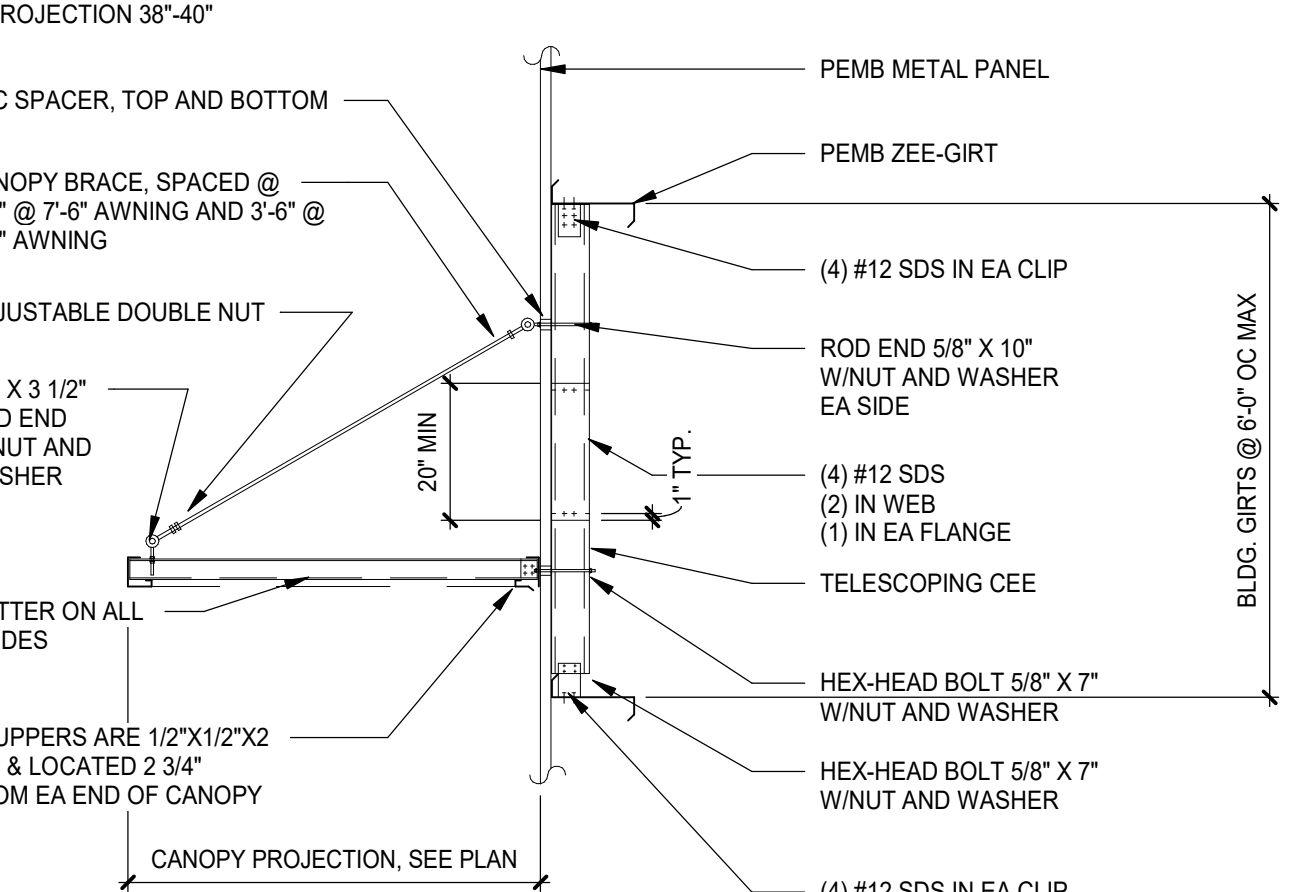
8 EXTERIOR WALL PRESCRIPTIVE REQUIREMENTS
SCALE: 1/2" = 1'-0"



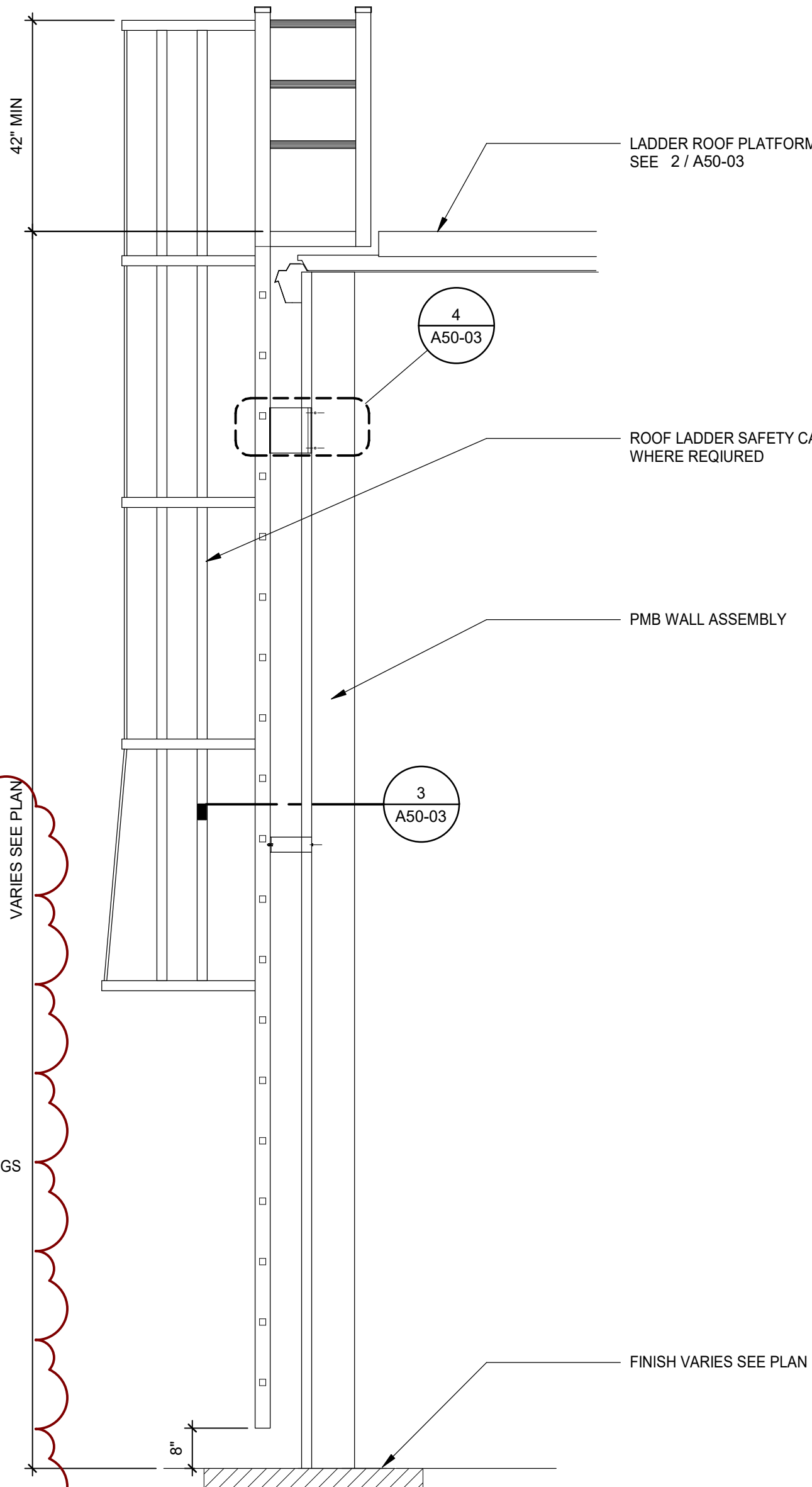
4 LADDER BRACKET SIDE VIEW
SCALE: 3" = 1'-0"



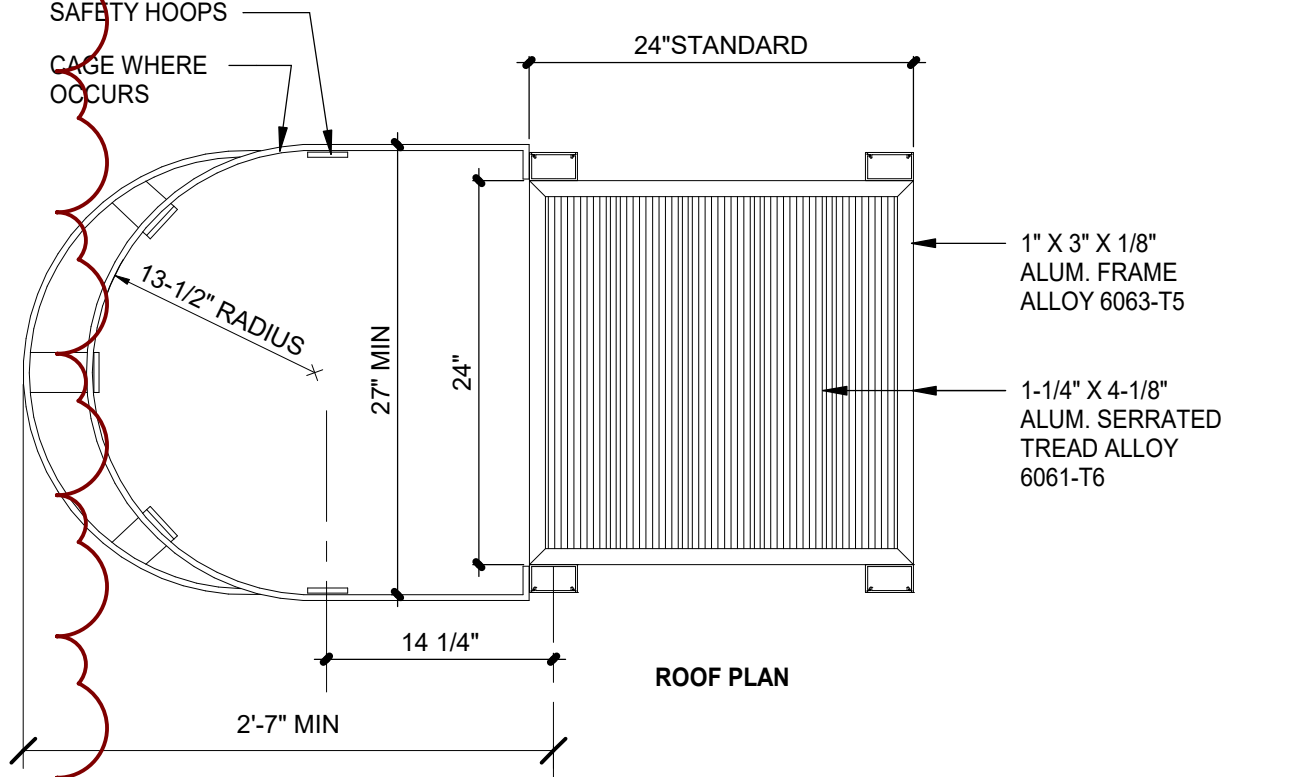
1 LADDER SIDE ELEVATION
SCALE: 1/2" = 1'-0"



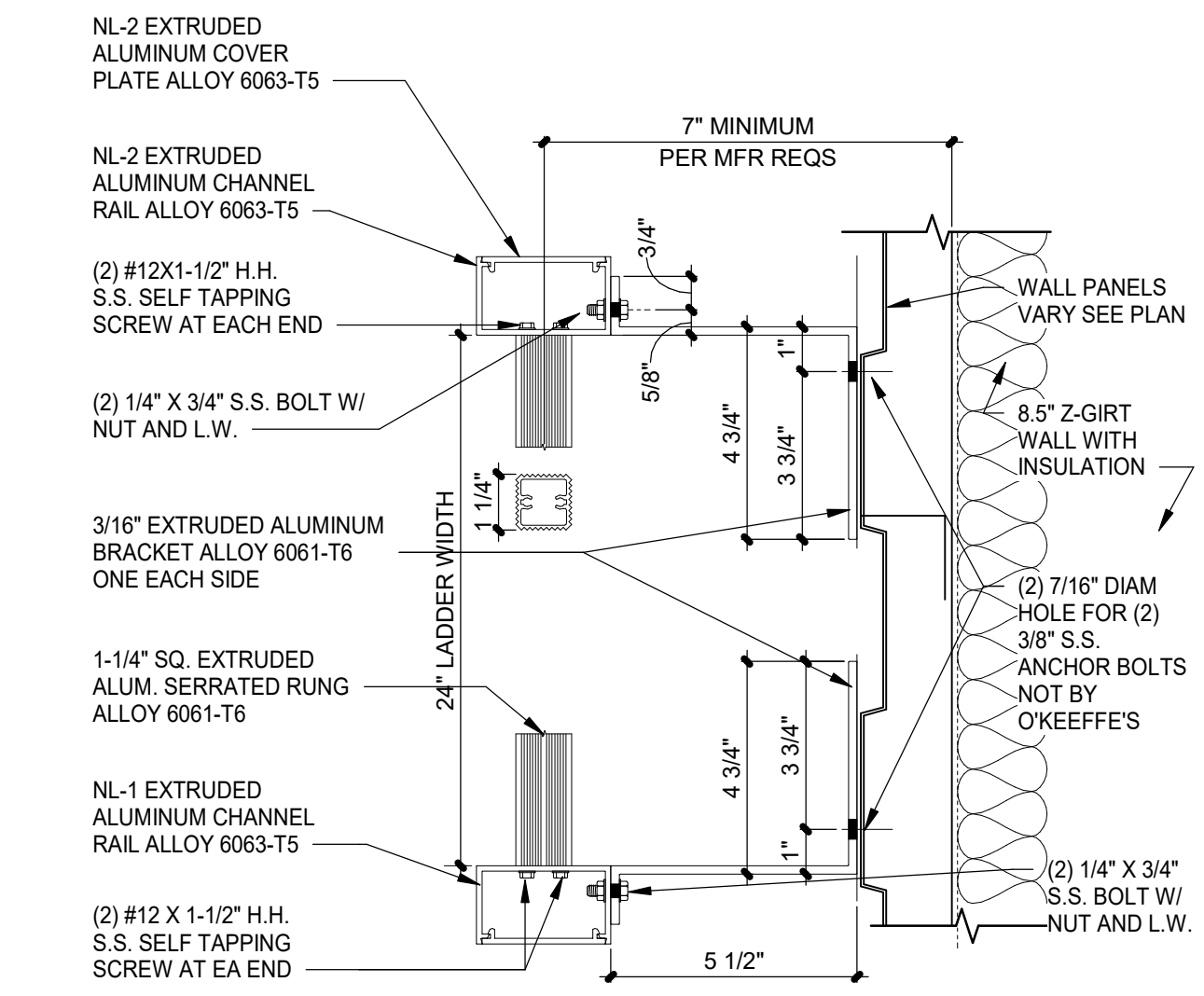
11 METAL CANOPY
SCALE: 3/8" = 1'-0"



1 LADDER ROOF PLATFORM
SCALE: 1" = 1'-0"



2 ROOF PLAN
SCALE: 1" = 1'-0"



3 LADDER BRACKET PLAN VIEW
SCALE: 3" = 1'-0"

DATE	ISSUED FOR	REV
06/26/2026	BID ADDENDUM	3
07/07/26	BID ADDENDUM #3	4

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Project Component

Key Plan

Consultants
Survey: Brandley Engineering
Civil: Kimley-Horn
Architecture: NORR
Structural: Bevier Structural Eng
Mechanical: NORR
Electrical: NORR
Interiors: NORR
Fire Sprinkler: Sacramento Engineering Consultants



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JON PRICE

Project Leader Checked
MIKE NOVAK

Client
MAMMOTH YOSEMITE AIRPORT

Project
MAMMOTH SRE BLUIDING

MAMMOTH, CALIFORNIA
Drawing Title
EXTERIOR DETAILS

Scale
As indicated

Project No.
IN2024-0022

Drawing No.
A50-03