

June 22, 2026

ADDENDUM NO. 4

Interim Fire Station No. 7

Specification No. 2025-010

This addendum is hereby made a part of the contract documents to the same extent as if it were originally included. Contractor shall download the addendum through the QuestCDN bidding site. Failure to download this addendum will negate the Contractor's ability to submit a bid.

BID OPENING DATE

The Bid Opening Date has not changed. Bids are due **June 25th, 2026, at 2:00 p.m.**

DRAWINGS

KPFF Sheets 21–33 have been replaced in their entirety. Sheets 24, 27, 31 and 32 have been revised under Addendum No. 4; all other sheets in the set remain unchanged.

TECHNICAL SPECIFICATIONS:

Section 03 35 23 – Polished Concrete Flooring has been added. See Attachment B of this addendum and the revised Table of Contents in Exhibit A.

The liquidated damages (LD) rates on pages 79 and 80 have been revised, with changes shown in red, to align with the LD rates on page 12. Refer to Exhibit C of this addendum

QUESTIONS AND ANSWERS

Refer to ADDENDUM No. 4. – Question and Answers Log No. 1, attached hereto, for responses to questions received.

By downloading this addendum, Bidder acknowledges receipt of all issued addenda as uploaded to the QuestCDN bidding platform.

ATTACHMENTS

ADDENDUM No. 4. – Question and Answers Log No. 1

EXHIBIT A -Table of Contents

EXHIBIT B - Section 03 35 23 – Polished Concrete Flooring

EXHIBIT C – Revised Liquidated Damages (LD) Rates (Pages 79–80 updated to match Page 12)

KPFF Sheets 21-33 (Revised under Addendum No.4)

A handwritten signature in black ink that reads "Travis Gonsalves". The signature is written in a cursive, flowing style.

Travis Gonsalves
Acting Principal Civil Engineer

ADDENDUM No. 4. – Question and Answers Log No. 1

1-1	The joint sealant specification included in addendum 3 reference locations on the inside and outside of the buildings. Is the GC responsible for any interior joint sealants other than on the Apparatus Bay concrete slab, or any exterior joint sealants on the buildings themselves?	The Contractor is not responsible for an interior or exterior joint sealants, beyond that already specified in Addendum 3.
1-2	There is a lot of concrete flatwork on this job: Is the cost to fill all joints and saw cuts in all concrete flatwork on this site to be included in GC bids? If not, please further specify what concrete flatwork joints are to be filled.	The Contractor is responsible to fill all joints and saw cuts in concrete flat work for the Project.
1-3	No concrete slabs, "rat slabs", are shown under the modular buildings between the footings: Are rat slabs required? If so, please provide specifications, thickness, etc.	The project does not require "rat slabs".
1-4	Beach Strawberry is listed on the Plant Schedule, but not called-out on the plans: Is this to be planted? If so, please specify where...	Refer to Sheet No 91 - the Beach Strawberry is shown over the bioswale on the south side of the Project site.
1-5	Can the irrigation pipes installed on slopes be on the surface?	Refer to Sheet # 93. Irrigation lines are not to be on the surface.
1-6	We are told that the Landscape plans actually scale at about 1':18', not the 1':20' listed. Please review and update scale if required to ensure apples to apples bidding.	The scale on the landscape plans is correct, at 1":20'.
1-7	Please verify if page 26 of the Proposal is required or not - the watermark says "not applicable for online bidding".	Page 26 is not required if online bidding.
1-8	The LD rates listed on page 12 are different than the LD rates listed on page 79, please confirm which are the correct rates.	LD's shown on Page 12 are correct. See Addendum 4, STANDARD SPECIFICATIONS.
1-9	With reference to Sheet 19 of 151 and Addendum 3, items 1-5, the contractor is required to provide service from Alessandro Drive to the telecom room. Sheet 19 specifies eight total 2" low-voltage service conduits entering the living quarters from the southeast side and running southwest toward Alessandro Drive. However, the plans do not indicate where these conduits terminate or connect at Alessandro Drive. Please provide this missing information.	Refer to Sheet 9 UTILITY PLAN. The plan shows that the TELECOM line terminates on the south side of Alessandro Drive to a SPECTRUM Slice Box.

TECHNICAL SPECIFICATION**TABLE OF CONTENTS**

- Structural Foundation
 - Section 031000 Structural Cast-In-Place Concrete Formwork
 - Section 032000 Concrete Reinforcement
 - Section 033000 Cast-In-Place Concrete Structural Foundation
 - Section 079200 Joint Sealants
 - Section 033523 Polished Concrete Flooring**
- Seismic Details
 - Section 079513 Expansion and Seismic Joint Cover
- Plumbing
 - Section 211313 Wet-Pipe Fire Sprinkler System
 - Section 220000 Plumbing
 - Section 221113 Facility Water Sewer And Storm Water Service Piping
 - Section 221323 Sewer Clarifier
 - Section 223313 Electric Commercial Water Heater
- Mechanical
 - Section 230000 Heating, Ventilating, And Air Conditioning
 - Section 230013 General Mechanical Requirements
 - Section 232516 Straight Rail Vehicle Exhaust Removal System
- Communications
 - Section 078413 Penetration Firestopping
 - Section 270528 Pathways For Communications Systems
 - Section 270526 Grounding And Bonding For Communications Systems
 - Section 270536 Cable Trays For Communications Systems
 - Section 271500 Communications Horizontal Cablings
 - Section 270544 Sleeves And Sleeve Seals For Communications Pathways And Cabling
- Fire Alarm System
 - Section 283111 - Digital, Addressable Fire-Alarm System
- Fire Alerting Dispatching System

Section 275000 Distributed Communication and Monitoring System

Electrical

Section 013300 Submittal Procedures

Section 017700 Closeout Procedures

Section 260500 Common Work Results For Electrical

Section 260503 Utility Coordination

Section 260521 Low Voltage Wire Connections

Section 260526 Grounding And Bonding

Section 260529 Hangers And Supports

Section 260533 Conduits

Section 260534 Boxes

Section 260553 Identification For Electrical Systems

Section 260573 Electrical Power System Studies

Section 260850 Field Electrical Acceptance Tests

Section 262214 Dry-Type Transformers

Section 262413 Switchboards

Section 262416 Panelboards

Section 262726 Wiring Devices

Section 262801 Low Voltage Molded Case Circuit Breakers

Section 263214 Diesel Generator Sets

Section 263624 Automatic Transfer Switches

Section 264314 Surge Protective Devices

• LIGHT

Section 260145 Lighting Control Devices

Section 265613 Lighting Pole And Standard

Section 265619 LED Exterior Lighting

SECTION 03 35 23**POLISHED CONCRETE FLOORING****PART 1 - GENERAL****1.01 GENERAL REQUIREMENTS**

- A. Section 923 Concrete Structural Foundation

1.02 SCOPE

- A. The work encompassed by this Section includes performing all operations and furnishing all labor, materials, tools, equipment, and incidentals as necessary to provide Polished Concrete Flooring on floors as indicated on the Room Finish Schedule.
- B. Work to be performed:
 - 1. Dry diamond grinding and polishing of concrete floors.
 - 2. Applying densifying impregnator, sealer and dust-proofing chemicals.
 - 3. Exposing aggregate to approved level.
 - 4. Polishing to specified sheen level.

1.03 RELATED DOCUMENTS

- A. Cast-In-Place Concrete: Section 03 30 00.

1.04 SUMMARY

- A. This section includes the following:
 - 1. Polished Concrete Flooring

1.05 ACTION SUBMITTALS

- A. Color charts for initial color selection.
- B. Samples of color for final selection.
- C. On-Site Mock-Up

1.06 INFORMATIONAL SUBMITTALS

- A. Installer's Project References: Submit installer's list of success fully completed polished concrete floor system projects, including project name, location, client and

project architect. Define the type of project, quantity of area polished concrete flooring and system used.

- B. Project Record Drawings
- C. Product Data for each product specified including the following: Test Reports: Provide certified test reports, prepared by an independent testing laboratory, confirming compliance with specified performance criteria.
 1. Submit special concrete finishes manufacturer's specifications and test data.
 2. Submit special concrete finishes describing product to be provided, giving manufacturer's name and product name for the specified material proposed to be provided under this section.
 3. Submit special concrete finishes manufacturer's recommended installation procedures; which when approved by Engineer, will become the basis for accepting or rejecting actual installation procedure used on the work.
 4. Submit special concrete finishes technical data sheet giving descriptive data, curing time, and application requirements.
 5. Submit special concrete finishes manufacturer's Material Safety Data Sheet (MSDS) and other safety requirements.
 6. Follow all special concrete finishes published manufacturer's installation instructions.
- D. Installation instructions
- E. Maintenance Data: Submit installer's maintenance manual, including maintenance and cleaning instructions for polished concrete floor system.

1.07 QUALITY ASSURANCE

- A. American Society for Testing and Materials:
 1. ASTM C642 Standard Test Method for Density, Absorption, and Voids in Hardened Concrete.
 2. ASTM D5178 Standard Test Method for Mar Resistance of Organic Coatings.
 3. ASTM D4060 Standard Test Method for Abrasion Resistance of Organic Coatings by the Taber Abraser:
 4. ASTM G15 Standard Practice for Operating Fluorescent Ultraviolet (UV) LAMP Apparatus for Exposure of Nonmetallic Materials.
 5. ASTM D4541 Standard Test Method for Pull-Off Strength of Coatings Using Portable Adhesion Testers.

- | | | |
|-----|------------|--|
| 6. | ASTM D2369 | Standard Test Method for Volatile Content of Coatings. |
| 7. | ASTM D2047 | Standard Test Method for Static Coefficient of Friction of Polish-Coated Flooring Surfaces as Measured by the James Machine. Concrete used as finish floor is required to be slip-resistant (per 11B-302.1). |
| 8. | ASTM C1378 | Standard Test Method for Determination of Resistance to Staining. |
| 9. | ASTM D2047 | Standard Test Method for Static Coefficient of Friction of Polish-Coated Flooring Surfaces as Measured by the James Machine. Concrete used as finish floor is required to be slip-resistant (per 11B-302.1). |
| 10. | ASTM C150 | Standard Specification for Portland Cement. |
| 11. | ASTM C33 | Standard Specification for Concrete Aggregates. |
| 12. | ASTM D523 | Standard Test Method for Specular Gloss. |
| 13. | ASTM D1455 | Standard Test Method for 60° Specular Gloss of Emulsion Floor Polish. |
| 14. | ASTM E1155 | Standard Test Method for Determining FF Floor Flatness and FL Floor Levelness Numbers. |
- B. American Concrete Institute
1. ACI 302.1R, Guide to Concrete Floor and Slab Construction
- C. Reflectivity according to use of Horiba IG-320 Gloss Checker. Installer Qualifications:
1. Use an experienced installer and adequate number of skilled workmen which are thoroughly trained and experienced in the necessary craft.
 2. The special concrete finish manufacturer shall certify applicator.
 3. Applicator shall be familiar with the specified requirements and the methods needed for proper performance of work of this section.
- D. Manufacturer's Certification:
1. Provide letter of certification from concrete finish manufacturer stating that installer is certified applicator of special concrete finishes, and is familiar with proper procedures and installation requirements required by the manufacturer.
- E. Mock-ups:

1. Apply mock-ups of each type finish, to demonstrate typical joints, surface finish, color variation (if any) crack repair, and standard of workmanship.
2. Notify Engineer or City Representative seven days in advance of dates and times when mockups will be constructed.
3. Obtain from the Engineer or City Representative approval of mock-ups before starting construction.
4. If the Engineer or City Representative determines that mock-ups do not meet requirements, demolish and remove them from the site if instructed to do so and cast others until mock-ups are approved.
5. Maintain mock-ups during construction in an undisturbed condition as a standard for judging the completed work.
6. State approved mock-ups may become part of the completed work if undisturbed at time of acceptance of the work.

F. Protection

1. No satisfactory chemical or cleaning procedure is available to remove petroleum stains from the concrete surface. Prevention is therefore essential.
2. All hydraulic powered equipment must be diapered to avoid staining of the concrete.
3. No trade will park vehicles on the inside slab. If necessary to complete their scope of work, drop cloths shall be placed under vehicles at all times.
4. No pipe cutting machine shall be used on the inside floor slab.
5. Steel shall not be placed on interior slab to avoid rust staining.
6. Acids and acidic detergents shall not come into contact with slab
7. All trades shall be informed that the slab must be protected at all times.
8. All equipment must be equipped with non-marking tires.
9. Provide protective covering such as "SKUDO" or equivalent.

G. Pre-Installation Conference:

1. Conduct conference at project site to comply with requirements in Division 1 Section " Project Management and Coordination"
2. Notes of conference will be distributed to all attendees.

1.08 DELIVERY, STORAGE AND HANDLING

- A. Delivery: Deliver materials to site in manufacturer's original, unopened containers and packaging, with labels clearly identifying product name and manufacturer.
- B. Storage: Store materials in clean, dry area indoors in accordance with manufacturer's instructions. Keep materials from freezing.
- C. Handling: Protect materials during handling and application to prevent contamination or damage.

PART 2 - PRODUCTS

2.01 SYSTEM DESCRIPTION

- A. Polished Concrete: Includes grinding installation of silicate sealer, (hardener, densifier), polishing, and a stain repellent.
- B. Performance Criteria
 - 1. ASTM C642 Absorbability: Reduction of 75% of Control.
 - 2. ASTM D5178 Balance Beam Mar Tester: Greater than 50% harder.
 - 3. ASTM D2486e1 Abrasive Scrub: 1200 Cycles.
 - 4. ASTM D4060 Modified Taber Abrasion 600 Rev: 0.37% treated vs. 0.68% untreated.
 - 5. ASTM G154: 5000 HR QUV: No fade, change or erosion.
 - 6. ASTM D4541 Bonding: Greater than 50 psi.
 - 7. ASTM D2369 Solids: 18% Min.
 - 8. Reflectivity: Change in gloss to 30, 60 or 80 depending on Certi-Shine system, as measured using a gloss meter in accordance with Horiba IG-320 Gloss Checker.
 - 9. ASTM C137 Stain resistance: Food, Chemical, Oil and common stain resistance. See manufacturer's literature for list.

PART 3 - EXECUTION

3.01 PROJECT CONDITION

- A. Floor Finish:
 - 1. Slabs and flatwork shall be placed and finished monolithically.
 - 2. Strike off and laser screed slabs to true, plane surfaces at required elevations.

3. Thoroughly compact concrete with vibrators, floats, and tampers to force coarse aggregate below the surface.
4. Power trowel with no hand finishing.
5. Pan float.
6. Steel finish.
7. Surface should not be burned due to excessive troweling.
8. Imprints are not acceptable (i.e. boots, foreign objects dropped into concrete).

B. Floor and Joints:

1. Free of debris and excessive dirt, dust, clay, and mud.
2. Dry.

C. Floor Surface Profile:

1. Floor Flatness Number (FF): 50 (preferred) 45 (minimum).
2. Floor Levelness Number (FL): 35 (preferred) 30 (minimum).

D. Concrete Compressive Strength: As noted on construction documents.

E. Concrete Curing: Minimum 8 days water cured or dissipating curing compound applied.

F. Concrete Adjacent to Floor Penetrations: Troweled flat and level with surrounding concrete.

G. Concrete Adjacent to Drains, clean-outs, etc: Finish level to the top of the structure.

3.02 SURFACE PREPERATION

A. Protect surrounding areas and adjacent surfaces from the following:

1. Minimal accumulation of dust from grinding and polishing
2. Contact with overspray of concrete densifier.
3. Contact with overspray of concrete sealer.

B. Prepare surfaces in accordance with installer's instructions.

C. Clean Surfaces: Remove dirt, dust, debris, oil, grease, curing agents, bond breakers, paint, coatings, and other surface contaminants which could adversely affect installation of polished concrete floor system.

3.03 INSTALLATION

- A. Install polished concrete floor system in accordance with installer's instructions.
- B. Start floor finish applications in presence of manufacturer's technical representative if practical.
- C. Aggregate Exposure: Small Aggregate: Mottled salt-and-pepper course aggregate exposure.
- D. Polished Concrete Finish: Level 3 – High Gloss.

3.04 APPLICATION

- A. Level floor by grinding with 40-grit metal-bonded diamonds (Note: The exact number of grinding and polishing steps required will be determined by the flatness achieved by the concrete finisher, along with the desired look that is specified).
- B. Prepare concrete to accept densifier by grinding with 80-grit metal-bonded diamonds.
 - 1. Apply concrete densifier to deeply saturate floor.
 - 2. Remove residue of concrete densifier dried on floor surface by grinding with 150-grit metal-bonded diamonds.
- C. Polishing
 - 1. Remove 150-grit metal-bonded diamond scratches by grinding with 100-grit resin-bonded diamonds.
 - 2. Remove 150-grit metal-bonded and 100-grit resin-bonded diamond scratches by grinding with 200-grit resin-bonded diamonds.
 - 3. Prepare floor for polishing by grinding with 400-grit resin bonded diamonds.
 - 4. Achieve light-reflective finish when viewed from a distance of 30 feet by grinding with 800-grit resin-bonded diamond.
- D. Sealing, Hardening and Polishing of Concrete Surface
 - 1. Concrete must be in place a minimum of 28 days or as directed by the manufacturer before application can begin.
 - 2. Polish to pre-determined level based on test sample.
 - 3. Application is to take place at least 10 days prior to racking and other in-store accessory installation, thus providing a complete, uninhibited concrete slab for application.
 - 4. Applicable procedures must be followed as recommended by the product manufacturer and as required to match approved test sample.

5. Achieve hardening, dust-proofing, and abrasion resistance of the surface without changing the natural appearance of the concrete, except for the sheen.
6. Finish to within 3" of vertical surfaces where practical.

3.05 QUALITY CONTROL

- A. Inspect completed polished concrete floor system with Engineer or City Representatives.
- B. Review procedures with Engineer to correct unacceptable areas of completed polished concrete floor system.
- C. Testing: Test the following from completed polished concrete floor system:
 1. Static Coefficient of Friction:
 - a. Dry surface.
 - b. Wet surface.
- D. Specular Gloss/Reflectance (Test Test Method for Specular Gloss of Emulsion Floor Polish):
 1. 20 degrees.
 2. 60 degrees.
- E. Floor Surface Profiles:
 1. Floor Flatness Number (FF).
 2. Floor Levelness Number (FL).
- F. Test Results:
 1. Report test results in writing to Engineer or City Representative within 24 hours after tests.
 2. Compare test results from tests performed before and after installation of polished concrete floor system.

3.06 PROTECTION

- A. Protect completed polished concrete floor system from damage until acceptance of the work.
 1. Do not allow vehicle and pedestrian traffic on unprotected floor.
 2. Do not allow construction materials, equipment, and tools on unprotected floor.

- B. Immediately remove mortar splatter, spilled liquids, oil, grease, paint, coatings, and other surface contaminants which could adversely affect completed polished concrete floor system.
- C. Repair damaged areas of completed polished concrete floor system to satisfaction of Engineer or City Representative.

END OF SECTION

damages as are herein provided, and in case the same are not paid, agrees that the City may deduct the amount thereof from any money due or that may become due to the Contractor under the contract.

6-9.2 Failure to Keep Traffic Lanes Open, Coordinate Signal Turn-ons, Striping and Signing, and Signal System Disruption

If traffic lanes are not kept open for public use, on the days, at the times, and in the manner specified in the Special Provisions of the Specifications, and the City approved traffic control plans, damage will be sustained by the City and its residents. Damage will also be sustained by the City and its residents if signal operations, roadway striping, and signing are not coordinated and completed. Since it is, and will be, impracticable to determine the actual damage which the City and its provisions of the Special Provisions of the Specifications and the City approved traffic control plans, City and Contractor agree that Contractor will pay to City, not as penalty, but as predetermined liquidated damages, \$750 per day for each of the following items (except for 6-9.2(a) which is \$62.5 per hour per lane), for any period of time during the day in which the infraction occurs:

- (a) Contractor fails to keep open any traffic lane for public use as required by the Special Provisions of the Specifications and the City approved traffic control plans; or
- (b) Contractor fails to temporarily stripe the roadway prior to opening up the roadway for normal vehicular travel immediately following resurfacing; or
- (c) Contractor fails to permanently restripe the roadway and remove any conflicting striping or delineation within seven (7) calendar days following resurfacing of that section of roadway. At the discretion of the Engineer, temporary paint may be installed prior to permanent striping so that traffic signal detection or miscellaneous roadway work items, including the raising of utility covers, may be completed. Temporary striping shall be in place for no longer than fourteen (14) calendar days; at which time the Contractor will have an additional seven (7) calendar days to completely install permanent striping; or
- (d) Contractor fails to remove signs which conflict with new striping or fails to install signs required by the Contract Documents which are necessary for safe traffic movement; or
- (e) Contractor fails to install traffic signal detection within fourteen (14) calendar days following the completion of a resurfacing project. Traffic signal detection will be considered installed when the detector actuates the traffic signal for an arriving vehicle; or
- (f) Contractor fails to complete striping, install traffic signal detection and/or install required signs prior to a traffic signal being turned on.

A traffic signal shall not be turned on until roadway striping, signing and traffic signal detection has been completed; or

- (g) Contractor fails to replace interconnect that has been removed either inadvertently or as part of the Contract Documents. Liquidated damages will become effective one day following the inadvertent removal of interconnect or one day after the Engineer/Contractor agreed upon time limit for a planned removal of interconnect.

The Contractor agrees to pay such liquidated damages as are provided for in this paragraph, and in case the same are not paid, Contractor agrees that City may deduct the amount of such liquidated damages from any money that is due or that may be due the Contractor under the contract.

6-9.3 Failure to Replace Temporary Resurfacing within 30 Calendar Days

If temporary resurfacing is not replaced with permanent pavement resurfacing within 30 calendar days of beginning demolition work on a street, if required in the Contract Documents, City will sustain damages. Since it is and will be impracticable to determine what actual damages the City will sustain as a result of the Contractor failing to comply with the Contract Documents, the Contractor agrees to pay City the sum of **\$750** per calendar day per City block for each and every calendar day that a street does not have temporary resurfacing replaced with permanent resurfacing as required by the Contract Documents. This sum is not a penalty, but predetermined liquidated damages. The Contractor agrees to pay such liquidated damages as are provided for in this paragraph, and in case the same are not paid, Contractor agrees that City may deduct such liquidated damages from any money that is due or may become due to the Contractor under the Contract.

6-9.4 Failure to Reestablish Water Service

If a disrupted water service is not reestablished within the designated period allowed for in the Specifications, the City will sustain damages. Since it is and will be impracticable to determine what actual damages the City will sustain as a result of the Contractor failing to comply with the Contract Documents, the Contractor agrees to pay City the sum of **\$750** per hour that one or more water services are not pressurized and ready to provide safe drinking water. This sum is not a penalty, but predetermined liquidated damages. The Contractor agrees to pay such liquidated damages as are provided for in this paragraph, and in case the same are not paid, Contractor agrees that City may deduct such liquidated damages from any money that is due or may become due to the Contractor under the Contract.

6-10 DISPUTES AND CLAIMS; PROCEDURE

SYMBOLS

	SECTION REFERENCE BUBBLE
	DETAIL REFERENCE BUBBLE WITH ARROW
	DETAIL REFERENCE BUBBLE
	FULL HEIGHT SECTION INDICATOR
	BUILDING SECTION INDICATOR
	ELEVATION OF WALL OR FRAME
	NORTH ARROW
	SLOPE
	EARTH LAYER
	STEPPED SURFACE; FLOOR DEPRESSION
	SLOPED SURFACE
	INDICATES SAND OR GROUT
	INDICATES GRAVEL
	WELDED WIRE FABRIC (WWF LAYER)
	STEEL TUBE COLUMN
	STEEL PIPE COLUMN
	WIDE FLANGE STEEL COLUMN
	MEMBER SPLICE
	STEEL IN CROSS SECTION
	DIRECTION OF SPAN

ABBREVIATIONS

SCHED	SCHEDULE	GA	GAUGE	AB	ANCHOR BOLT
SECT	SECTION	GALV	GALVANIZED	ACI	AMERICAN CONCRETE INSTITUTE
SEP	SEPERATION	GB	GRADE BEAM	ADDL	ADDITIONAL
SHT	SHEET	GLB	GLUED LAMINATED BEAM	ADJ	ADJACENT
SHTG	SHEATHING	GR	GRADE	AESS	ARCHITECTURAL EXPOSED STRUCTURAL STEEL
SIM	SIMILAR	GRND	GROUND	AGGR	AGGREGATE
SLBB	SHORT LEGS BACK-TO-BACK	H or HORIZ	HORIZONTAL	AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
SOG	SLAB ON GRADE	HDR	HEADER	ALT	ALTERNATE
SPCG	SPACING	HGR	HANGER	ALUM	ALUMINUM
SPECS	SPECIFICATIONS	HOSP	HOSPITAL	ANCH	ANCHOR
SPCL	SPECIAL	HP	HIGH POINT	ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE
SQ	SQUARE	HS	HIGH STRENGTH	APA	AMERICAN PLYWOOD ASSOCIATION
SS	SELECT STRUCTURAL	HSB	HORIZONTALLY SLOTTED HOLES	APPVD	APPROVED
SSL	SHORT SLOTTED HOLES	HT	HEIGHT	APPROX	APPROXIMATE
STAGG	STAGGER	HR	HARD ROCK	ARCH	ARCHITECTURAL; ARCHITECT
STD	STANDARD	ID	INSIDE DIAMETER	ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
STIFF	STIFFENERS	IF	INSIDE FACE	AWPA	AMERICAN WOOD PRESERVERS ASSOCIATION
STIRR	STIRRUP	I-JST	I-JOIST	AWS	AMERICAN WELDING SOCIETY
STL	STEEL	IN	INCH	AITC	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION
STRUCT	STRUCTURAL	INCL	INCLUDE	ASTM	AMERICAN SOCIETY FOR TESTING MATERIALS
STRUCT I	STRUCTURAL I	INFO	INFORMATION	&	AND
SW	SHEAR WALL	INSP	INSPECTION	@	AT
SYM	SYMMETRICAL	INT	INTERIOR	BLDG	BUILDING
TB	TIE BEAM	JST	JOIST	BLK	BLOCK
T&B	TOP AND BOTTOM	JT	JOINT	BLKG	BLOCKING
T&G	TONGUE & GROOVE	K	KIPS	BM	BEAM
		KSI	KIPS PER SQUARE INCH	BN	BOUNDARY NAIL
TOC	TOP OF CONCRETE	LAB	LABORATORY	BNDRY	BOUNDARY
TOF	TOP OF FOOTING	LB(S) OR #	POUND(S)	BOT OR B	BOTTOM
TEMP	TEMPERATURE; TEMPORARY	LF	LINEAL FOOT	BRC	BRACE
THRU	THROUGH	LIN	LINEAL; LINEAR	BRG	BEARING
THK	THICKNESS/THICK	LLBB	LONG LEGS BACK-TO-BACK	BT	BENT
THR	THREADED	LLH	LONG LEG HORIZONTAL	BTWN	BETWEEN
TOP or T	TOP	LLV	LONG LEG VERTICAL	CANT	CANTILEVER
TOS	TOP OF STEEL	LP	LOW POINT	CAM OR C	CAMBER
TOW	TOP OF WALL	LSL	LONG SLOTTED HOLES	CC	CENTER TO CENTER
TSG	TAPERED STEEL GIRDER	LT WT	LIGHTWEIGHT	CG	CENTER OF GRAVITY
TYP	TYPICAL	LVL	LEVEL	CIP	CAST-IN-PLACE
UBC	UNIFORM BUILDING CODE	MAS	MASONRY	CJ	CONSTRUCTION JOINT; CONTROL JOINT
UNO	UNLESS NOTED OTHERWISE	MATL	MATERIAL	CL	CENTER LINE
UT	ULTRA-SONIC TEST	MAX	MAXIMUM	CLR	CLEARANCE; CLEAR
VERT	VERTICAL	MB	MACHINE BOLT	CMU	CONCRETE MASONRY UNIT
VSH	VERTICAL SLOTTED HOLES	MC	MISCELLANEOUS CHANNEL SHAPE	COL	COLUMN
W/	WITH	MECH	MECHANICAL	COMP	COMPRESSION
W/O	WITHOUT	MFR	MANUFACTURER	CONC	CONCRETE
WD	WOOD	MIN	MINIMUM; MINUTE	CONN	CONNECTION; CONNECT
WP	WORK POINT; WATERPROOF	MISC	MISCELLANEOUS	CONSTR	CONSTRUCTION
WT	WEIGHT	(N)	NEW	CONT	CONTINUE; CONTINUOUS
WWF	WELDED WIRE FABRIC	N	NORTH	CONTR	CONTRACTOR
		NF	NEAR FACE	CJP	COMPLETE JOINT PENETRATION WELD
		NIC	NOT IN CONTRACT	CTR	CENTER
		NORM	NORMAL	CTSK	COUNTERSINK; COUNTERSUNK
		NO or #	NUMBER	CU FT	CUBIC FOOT
		NS	NEAR SIDE	d	PENNY (NAIL OR BAR DIA)
		NTS	NOT TO SCALE	DBL	DOUBLE
		OC	ON CENTER	DEPT	DEPARTMENT
		OD	OUTSIDE DIAMETER	DET	DETAIL
		OF	OUTSIDE FACE	DF	DOUGLAS FIR/LARCH
		OH	OPPOSITE HAND	DIA OR ø	DIAMETER
		OPNG	OPENING	DIAG	DIAGONAL
		OPP	OPPOSITE	DIAPH	DIAPHRAGM
		ORIG	ORIGINAL	DIM	DIMENSION
		OSB	ORIENTED STRAND BOARD	DN	DOWN
		PARA OR //	PARALLEL	DO	DITTO (REPEAT)
		PC	PRECAST; PIECE	DWG	DRAWING
		PERP	PERPENDICULAR	DWL	DOWEL
		PI	PLYWOOD INDEX	EA	EACH
		PL	PLATE	EF	EACH FACE
		ℙ	PROPERTY LINE	EJ	EXPANSION JOINT
		PLF	POUNDS PER LINEAL FOOT	EL	ELEVATION
		PLCS	PLACES-	ELEC	ELECTRICAL
		PLY	PLYWOOD	ELEV	ELEVATOR
		PROP	PROPERTY	EMBED	EMBEDMENT
		PT	POST TENSIONED	EN	EDGE NAIL
		PW	PLATE WASHER	ENGR	ENGINEER
		PJP	PARTIAL JOINT PENETRATION WELD	EQ	EQUAL OR EQUIVALENT
		PREFAB	PREFABRICATED	EQUIP	EQUIPMENT
		PSF	POUNDS PER SQUARE FOOT	ES	EACH SIDE
		PSI	POUNDS PER SQUARE INCH	ETC	ET CETERA
		PVC	POLYVINYL CHLORIDE	EW	EACH WAY
		PVMT	PAVEMENT	EXIST or (E)	EXISTING
		#	POUND; NUMBER	EXT	EXTERIOR
		REF	REFERENCE	FDN	FOUNDATION
		REINF	REINFORCE; REINFORCING	FF	FAR FACE
		REQD	REQUIRED	FF	FINISHED FLOOR
		RF	ROOF	FIN	FINISH
		ø	DIAMETER	FJ	FLOOR JOIST
				FL	FLOOR LINE
				FLG	FLANGE
				FLR	FLOOR
				FN	FIELD NAIL
				FOC	FACE OF CONCRETE
				FOM	FACE OF MASONRY
				FOS	FACE OF STUD
				FOW	FACE OF WALL
				FP	FULL PENETRATION; FIRE PROOFING
				FRMG	FRAMING
				FS	FULL SIZE; FAR SIDE
				FT	FOOT; FEET
				FTG	FOOTING

SHEET INDEX

SHEET NUMBER	SHEET NAME
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REV.	DESCRIPTION	CKD	APP	DATE
4	ADDENDUM NO. 4			6/22/26
3	PLAN CHECK 3			4/17/26
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PUBLIC WORKS DEPARTMENT		ENGINEERING DIVISION	
CITY OF SAN BUENAVENTURA			
INTERIM FIRE STATION NO. 7			
SHEET LIST, ABBREVIATIONS AND SYMBOLS			
DRN. BY:	CC	DES. BY:	KK
CKD. BY:	RT		
PRINCIPAL ENGINEER		DATE	
CITY ENGINEER		DATE	
SPEC. NUMBER	2025-010	PROJECT NO.	FA11-1061
SEPTEMBER 7, 2025	SHEET 21 OF 151	DWG. NO.	2025-D-010

CONCRETE

- ALL CONCRETE CONSTRUCTION SHALL CONFORM WITH CHAPTER 19 OF THE CODE AND WITH THE PROVISIONS OF ACI 318, LATEST EDITION.
- REINFORCED CONCRETE IS DESIGNED BY THE "ULTIMATE STRENGTH DESIGN METHOD".
- CONCRETE MIXTURES SHALL BE DESIGNED BY THE APPROVED TESTING LABORATORY AND REVIEWED BY THE STRUCTURAL ENGINEER. THE COMPRESSIVE STRENGTH OF THE CONCRETE SHALL BE PROPORTIONED BASED AS INDICATED ON ACI 318-19.
- SCHEDULE OF STRUCTURAL CONCRETE 28-DAY STRENGTH AND TYPES

LOCATIONS IN STRUCTURE	STRENGTH (PSI)	DENSITY (PCF)	MAX W/C RATIO
FOOTINGS AND FOUNDATION WALLS	4,000	145	0.45

- PROVIDE CONCRETE MIXTURES THAT MEET THE DURABILITY REQUIREMENTS INDICATED IN THE GEOTECHNICAL REPORT AND PROJECT SPECIFICATIONS.
- CONCRETE SHRINKAGE SHALL BE LIMITED TO 0.05 PERCENT PER ASTM C157.
- CONCRETE SHALL BE MAINTAINED ABOVE 50 DEGREES FAHRENHEIT AND IN MOIST CONDITION FOR A MINIMUM OF 7 DAYS AFTER PLACEMENT.
- PORTLAND CEMENT SHALL CONFORM TO ASTM C-150, TYPE II, OR AS REQUIRED BY THE GEOTECHNICAL REPORT AND PROJECT SPECIFICATIONS. PROVIDE TYPE V WHERE CONCRETE IS IN CONTACT WITH CORROSIVE SOIL.
- AGGREGATE FOR HARDROCK CONCRETE SHALL CONFORM TO ALL REQUIREMENTS AND TESTS OF ASTM C-33 AND PROJECT SPECIFICATIONS.
- CONCRETE MIXING OPERATIONS, ETC. SHALL CONFORM TO ASTM C-94..
- PLACEMENT OF CONCRETE SHALL CONFORM TO THE CODE CHAPTER 19 AND PROJECT SPECIFICATIONS. CLEAN AND ROUGHEN TO 1/4" AMPLITUDE ALL CONCRETE SURFACES AGAINST WHICH NEW CONCRETE IS TO BE PLACED UNLESS NOTED OTHERWISE.
- ALL REINFORCING BARS, ANCHOR BOLTS AND OTHER CONCRETE INSERTS, INCLUDING PIPES AND CONDUITS, SHALL BE SECURED IN POSITION PRIOR TO PLACING CONCRETE.
- CORING IN CONCRETE IS NOT PERMITTED WITHOUT ENGINEER'S REVIEW AND APPROVAL. NOTIFY THE STRUCTURAL ENGINEER IN ADVANCE OF CONDITIONS NOT SHOWN ON THE DRAWINGS.
- CONDUITS LARGER THAN 1-1/2" DIAMETER MAY BE EMBEDDED IN STRUCTURAL CONCRETE WHERE SPECIFICALLY APPROVED BY STRUCTURAL ENGINEER. UNLESS DETAILED OTHERWISE, EMBEDDED CONDUIT SHALL CONFORM TO THE FOLLOWING:
 - CONDUITS SHALL NOT DISPLACE OR INTERRUPT REINFORCING BARS.
 - DO NOT STACK AND/OR CLUSTER CONDUITS.
 - SPACE EMBEDDED CONDUITS A MINIMUM OF 3 DIAMETERS CLEAR FROM OTHER EMBEDDED CONDUITS USING THE LARGER CONDUIT DIAMETER.
 - PROVIDE 1-1/2" CLEAR FROM REINFORCING BARS.
 - EMBEDDED CONDUITS SHALL BE PLACED IN THE MIDDLE THIRD OF SLABS, BEAMS, WALLS, FOOTINGS, ETC.
- PIPES ARE NOT PERMITTED TO BE EMBEDDED IN CONCRETE.
- SCREED CONCRETE FILL OVER STEEL DECK TO A CONSTANT THICKNESS AS SPECIFIED IN THE DECKING SCHEDULE.

CONSTRUCTION JOINTS

- ALL CONSTRUCTION JOINTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 19 AND ACI 318 26.5.6 OF THE CODE AND THE TYPICAL CONSTRUCTION JOINT DETAILS SHOWN ON THE STRUCTURAL DRAWINGS.
- ALL SURFACES OF CONSTRUCTION JOINTS SHALL BE CLEANED TO REMOVE DUST, CHIPS, STANDING WATER OR OTHER FOREIGN MATTER PRIOR TO PLACING CONCRETE.
- THE CONTRACTOR SHALL SUBMIT THE PROPOSED LOCATIONS OF CONSTRUCTION JOINTS TO THE STRUCTURAL ENGINEER FOR REVIEW BEFORE STARTING CONSTRUCTION.

REINFORCING STEEL

- REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 19 OF THE CODE, ASTM 706, GRADE 60 (UNO). DEFORMATIONS SHALL BE IN ACCORDANCE WITH ASTM A-305.
- BARS SHALL BE CLEAN OF RUST GREASE OR OTHER MATERIALS LIKELY TO IMPAIR BOND. ALL REINFORCING BAR BENDS SHALL BE MADE COLD.
- DETAIL, FABRICATE, AND INSTALL REINFORCING IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 301, ACI 117, AND THE "CRSI MANUAL OF STANDARD PRACTICE."
- REINFORCING BAR SPLICES SHALL BE MADE AS INDICATED ON THE DRAWINGS. MINIMUM SPLICE LENGTH FOR REINFORCING STEEL BARS IN CONCRETE SHALL BE AS REQUIRED FOR CLASS B SPLICE PER ACI 318 SECTION 25.5.2 UNO. LAP ALL HORIZONTAL BARS AT CORNERS AND INTERSECTIONS.
- ALL BARS SHALL BE MARKED SO THEIR IDENTIFICATION CAN BE MADE WHEN THE FINAL IN-PLACE INSPECTION IS MADE.
- WHERE WELDING OF REINFORCING IS APPROVED BY THE STRUCTURAL ENGINEER, IT SHALL BE DONE BY AWS CERTIFIED WELDERS USING E90XX OR APPROVED ELECTRODES. WELDING PROCEDURES SHALL CONFORM TO THE REQUIREMENTS OF STRUCTURAL WELDING CODE REINFORCING STEEL AWS-D1.4 LATEST REVISION. REINFORCING BARS TO BE WELDED SHALL CONFORM TO THE REQUIREMENTS OF ASTM A-706.
- BARS IN SLABS SHALL BE SECURELY SUPPORTED ON WELL-CURED CONCRETE BLOCKS OR APPROVED METAL CHAIRS, PRIOR TO PLACING CONCRETE.
- COMPLETE AND DETAILED REINFORCING PLACEMENT DRAWINGS SHALL BE PREPARED AND SUBMITTED TO THE ARCHITECT FOR REVIEW BY THE STRUCTURAL ENGINEER PRIOR TO FABRICATION IN ACCORDANCE WITH THE SPECIFICATIONS AND APPLICABLE CODES. THESE DRAWINGS SHALL BE AVAILABLE ON THE JOB SITE PRIOR TO PLACING OF CONCRETE.
- MILL TEST REPORTS FOR GRADE 60 BARS SHALL BE SUBMITTED PRIOR TO PLACEMENT OF CONCRETE.
- CONTINUOUS INSPECTION OF CONCRETE SHALL INCLUDE INSPECTION DURING INSTALLATION OF REINFORCING STEEL. INSPECTION SHALL BE SCHEDULED SO THAT PLACEMENT OF REINFORCING STEEL, CONDUIT SLEEVES AND EMBEDDED ITEMS MAY BE CORRECTED PRIOR TO PLACEMENT OF OVERLYING GRIDS OF REINFORCING STEEL.
- CONCRETE PROTECTION FOR REINFORCEMENT:

CAST-IN-PLACE CONCRETE: THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

	MINIMUM COVER
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3"
CONCRETE EXPOSED TO EARTH OR WEATHER	
#6 & LARGER	2"
#5 & SMALLER	1 1/2"
SLABS, WALLS, OR JOISTS NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND	
#14 & LARGER	1 1/2"
#11 & SMALLER	3/4"
BEAM AND COLUMN TIES & STIRRUPS NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	1 1/2"

FOUNDATION

- FOUNDATION DESIGN BASED ON THE GEOTECHNICAL INVESTIGATION REPORT 306642-001 BY EARTH SYSTEMS PACIFIC DATED DECEMBER 16, 2024.
- CONVENTIONAL FOUNDATIONS AND SLABS-ON-GRADE SHALL BEAR ON COMPACTED FILL. ALL UNSUITABLE SOILS SHALL BE REMOVED AND RECOMPACTED UNDER THE SUPERVISION OF THE GEOTECHNICAL ENGINEER OR HIS/HER REPRESENTATIVE. REFER TO THE GEOTECHNICAL REPORT AND PROJECT SPECIFICATIONS FOR EXTENT OF REMOVAL AND COMPACTION REQUIREMENTS.
- FOOTINGS ARE DESIGNED BASED ON THE FOLLOWING INFORMATION FROM THE GEOTECHNICAL REPORT:

MAT FOUNDATION

ALLOWABLE NET BEARING PRESSURE UNDER DEAD AND SUSTAINED LIVE LOADS = 550 PSF
 ALLOWABLE NET BEARING PRESSURE UNDER CONCENTRATED LOAD AREAS = 2,000 PSF
 SUBGRADE MODULUS, $k_s = 21.8$ PSI/IN

MAT FOUNDATION SHALL BE UNDERLAIN AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER. BEARING CAPACITY IS BASED ON 12-INCH MINIMUM FOOTING DEPTH BELOW ADJACENT GRADE.

CONTINUOUS FOOTINGS

ALLOWABLE BEARING PRESSURE = 2,000 PSF

BEARING CAPACITY IS BASED ON 15-INCH MINIMUM FOOTING WIDTH AND 12-INCH MINIMUM FOOTING DEPTH BELOW ADJACENT GRADE.

INCREASE FOR SHORT TERM LOADS

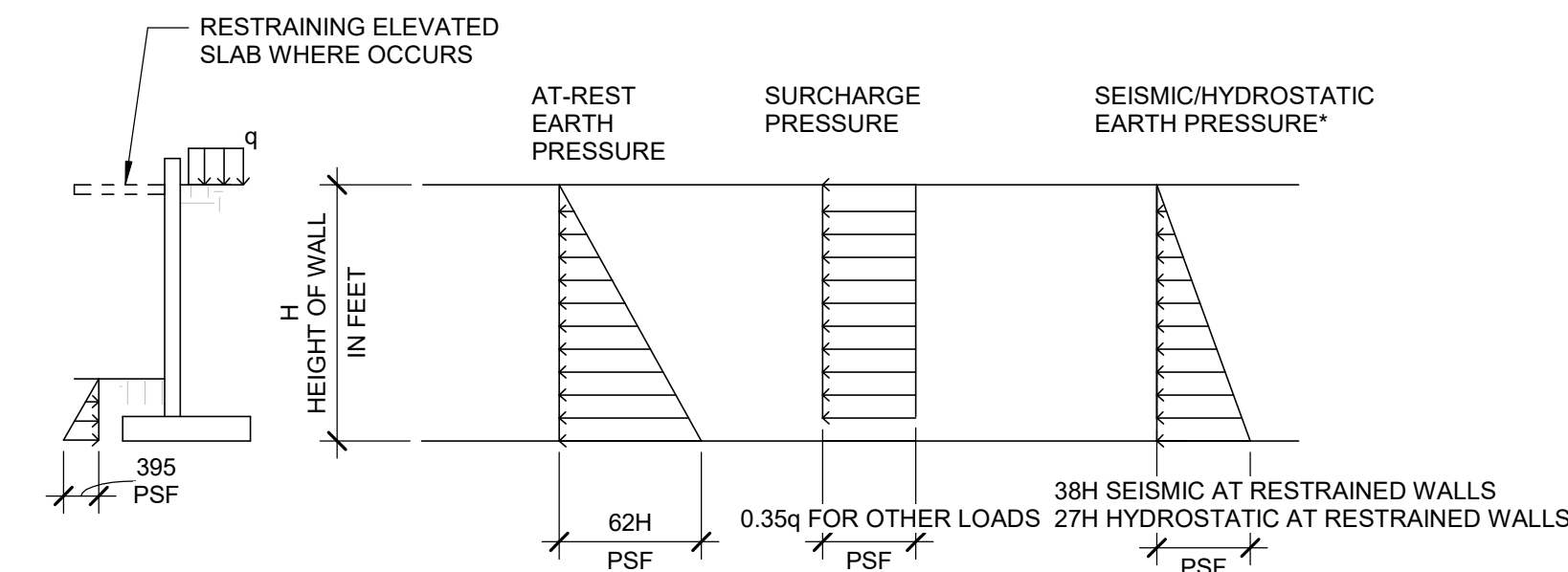
ALLOWABLE BEARING PRESSURES MAY BE INCREASED BY 1/3 WH WHEN APPLIED DEAD AND LIVE LOADS ARE COMBINED WITH WIND AND SEISMIC FORCES.

LATERAL RESISTANCE

ALLOWABLE PASSIVE EARTH PRESSURE = 395 PCF
 COEFFICIENT OF FRICTION = 0.60
 SLIDING FACTOR OF SAFETY = 1.5

PASSIVE AND FRICTION RESISTANCE MAY BE COMBINED TO RESIST LATERAL LOADS INCLUDING WIND AND SEISMIC WITHOUT REDUCTION.

- RETAINING WALLS ARE DESIGNED BASED ON THE FOLLOWING INFORMATION:



- REFER TO GEOTECHNICAL REPORT FOR GROUNDWATER ELEVATION. CONTRACTOR TO PROVIDE DE-WATERING OF EXCAVATIONS FROM EITHER SURFACE WATER, GROUND WATER AND/OR SEEPAGE, IF REQUIRED.
- EXCAVATION FOR FOOTINGS SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE AND REINFORCING. CONTRACTOR TO NOTIFY THE SOILS ENGINEER WHEN INSPECTION OF EXCAVATION IS READY. SOILS ENGINEER TO SUBMIT LETTER OF COMPLIANCE.
- ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS BEFORE CONCRETE OR GROUT HAS ATTAINED FULL DESIGN STRENGTH. BRACE OR PROTECT ALL BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHING FLOORS ARE IN PLACE AND HAVE ATTAINED FULL STRENGTH. CONTRACTOR SHALL PROVIDE FOR DESIGN, PERMITS AND INSTALLATION OF SUCH BRACING.
- FOOTING BACKFILL AND UTILITY BACKFILL WITHIN BUILDING AREA SHALL BE MECHANICALLY COMPACTED IN LAYERS IN ACCORDANCE WITH THE SOILS REPORT AND APPROVED BY THE SOILS ENGINEER. FLOODING WILL NOT BE PERMITTED. ALL FILLS USED TO SUPPORT FOUNDATIONS SHALL BE INSPECTED BY THE SOILS ENGINEER REPRESENTATIVE
- REMOVE ALL ABANDONED FOOTINGS, UTILITIES, ETC.
- PLACE A MOISTURE BARRIER MEMBRANE OVER SAND AND GRAVEL BASE UNDER SLABS-ON-GRADE PER THE GEOTECHNICAL REPORT, OR AS INDICATED ON THE DRAWINGS AND PROJECT SPECIFICATIONS.

STRUCTURAL OBSERVATIONS

- STRUCTURAL OBSERVATION SHALL BE PERFORMED BY THE STRUCTURAL ENGINEER OF RECORD OR DESIGNEE IN ACCORDANCE WITH SECTION 1704.6 OF THE CODE.
- STRUCTURAL OBSERVATION IS THE VISUAL OBSERVATION OF THE ELEMENTS AND CONNECTIONS OF THE STRUCTURAL SYSTEM AT SIGNIFICANT CONSTRUCTION STAGES AND THE COMPLETED STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATION. STRUCTURAL OBSERVATION DOES NOT WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED OF THE CITY OR DEPUTY INSPECTORS.
- THE CONTRACTOR SHALL COORDINATE AND CALL FOR A PRE-CONSTRUCTION MEETING BETWEEN THE ENGINEER OR ARCHITECT RESPONSIBLE FOR THE STRUCTURAL DESIGN, STRUCTURAL OBSERVER, CONTRACTOR, AFFECTED SUBCONTRACTORS, PROJECT INSPECTOR. THE PURPOSE OF THE MEETING SHALL BE TO IDENTIFY THE MAJOR STRUCTURAL ELEMENTS AND CONNECTIONS THAT AFFECT THE VERTICAL AND LATERAL LOAD SYSTEMS OF THE STRUCTURE AND TO REVIEW SCHEDULING OF THE REQUIRED OBSERVATIONS.
- THE STRUCTURAL OBSERVER SHALL PERFORM SITE VISITS AT THOSE STEPS IN THE PROGRESS OF THE WORK THAT ALLOW FOR CORRECTION OF DEFICIENCIES WITHOUT SUBSTANTIAL EFFORT OR UNCOVERING OF THE WORK INVOLVED. AT A MINIMUM, THE FOLLOWING SIGNIFICANT CONSTRUCTION STAGES REQUIRE A SITE VISIT AND AN OBSERVATION REPORT FROM THE STRUCTURAL OBSERVER.

CONSTRUCTION STAGES	ELEMENTS/CONNECTIONS TO BE OBSERVED
FOUNDATIONS CONCRETE WALLS CONCRETE SLABS	REINF/CONC/ANCHOR BOLTS REINF/EMBEDS REINF/EMBEDS

- THE STRUCTURAL OBSERVER SHALL PREPARE A REPORT FOR EACH SIGNIFICANT STATE OF CONSTRUCTION OBSERVED. A COPY OF THE OBSERVATION REPORT SHALL BE SENT TO THE OWNER, CONTRACTOR AND INSPECTOR OF RECORD (IOR).

DESIGN LOADS

- DEAD LOADS

SELF-WEIGHT OF MATERIALS

PRE-ENGINEERED METAL BLDG (PEMB):

ROOF DEAD LOAD = 2.2 PSF
 SUPERIMPOSED DL = 2.0 PSF
 COLLATERAL DL = 10.0 PSF

MODULAR LIVING QUARTERS & GYM (MODULAR):

ROOF DEAD LOAD* = 13 PSF
 FLOOR DEAD LOAD = 8 PSF

- FLOOR LIVE LOADS:

PEMB

STORAGE ROOMS = 125 PSF (NON-REDUCIBLE)

EQUIPMENT ROOMS = 150 PSF (NON-REDUCIBLE)

MODULAR LIVE LOADS*

ASSEMBLY AREAS = 100 PSF (NON-REDUCIBLE)

- ROOF LIVE LOADS:

PEMB BLDG ROOF = 20 PSF (REDUCIBLE)

MODULAR = 20 PSF (REDUCIBLE)

- WIND LOADS:

WIND LOADS ARE IN ACCORDANCE WITH SECTION 1609 OF THE CODE.

RISK CATEGORY: IV

WIND SPEED: $V_{ult} = 105$ MPH (3-SECOND GUST)

WIND EXPOSURE: C

INTERNAL PRESSURE COEFFICIENT: $G_{CP} = \pm 0.18$

- SEISMIC DESIGN PARAMETERS

DESIGN PARAMETERS	SYMBOL	RECOMMENDED VALUE
SITE CLASS	-	E
MAPPED SPECTRAL ACCELERATION FOR SHORT PERIODS	S_s	1.956g
MAPPED SPECTRAL ACCELERATION FOR A 1-SECOND PERIOD	S_1	0.735g
SITE COEFFICIENT	F_a	1.00
SITE COEFFICIENT	F_v	4.00
DESIGN SPECTRAL RESPONSE ACCELERATION FOR SHORT PERIODS	S_{DS}	1.191g
DESIGN SPECTRAL RESPONSE ACCELERATION FOR 1-SECOND PERIOD	S_{D1}	1.895g

- SEISMIC ANALYSIS OF PRIMARY STRUCTURE

SEISMIC IMPORTANCE FACTOR, I_e	1.5
RISK CATEGORY	IV
SEISMIC DESIGN CATEGORY	D
ANALYSIS PROCEDURE USED	STATIC EQUIVALENT LATERAL FORCE PROCEDURE
PEMB SEISMIC FORCE RESISTING SYSTEM	
NORTH/SOUTH DIRECTION	
ORDINARY STEEL CONCENTRICALLY BRACED FRAMES / ORDINARY STEEL MOMENT FRAMES	
R	3.25
Ω_0	3
Cd	3.25
EAST/WEST DIRECTION	
ORDINARY STEEL MOMENT FRAMES	
R	3.5
Ω_0	3
Cd	3
MODULAR SEISMIC FORCE RESISTING SYSTEM	
ORDINARY STEEL MOMENT FRAMES	
R	3.5
Ω_0	3
Cd	3

- LOADING MARKED WITH AN ASTERISK (*) ABOVE WAS PROVIDED BY THE PEMB OR MODULAR FABRICATOR AS APPLICABLE FOR USE IN FOUNDATION DESIGN.

GENERAL

- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO STARTING CONSTRUCTION. THE ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES.
- ALL DRAWINGS, SPECIFICATIONS AND DOCUMENTS ENUMERATED IN THE OWNER/CONTRACTOR AGREEMENT ARE CONSIDERED TO BE PART OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REVIEW AND COORDINATION OF ALL DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF CONSTRUCTION. ANY DISCREPANCIES THAT OCCUR SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO START OF CONSTRUCTION SO THAT A CLARIFICATION CAN BE ISSUED. ANY WORK PERFORMED IN CONFLICT WITH THE CONTRACT DOCUMENTS OR ANY CODE REQUIREMENTS SHALL BE CORRECTED BY THE CONTRACTOR AT HIS OWN EXPENSE AND AT NO EXPENSE TO THE OWNER OR ARCHITECT.
- NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE GIVEN, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK. WHERE CONFLICTS BETWEEN DRAWINGS AND THE SPECIFICATIONS OCCUR, CONSTRUCTION SHALL FOLLOW THE MORE STRINGENT REQUIREMENT UNLESS OTHERWISE APPROVED BY SEOR.
- ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS OF THE FOLLOWING CODES:

2022 CALIFORNIA BUILDING CODE, CALIFORNIA CODE OF REGULATIONS, TITLE 24, PART 2, VOLUME 2 OF 2 AND LATEST REVISIONS ADOPTED PRIOR TO PERMITTING REFERRED TO HERE AS "THE CODE", AND ANY OTHER REGULATING AGENCIES WHICH HAVE AUTHORITY OVER ANY PORTION OF THE WORK, INCLUDING STATE OF CALIFORNIA DIVISION OF OCCUPATIONAL SAFETY AND HEALTH (DOSH), AND THOSE CODES & STANDARDS LISTED IN THESE NOTES AND SPECIFICATIONS.
- ALL REFERENCED STANDARDS SHALL BE THE VERSION INDICATED IN CHAPTER 35 OF THE CODE.
- SEE DRAWINGS PREPARED BY OTHER CONSULTANTS AND/OR VENTORS FOR THE FOLLOWING:
 - SIZE AND LOCATION OF ALL DOOR AND WINDOW OPENINGS, EXCEPT AS NOTED.
 - SIZE AND LOCATION OF ALL INTERIOR AND EXTERIOR NON-BEARING PARTITIONS UNLESS NOTED AND/OR DETAILED ON THE STRUCTURAL DRAWINGS.
 - SIZE AND LOCATION OF ALL CONCRETE CURBS, EQUIPMENT PADS, PITS, FLOOR DRAINS, SLOPES, DEPRESSED AREAS, CHANGES IN LEVEL, CHAMFERS, GROOVES, INSERTS, ETC.
 - SIZE AND LOCATION OF ALL FLOOR AND ROOF OPENINGS EXCEPT AS SHOWN.
 - FLOOR AND ROOF FINISHES.
 - MISCELLANEOUS DRAINAGE AND WATERPROOFING.
 - ALL FIREPROOFING REQUIREMENTS INCLUDING FIREPROOFING OF STRUCTURAL STEEL.
 - DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS.

- SEE MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS FOR THE FOLLOWING:

- PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC., EXCEPT AS SHOWN OR NOTED.
- ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS EXCEPT AS SHOWN OR NOTED.
- CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL OR PLUMBING FIXTURES, EXCEPT AS SHOWN OR NOTED.
- SIZE AND LOCATION OF MACHINE OR EQUIPMENT BASES, ANCHOR BOLTS FOR MOTOR MOUNTS.

- THE STRUCTURAL CONTRACT DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION, INCLUDING BUT NOT LIMITED TO BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT ETC. OBSERVATION VISITS TO THE SITE BY THE STRUCTURAL ENGINEER DO NOT INCLUDE REVIEW OF THE ABOVE ITEMS.
- INVESTIGATE SITE DURING CLEARING AND EARTHWORK OPERATIONS FOR FILLED EXCAVATIONS OR BURIED STRUCTURES, SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, ETC. IF ANY SUCH STRUCTURES ARE FOUND, NOTIFY THE CITY IMMEDIATELY.
- OPENINGS, POCKETS, PENETRATIONS, NON-STRUCTURAL EMBEDS, ETC. SHALL NOT BE PLACED IN ANY STRUCTURAL MEMBER EXCEPT AS NOTED OR DETAILED ON THESE DRAWINGS. NOTIFY THE STRUCTURAL ENGINEER WHEN DRAWINGS BY OTHERS SHOW SUCH ITEMS PLACED IN A MANNER THAT IS NOT CONSISTENT WITH THE STRUCTURAL DRAWINGS.
- THE CONTRACT STRUCTURAL DRAWINGS SHOW THE BUILDING IN ITS FINAL INTENDED POSITION. MAKE PROVISIONS IN THE CONSTRUCTION SEQUENCING OF THE BUILDING TO TAKE INTO ACCOUNTS SHRINKAGE, CREEP, SHORTENING, THERMAL EXPANSION, ETC..
- CONTRACTOR SHALL PROVIDE FOR DESIGN AND INSTALLATION OF ALL CRIBBING, SHEATHING AND SHORING REQUIRED AND SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH ALL NATIONAL, STATE AND LOCAL SAFETY ORDINANCES.
- COORDINATE AND VERIFY EDGE OF SLAB DIMENSIONS PRIOR TO FABRICATION OR PLACEMENT OF FORMWORK.
- OBTAIN AN UNDERGROUND SERVICE ALERT INQUIRY IDENTIFICATION NUMBER AT LEAST TWO WORKING DAYS BEFORE STARTING WORK WITH THIS PERMIT. DIAL 811 (OR 1-800-422-4133 FOR SOUTHERN CALIFORNIA PROJECTS)
- THE CONTRACTOR SHALL VERIFY THE EXTENT AND LOCATIONS OF SITE UTILITIES PRIOR TO EXCAVATION OR SHORING. THERE MAY BE DISCREPANCIES BETWEEN THE LOCATION INDICATED ON THE SITE SURVEY AND ACTUAL VERIFIED LOCATIONS. IF THE ACTUAL FIELD VERIFIED LOCATION OF UTILITIES COULD RESULT IN A CONFLICT WITH THE NEW CONSTRUCTION OR SHORING, THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY.
- CONTRACTOR SHALL COORDINATE SHORING WITH DRAWINGS OF RECORD TO ENSURE PROVISIONS FOR POCKETS, BLOCKOUTS, OFFSETS, STEPPED FOOTINGS AND ANY OTHER ITEMS AFFECTED BY THE SHORING.
- VERIFY THAT THE ACTUAL OPERATING WEIGHT OF ALL EQUIPMENT DOES NOT EXCEED THAT SHOWN ON THE DRAWINGS. NOTIFY THE ARCHITECT OF ANY DISCREPANCIES PRIOR TO INSTALLATION.
- CONTRACTOR SHALL SUBMIT PLANS SHOWING ALL PROPOSED OPENINGS (SIZE & LOCATIONS) TO AOR & SEOR FOR REVIEW AND APPROVAL BEFORE START OF WORK.
- STOCK PILING OR STORAGE OF MATERIAL ON OR NEAR SHOWING BULKHEAD IS NOT PERMITTED.

4	ADDENDUM NO. 4	6/22/26
3	PLAN CHECK 3	4/17/26
2	PLAN CHECK 2	2/17/26
1	PLAN CHECK 1	8/11/25

REV. DESCRIPTION CKD APP DATE

PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION

CITY OF SAN BUENAVENTURA

INTERIM FIRE STATION NO. 7

STRUCTURAL GENERAL NOTES

DRN. BY: CC DES. BY: KK CKD. BY: -

PRINCIPAL ENGINEER DATE

CITY ENGINEER DATE

SPEC. NUMBER 2025-010 PROJECT NO. FA11-1061

SEPTEMBER 7, 2025 SHEET 22 OF 151 DWG. NO. 2025-D-010



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TABLE 1 - REQUIRED SPECIAL INSPECTIONS				
SYSTEM OR MATERIAL	INSPECTION			REMARKS
	INSPECTION TYPE	CODE REFERENCE	REFERENCED STANDARDS	
SOILS				
1. VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	PERIODIC	1705.6	GEOTECHNICAL REPORT	BY THE GEOTECHNICAL ENGINEER
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	PERIODIC			
3. PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS	PERIODIC			
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL	CONTINUOUS			
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	PERIODIC			
GEOTECHNICAL				
FILL IN-PLACE DENSITY OR PREPARED SUBGRADE DENSITY		1705.6	ASTM D1557	BY GEOTECHNICAL ENGINEER
MATERIAL VERIFICATION			CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIAL	BY GEOTECHNICAL ENGINEER
CONCRETE				
CONCRETE STRENGTH		1903 1705.3	ASTM C39	FABRICATE SPECIMENS AT TIME FRESH CONCRETE IS PLACED. TEST EACH 150 CY NOR LESS THAN EACH 5000 SF OF SLAB OR WALL PLACED EACH DAY
CONCRETE SLUMP			ASTM C143	
CONCRETE AIR CONTENT			ASTM C231	
CONCRETE TEMPERATURE			ASTM C1064	
UNIT WEIGHT OF FRESH STRUCTURAL LIGHTWEIGHT CONCRETE			ASTM C567	
FLOOR FLATNESS AND LEVELNESS			ASTM E1155	TEST WITHIN 24 HOURS OF FINISHING
SHOTCRETE STRENGTH		1910.5 1910.10	ASTM C39	SPECIMEN TAKEN FROM THE IN-PLACE OR FROM TEST PANELS. EACH 50 CY NOR LESS THAN EACH 5000 SF OF WALL PLACED EACH SHIFT
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS AND VERIFY PLACEMENT	PERIODIC	1705.3 1908.4	ACI 318 CHAPTER 20, 25.2 - 25.3, 26.6.1 - 26.6.3	TOLERANCE AND REINFORCING PLACEMENT PER ACI 318, SECTION 26.6.2
2. REINFORCING BAR WELDING		1705.3	ACI 318 26.6.4 AWS D1.4, SECTION 7	VISUALLY INSPECT ALL WELDS. MATERIAL VERIFICATION OF REINFORCING STEEL FOR WELDING (CERTIFIED MILL TEST REPORTS), VERIFICATION OF WELD FILLER METALS, USE OF PROPER WPS'S AND WELDER QUALIFICATIONS
A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706	PERIODIC			
B. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"	PERIODIC			
C. INSPECT ALL OTHER WELDS	CONTINUOUS			
3. INSPECT ANCHORS CAST IN CONCRETE	PERIODIC		ACI 318 17.8.2	
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.				INSPECTION REQUIREMENTS PER ICC EVALUATION REPORT.
A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATION TO RESIST SUSTAINED TENSION LOADS.	CONTINUOUS		ACI 318 17.8.2.4	VERIFY THAT ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATION ARE INSTALLED BY CERTIFIED INSTALLERS.
B. MECHANICAL ANCHORS AND ADHESIVE NOT DEFINED IN 4A.	PERIODIC		ACI 318 17.8.2	
5. VERIFY USE OF REQUIRED DESIGN MIX	PERIODIC	1904.1, 1904.2, 1908.2, 1908.3	ACI 318 CH 19, 24.4.3, 26.4.4	VERIFY THAT ALL MIXES USED COMPLY WITH APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED CODES AND STANDARDS
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TEST, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE TEMPERATURE OF CONCRETE.	CONTINUOUS	1908.10	ASTM C172 ASTM C31 ACI 318 26.4, 26.12	
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	CONTINUOUS	1904.6, 1908.7, 1908.8	ACI 318 26.5	
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	1908.9	ACI 318 26.5.3 - 26.5.5	
9. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	PERIODIC		ACI 318 26.11.2	
10. INSPECT FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF CONCRETE MEMBER BEING FORMED.	PERIODIC		ACI 318 26.11.1.2(b)	
11. INSPECT INSTALLATION OF MECHANICAL COUPLING DEVICES.	CONTINUOUS		ICC EVALUATION REPORT	VERIFY GRADE AND SIZE OF REBAR BEING SPLICED, COUPLER IDENTIFICATION AND POSITION, AND INSTALLATION OF COUPLER
DESIGNATED SEISMIC SYSTEM				
1. DESIGNATED SEISMIC SYSTEM	PERFORM	1705.12.4		EXAMINE DESIGNATED SEISMIC SYSTEMS REQUIRING QUALIFICATION IN ACCORDANCE WITH SECTION 13.2.2 OF ASCE 7 AND VERIFY THAT THE LABEL, ANCHORAGE, AND MOUNTING CONFORM TO THE CERTIFICATE OF COMPLIANCE

TABLE 1 (CONT.) - REQUIRED SPECIAL INSPECTIONS				
SYSTEM OR MATERIAL	INSPECTION			REMARKS
	INSPECTION TYPE	CODE REFERENCE	REFERENCED STANDARDS	
STEEL				
INSPECTION TASKS PRIOR TO WELDING				
1. WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE	PERFORM	1705.2.1	AISC 360, TABLE N5.4-1 AISC 341, TABLE J6-1	VERIFY JOINT PREPARATION, DIMENSIONS, CLEANLINESS, TACK WELD QUALITY, BACKING TYPE AND FIT
2. MANUFACTURER CERTIFICATION FOR WELDING CONSUMABLES AVAILABLE	PERFORM			
3. MATERIAL IDENTIFICATION (TYPE/GRADE)	OBSERVE			
4. WELDER IDENTIFICATION SYSTEM	OBSERVE			
5. FIT-UP GROOVE WELDS (INCLUDING JOINT GEOMETRY)	OBSERVE			
6. CONFIGURATION AND FINISH OF ACCESS HOLE	OBSERVE			
7. FIT-UP OF FILLET WELDS	OBSERVE			
8. CHECK WELDING EQUIPMENT	OBSERVE			
INSPECTION TASKS DURING WELDING				
1. USE OF QUALIFIED WELDERS	OBSERVE	1705.2.1	AISC 360, TABLE N5.4-2 AISC 341, TABLE J6-2	VERIFY PACKAGING AND EXPOSURE CONTROL
2. CONTROL AND HANDLING OF WELDING CONSUMABLES	OBSERVE			
3. NO WELDING OVER TACK WELDS	OBSERVE			
4. ENVIRONMENTAL CONDITIONS	OBSERVE			
5. WELDING PROCEDURE SPECIFICATION (WPS) FOLLOWED	OBSERVE			
6. WELDING TECHNIQUES	OBSERVE			VERIFY WIND SPEED IS WITHIN LIMITS AS WELL AS PRECIPITATION AND TEMPERATURE
INSPECTION TASKS AFTER WELDING				
1. WELDS CLEANED	OBSERVE	1705.2.1	AISC 360, TABLE N5.4-3 AISC 341, TABLE J6-3	VERIFY CRACK PROHIBITION, WELD/BASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES AND SIZE, UNDERCUT, AND POROSITY. DOCUMENT FINDINGS IN REPORT.
2. SIZE, LENGTH, AND LOCATION OF WELDS	PERFORM			
3. WELDS MEET VISUAL ACCEPTANCE CRITERIA	PERFORM			
4. ARC STRIKES	PERFORM			
5. K-AREA	PERFORM			
6. BACKING REMOVED AND WELD TABS REMOVED WHERE REQUIRED	PERFORM			
7. PLACEMENT OF REINFORCING OR CONTOURING FILLET WELDS WHERE REQUIRED	PERFORM			
8. REPAIR ACTIVITIES	PERFORM			
9. DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	PERFORM			
INSPECTION TASKS PRIOR TO BOLTING				
1. MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	PERFORM	1705.2.1	AISC 360, TABLE N5.6-1 AISC 341, TABLE J7-1	VERIFY FAYING SURFACE CONDITION AND HOLD PREPARATION
2. FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	OBSERVE			
3. PROPER FASTENER SELECTED FOR THE JOINT DETAIL	OBSERVE			
4. PROPER BOLTING PROCEDURE SELECTED FOR THE JOINT DETAIL	OBSERVE			
5. CONNECTING ELEMENTS MEET APPLICABLE REQUIREMENTS	OBSERVE			
6. PRE-INSTALLATION VERIFICATION TESTING BY INSTALLING PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED.	OBSERVE			
7. PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENT	OBSERVE			

STRUCTURAL STEEL

- STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH CHAPTER 22 OF THE CODE AND THE FOLLOWING REFERENCES:
 - AISC 303 - "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"
 - AISC 360 - "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS"
 - AISC 341 - "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS" FOR MEMBERS OF THE SEISMIC FORCE RESISTING SYSTEM (SFRS)
 - RCSC's - "SPECIFICATIONS FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS"
- COMPLY WITH THE FOLLOWING PROVISIONS FOR ALL WELDED JOINTS:
 - AWS D11.1 - "STRUCTURAL STEEL WELDING CODE"
 - AWS D1.8 - "SEISMIC SUPPLEMENT" FOR CONNECTIONS OF THE SEISMIC FORCE RESISTING SYSTEM (SFRS)
- ALL STRUCTURAL STEEL SHALL CONFORM TO THE ASTM DESIGNATION AS INDICATED BELOW (UNO):

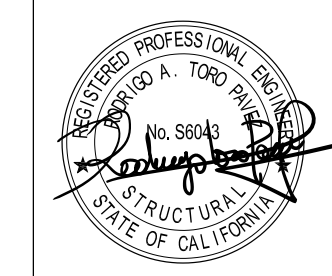
	ASTM SPECIFICATION
ANCHOR RODS	F1554 GR 55 F1554 GR 36/105 (WHERE INDICATED)
- THE STRUCTURAL STEEL FABRICATOR SHALL FURNISH SHOP DRAWINGS TO THE ENGINEER OF ALL STEEL FOR STRUCTURAL ENGINEER'S REVIEW AND APPROVAL BEFORE FABRICATION.
- ALL WELDING IS TO BE DONE BY CERTIFIED WELDERS USING E70XX ELECTRODES (UNO). ALL WELDS SHALL BE IN CONFORMITY WITH THE PROJECT SPECIFICATIONS AND THE CODE FOR WELDING IN BUILDING CONSTRUCTION (AWS D1.1 - 15) OF THE AMERICAN WELDING SOCIETY. SEE SPECIAL INSPECTION SECTION FOR WELDING REQUIREMENTS.
- WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED. WHERE FILLET WELD SYMBOL IS GIVEN WITHOUT INDICATION OF SIZE, USE MINIMUM SIZE WELDS AS SPECIFIED IN AISC 360 SECTION J2.2b.
- ALL STRUCTURAL STEEL AND MISCELLANEOUS METAL EXPOSED TO THE WEATHER SHALL BE HOT DIP GALVANIZED AFTER FABRICATION. PROTECT FIELD WELDS EXPOSED TO THE WEATHER VIA PRIME AND PAINT OR BRUSH/COLD GALVANIZING. REFER TO ARCHITECTURAL DRAWINGS FOR STEEL FINISH.
- DO NOT CUT OR DAMAGE EXISTING REINFORCEMENT. PRIOR TO FABRICATING PLATES, MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED TO REINFORCED CONCRETE/MASONRY USING POST-INSTALLED ANCHORS, LOCATE ALL REINFORCEMENT AND CONFIRM CONSTRUCTIBILITY OF ANCHOR LOCATIONS. SHOULD CONFLICTS WITH REINFORCEMENT OCCUR, SUBMIT ALTERNATE ANCHOR LOCATIONS AND REVISED STEEL FABRICATIONS TO ARCHITECT FOR REVIEW AND APPROVAL.
- BACKUP BARS MAY REMAIN IN PLACE UNLESS NOTED OTHERWISE ON THE DRAWINGS, OR WHEN ULTRASONIC TESTING INDICATES A POSSIBLE WELD DEFECT. IF DEFECTS ARE INDICATED BACKUP BAR IS TO BE REMOVED AND THE ROOT INSPECTED. IF IMPERFECTIONS ARE FOUND, THEY ARE TO BE REMOVED AND REPAIRED PER AWS REQUIREMENTS.
- THE USE OF E70T-4 WELDING WIRE IS NOT ALLOWED FOR ANY APPLICATION. ALL WELD FILLER METAL SHALL BE OF THE LOW HYDROGEN TYPE.
- WRITTEN WELDING PROCEDURE SPECIFICATIONS (WPS) PER THE RECOMMENDATION OF THE AMERICAN WELDING SOCIETY (AWS) SHALL BE DEVELOPED BY THE FABRICATOR/ERECTOR AND SUBMITTED FOR REVIEW TO THE ENGINEER PRIOR TO ANY WELDING OF THE STRUCTURAL STEEL. THE WELDING PROCEDURES SHALL INCLUDE ALL THE WELDED JOINTS AND CONFIGURATIONS TO BE USED ON THIS PROJECT-ONLY WPS WHICH ARE RELEVANT TO THIS PROJECT SHALL BE SUBMITTED. ALL WELDED JOINTS SHALL BE PREQUALIFIED PER AWS OR BE QUALIFIED BY TEST PER AWS. A PROCEDURE QUALIFICATION RECORD (PQR) SHALL BE INCLUDED WITH THE WPS IF THE WELDING PROCEDURE OR JOINT IS QUALIFIED BY TESTING. THE ELECTRODE MANUFACTURER AND PRODUCT/TRADE NAME SHALL BE IDENTIFIED IN THE WPS IN ADDITION TO THE AWS ELECTRODE CLASSIFICATION NAME. A COPY OF THE ELECTRODE MANUFACTURER'S TECHNICAL DATA SHEETS WITH THE RECOMMENDED WELDING PARAMETERS SHALL BE SUBMITTED WITH THE WPS.

SPECIAL INSPECTIONS

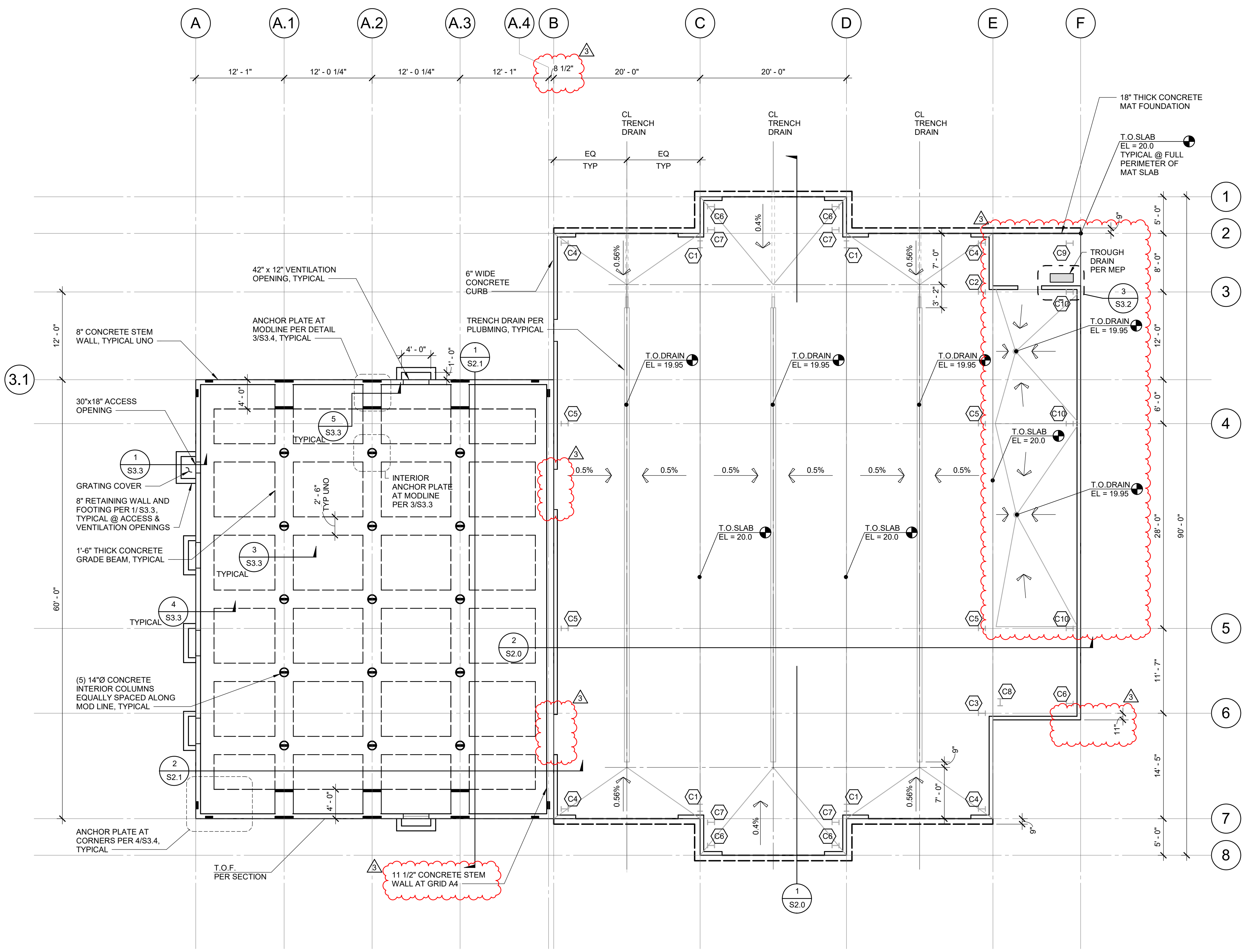
- THE FOLLOWING ELEMENTS OF CONSTRUCTION SHALL HAVE CONTINUOUS INSPECTION BY A PROJECT INSPECTOR APPROVED BY THE JURISDICTION.
 - CONCRETE.
 - BOLTS INSTALLED IN CONCRETE.
 - PLACING OF REINFORCING STEEL.
 - SEE GEOTECHNICAL ENGINEER'S REPORT FOR SPECIFIC INSPECTION REQUIREMENTS BY A SOILS ENGINEER REPRESENTATIVE.
- ALL SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 1704 OF THE CODE AND ANY ADDITIONAL REQUIREMENTS STATED IN THESE DRAWINGS AND/OR THE PROJECT SPECIFICATIONS.
- OWNER WILL RETAIN A QUALIFIED, INDEPENDENT SPECIAL INSPECTION AGENCY TO PERFORM SPECIAL INSPECTIONS AND TESTS PER CHAPTER 17 OF THE CODE. REFER TO TABLE 1 FOR TESTS AND INSPECTIONS THAT WILL BE PERFORMED.
- SPECIAL INSPECTOR WILL OBSERVE THE INDICATED WORK FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. ALL NON-CONFORMING WORK WILL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION AND NOTED IN THE INSPECTION REPORTS.
- TESTING FREQUENCIES IDENTIFIED IN THE STATEMENT OF SPECIAL INSPECTION ARE DEFINED AS FOLLOWS:
 - CONTINUOUS:** THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.
 - PERIODIC:** THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.
 - OBSERVE:** THE SPECIAL INSPECTOR WILL OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.
 - PERFORM:** THE SPECIAL INSPECTOR WILL PERFORM THESE TASKS FOR EACH ELEMENT.
 - DOCUMENT:** THE SPECIAL INSPECTOR WILL DOCUMENT IN A REPORT THAT THE WORK HAS BEEN PERFORMED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

4	ADDENDUM NO. 4	6/22/26
3	PLAN CHECK 3	4/17/26
2	PLAN CHECK 2	2/17/26
1	PLAN CHECK 1	8/11/25

REV.	DESCRIPTION	CKD	APP	DATE
PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION				
CITY OF SAN BUENAVENTURA				
INTERIM FIRE STATION NO. 7				
STRUCTURAL GENERAL NOTES				
DRN. BY:	CC	DES. BY:	KK	CKD. BY: -



PRINCIPAL ENGINEER	DATE
CITY ENGINEER	DATE
SPEC. NUMBER 2025-010	PROJECT NO. FA11-1061
SEPTEMBER 7, 2025	SHEET 23 OF 151 DWG. NO. 2025-D-010



PLAN NOTES

- SEE SHEETS S0.1 AND S0.2 FOR GENERAL NOTES.
- SEE SHEETS S3.0 & S3.1 FOR TYPICAL CONCRETE DETAILS.
- PRE-ENGINEERED METAL BUILDING DESIGN AND CONSTRUCTION SHALL INCLUDE CEILING AND STUD WALLS AT STORAGE ROOM, & STUD WALLS AT PERIMETER.
- ALL ADDED BAR SHALL BE STAGGERED WITH UNIFORM REINFORCEMENT.
- CONTRACTOR TO CONSULT WITH SEOR FOR PREFERRED CONSTRUCTION JOINT LOCATIONS.
- CONTRACTOR SHALL SUBMIT A DRAWING SHOWING ALL OF THE PROPOSED FINAL PREPARATION AND OPENING LOCATIONS TO SEOR FOR APPROVAL PRIOR TO CONCRETE POUR.
- VERIFY SIZE AND NUMBER OF ANCHOR BOLTS AND THEIR LOCATIONS WITH PRE-ENGINEERING METAL BUILDING DRAWINGS.
- PRE-ENGINEERED METAL BUILDING DESIGN NOT PART OF FOUNDATION SCOPE.
- MODULAR BUILDING DESIGN NOT PART OF FOUNDATION SCOPE.
- WALL ANCHOR AT ROLL-UP DOOR PER 2/S3.2.

TYPICAL FOUNDATION LEGEND:

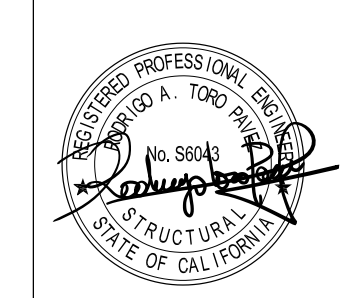
- INDICATES CONCRETE GRADE BEAM, SEE PLAN FOR SIZE
- INDICATES CONCRETE RETAINING WALL, SEE PLAN FOR SIZE.
- COLUMN ANCHOR ROD ANCHORAGE DETAILS ON S3.2
- INDICATES TOP OF SLAB
- INDICATES COLUMN LOCATION, REFER TO PRE-ENGINEERED METAL BUILDING DRAWINGS BY OTHERS
- INDICATES SLOPE OF TOP OF SLAB

1 PRE-ENGINEERED METAL BUILDING APPARATUS BAY AND LIVING QUARTERS FOUNDATION PLAN
SCALE: 1/8" = 1'-0"

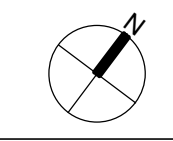
4	ADDENDUM NO. 4	6/22/26
3	PLAN CHECK 3	4/17/26
2	PLAN CHECK 2	2/17/26
1	PLAN CHECK 1	8/11/25

REV.	DESCRIPTION	CKD	APP	DATE
PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION				
CITY OF SAN BUENAVENTURA				
INTERIM FIRE STATION NO. 7				
FOUNDATION PLAN				
DRN. BY:	CC	DES. BY:	KK	CKD. BY:

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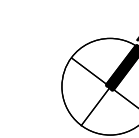
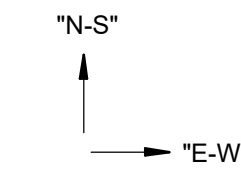
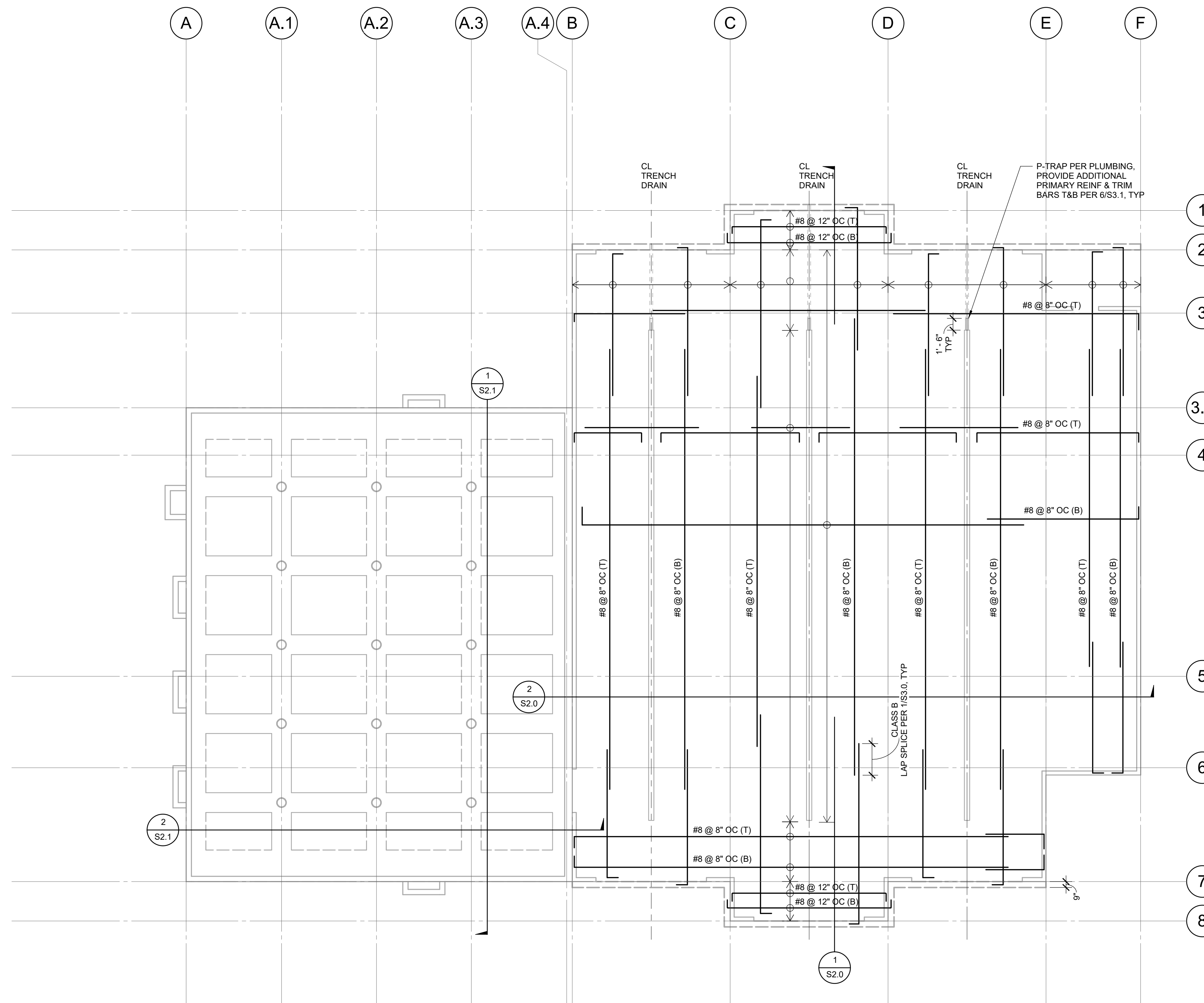


PRINCIPAL ENGINEER	DATE
CITY ENGINEER	DATE
SPEC. NUMBER 2025-010	PROJECT NO. FA11-1061
SEPTEMBER 7, 2025	SHEET 24 OF 151 DWG. NO. 2025-D-010



REINFORCING PLAN NOTES

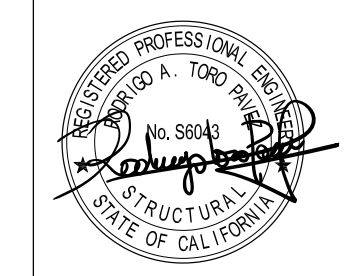
- FOR TYPICAL SECTION AT STRUCTURAL SLAB SEE SHEET S2.0
- ALL REINFORCING IN "N-S" DIRECTION TO BE PLACED IN OUTER LAYER.



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1 MAT FOUNDATION REINFORCEMENT PLAN
SCALE: 1/8" = 1'-0"

REV.	DESCRIPTION	CKD	APP	DATE
4	ADDENDUM NO. 4			6/22/26
3	PLAN CHECK 3			4/17/26
2	PLAN CHECK 2			2/17/26
1	PLAN CHECK 1			8/11/25

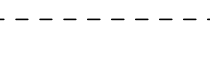
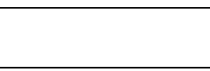


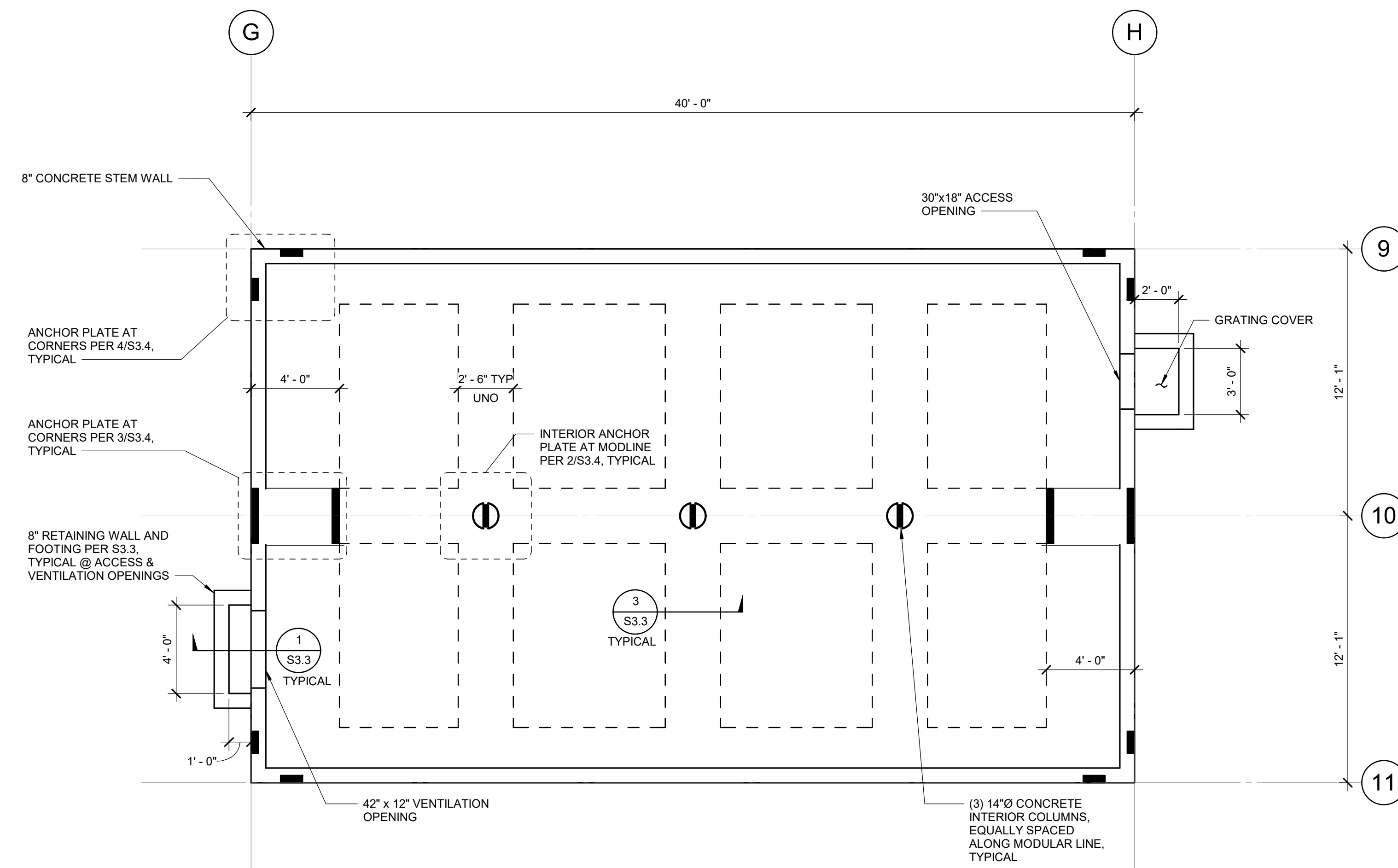
PUBLIC WORKS DEPARTMENT		ENGINEERING DIVISION	
CITY OF SAN BUENAVENTURA			
INTERIM FIRE STATION NO. 7			
REINFORCING PLAN			
DRN. BY:	CC	DES. BY:	KK
CKD. BY:			
PRINCIPAL ENGINEER		DATE	
CITY ENGINEER		DATE	
SPEC. NUMBER	2025-010	PROJECT NO.	FA11-1061
SEPTEMBER 7, 2025	SHEET 25 OF 151	DWG. NO. 2025-D-010	

PLAN NOTES

1. SEE SHEETS S0.1 AND S0.2 FOR GENERAL NOTES.
2. SEE SHEETS S3.0 & S3.1 FOR TYPICAL CONCRETE DETAILS.
3. CONTRACTOR SHALL SUBMIT A DRAWING SHOWING ALL OF THE PROPOSED FINAL PREPARATION AND OPENING LOCATIONS TO SECOR FOR APPROVAL PRIOR TO CONCRETE POUR.
4. MODULAR BUILDING DESIGN NOT PART OF FOUNDATION SCOPE.

TYPICAL FOUNDATION LEGEND:

-  INDICATES CONCRETE GRADE BEAM, SEE PLAN FOR SIZE.
-  INDICATES CONCRETE RETAINING WALL, SEE PLAN FOR SIZE.

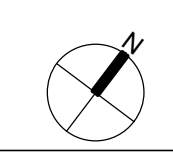
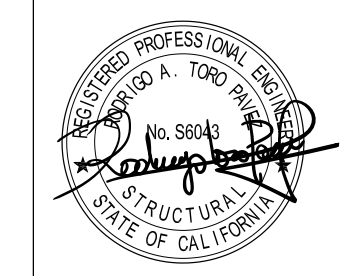


1 MODULAR GYM FOUNDATION
SCALE: 1/4" = 1'-0"

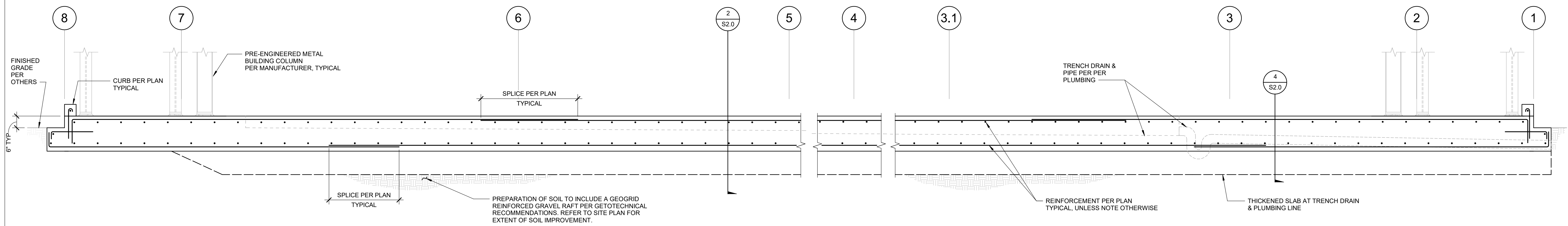
REV.	DESCRIPTION	CKD	APP	DATE
4	ADDENDUM NO. 4			6/22/26
3	PLAN CHECK 3			4/17/26
2	PLAN CHECK 2			2/17/26
1	PLAN CHECK 1			8/11/25

PUBLIC WORKS DEPARTMENT		ENGINEERING DIVISION	
CITY OF SAN BUENAVENTURA			
INTERIM FIRE STATION NO. 7			
MODULAR GYM FOUNDATION			
DRN. BY:	Author	DES. BY:	Designer
		CKD. BY:	Checker
PRINCIPAL ENGINEER		DATE	
CITY ENGINEER		DATE	
SPEC. NUMBER	2025-010	PROJECT NO.	FA11-1061
SEPTEMBER 7, 2025	SHEET 26 OF 151	DWG. NO. 2025-D-010	

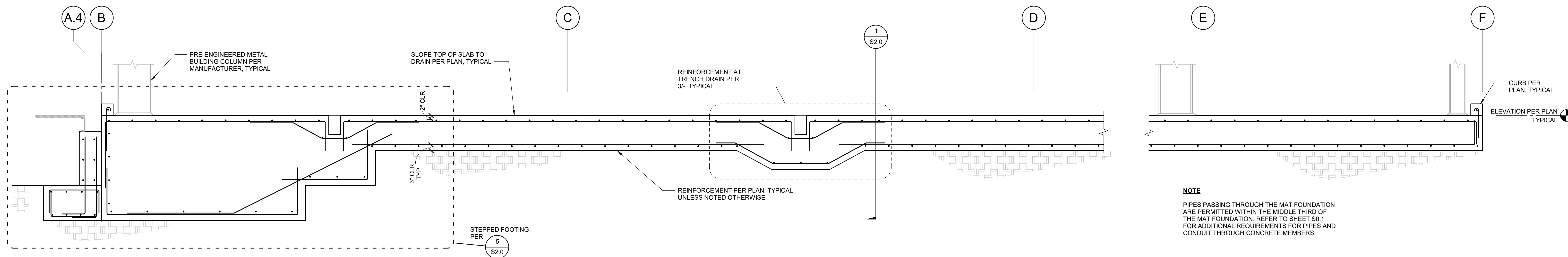
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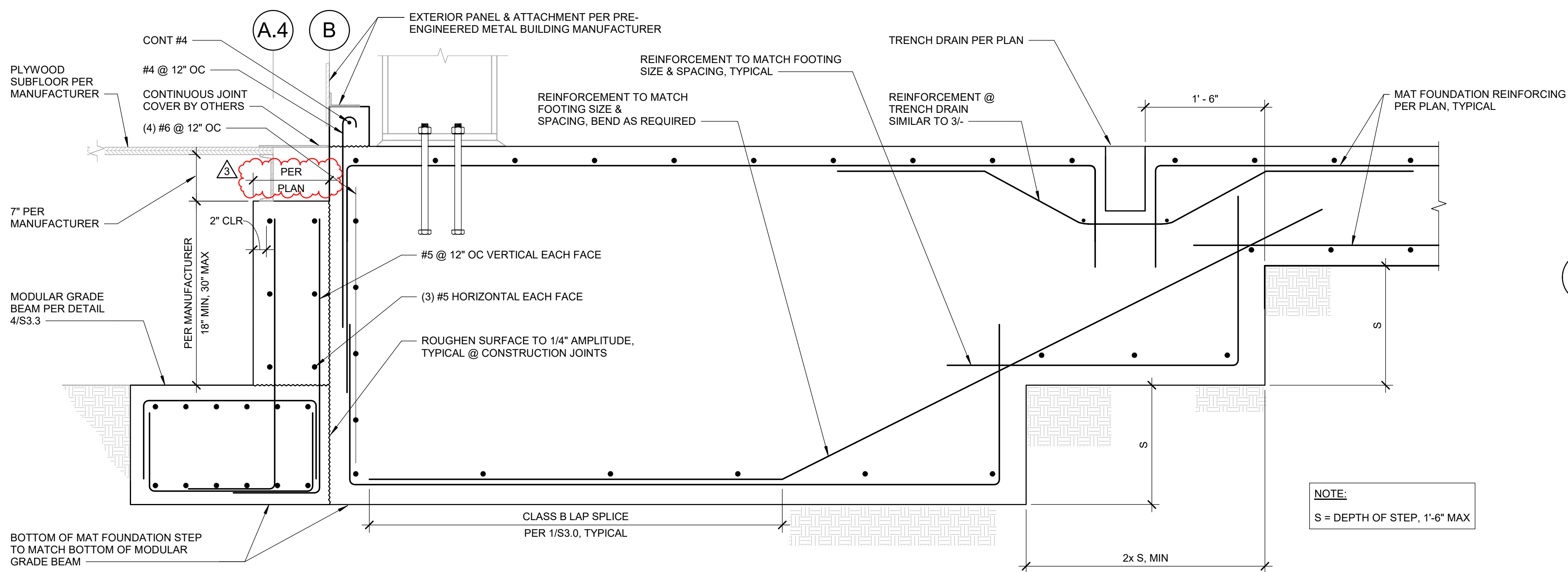
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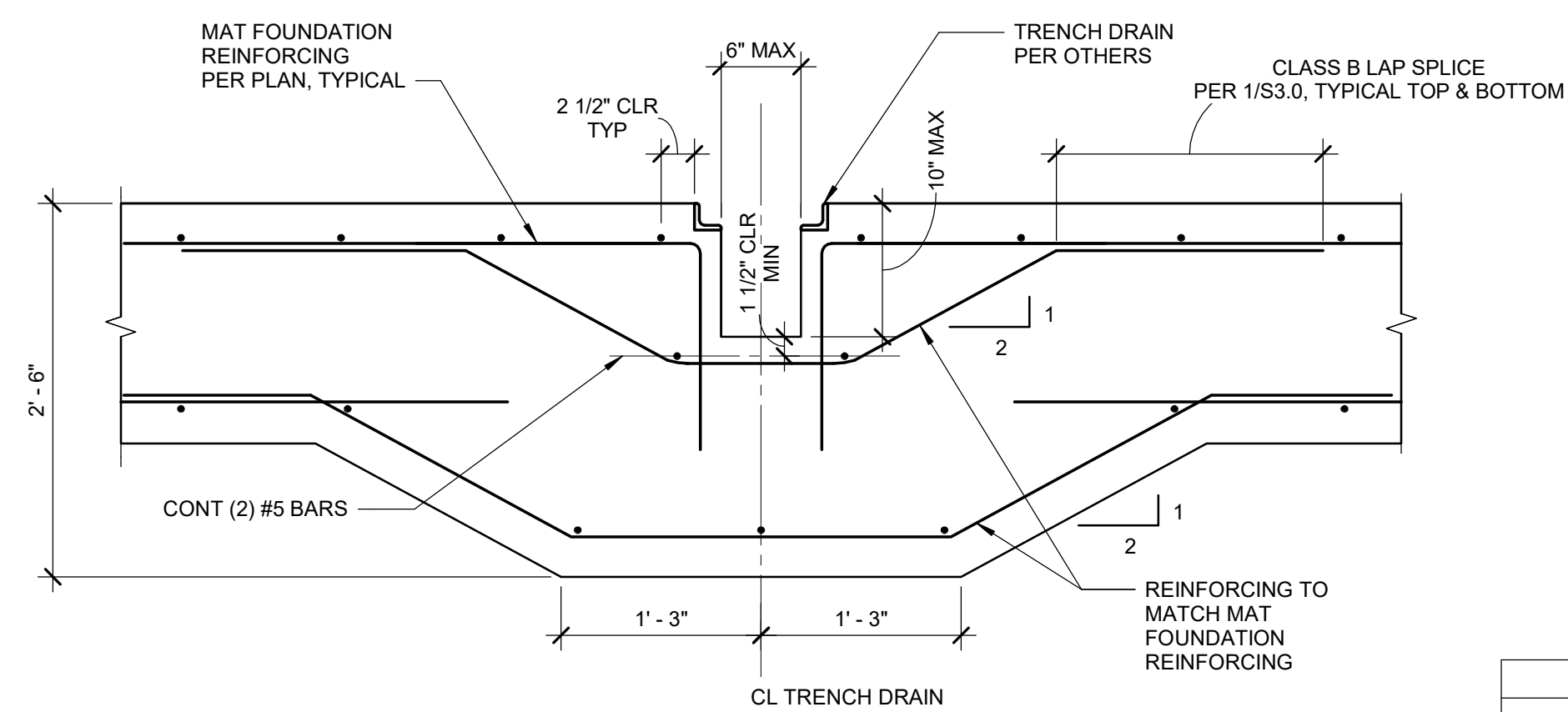
1 APPARATUS BAY MAT FOUNDATION ELEVATION
SCALE: 1/2" = 1'-0"



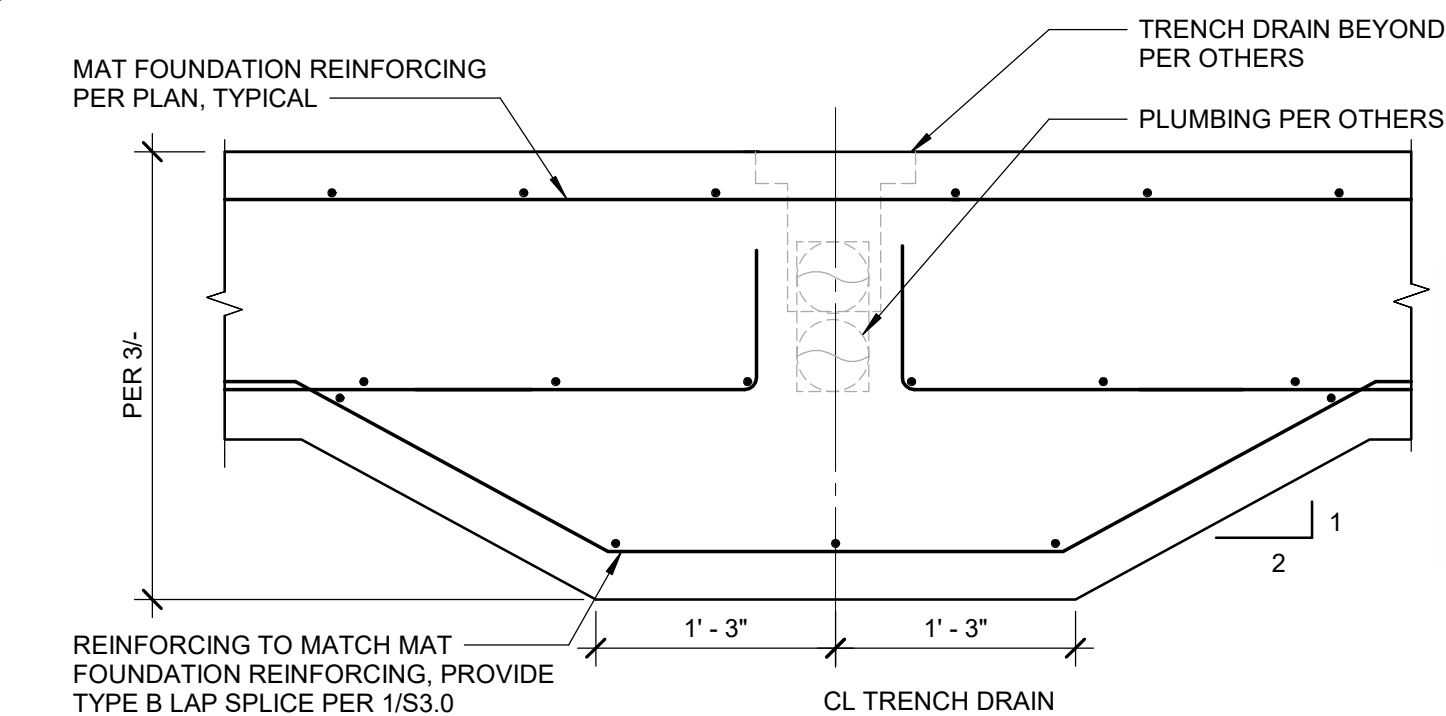
2 APPARATUS BAY & MODULAR HOUSING ELEVATION
SCALE: 1/2" = 1'-0"



5 STEPPED FOOTING DETAIL
SCALE: 1" = 1'-0"



3 TRENCH DRAIN DETAIL
SCALE: 1" = 1'-0"



4 THICKENED SLAB AT PLUMBING LINES
SCALE: 1" = 1'-0"

REV.	DESCRIPTION	CKD	APP	DATE
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1	PLAN CHECK 1			8/11/25

DRN. BY: CC DES. BY: KK CKD. BY: -

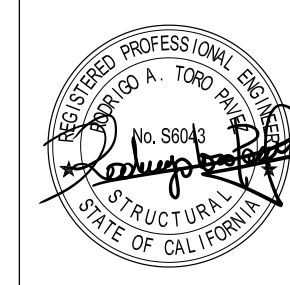
PRINCIPAL ENGINEER _____ DATE _____

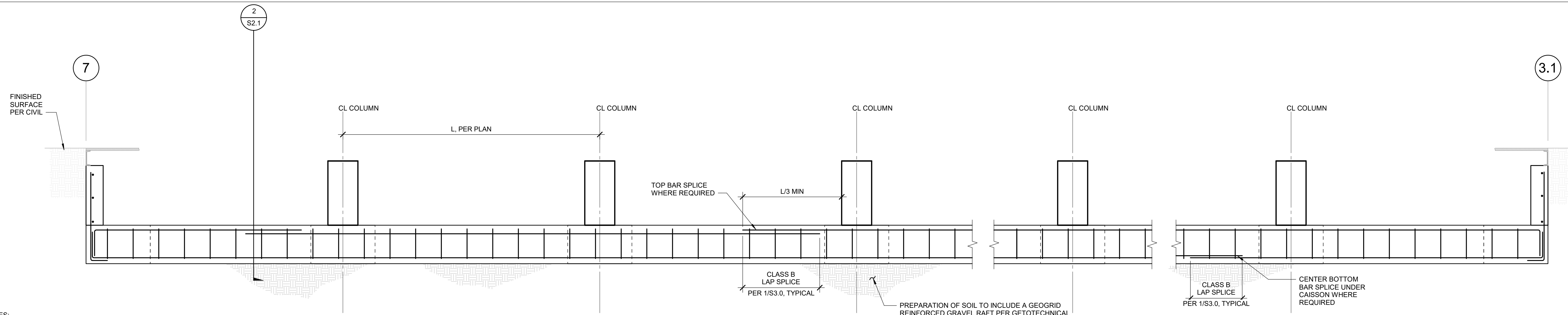
CITY ENGINEER _____ DATE _____

SPEC. NUMBER 2025-010 PROJECT NO. FA11-1061

SEPTEMBER 7, 2025 SHEET 27 OF 151 DWG. NO. 2025-D-101

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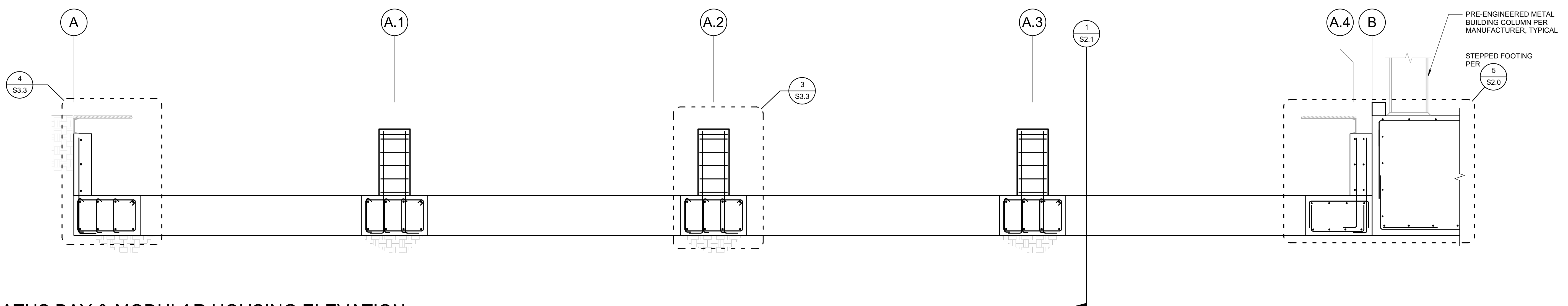




NOTES:

- 1. L = CENTER-TO-CENTER SPACING OF COLUMNS

1 APPARATUS BAY & MODULAR HOUSING ELEVATION
SCALE: 1/2" = 1'-0"

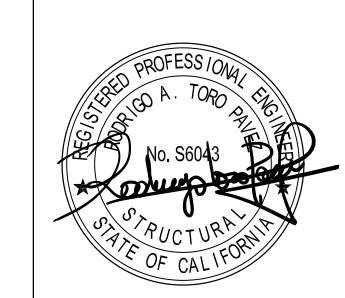


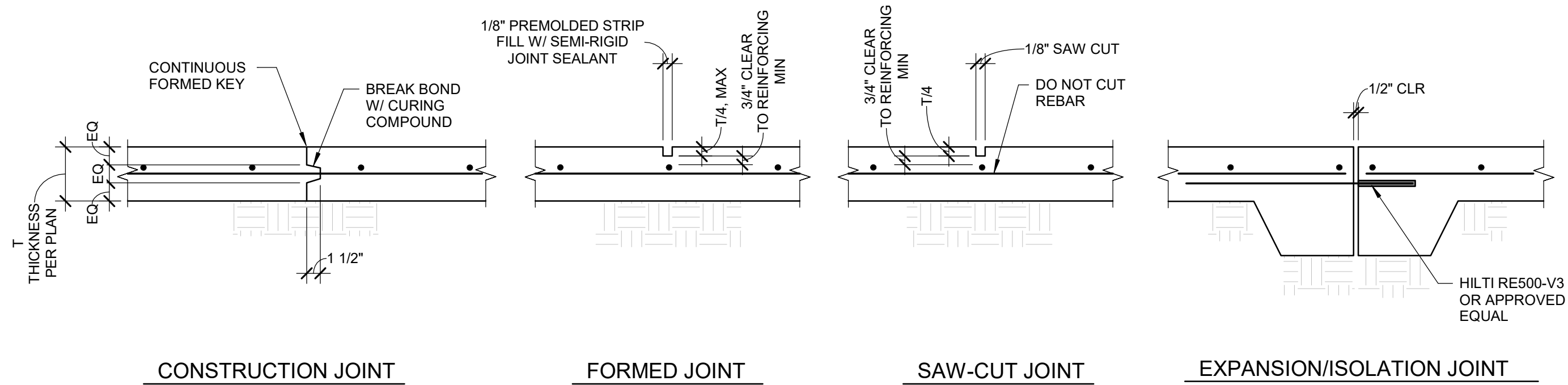
2 APPARATUS BAY & MODULAR HOUSING ELEVATION
SCALE: 1/2" = 1'-0"

REV.	DESCRIPTION	CKD	APP	DATE
4	ADDENDUM NO. 4			6/22/26
3	PLAN CHECK 3			4/17/26
2	PLAN CHECK 2			2/17/26
1	PLAN CHECK 1			8/11/25

PUBLIC WORKS DEPARTMENT		ENGINEERING DIVISION	
CITY OF SAN BUENAVENTURA			
INTERIM FIRE STATION NO. 7			
MODULAR ELEVATIONS			
DRN. BY:	CC	DES. BY:	KK
CKD. BY:			
PRINCIPAL ENGINEER		DATE	
CITY ENGINEER		DATE	
SPEC. NUMBER	2025-010	PROJECT NO.	FA11-1061
SEPTEMBER 7, 2025	SHEET 28 OF 151	DWG. NO.	2025-D-010

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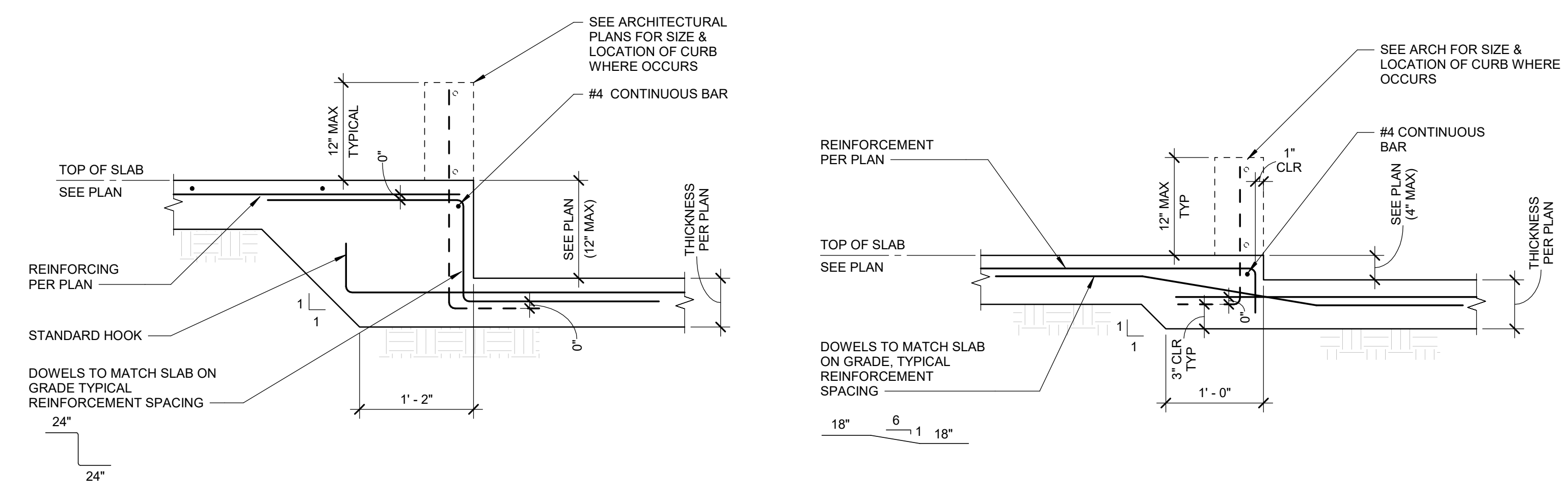


CONTROL JOINTS

- NOTES:**
- CONTROL JOINTS TO BE LOCATED AT COLUMN CENTER LINES AND AT 15'-0" MAX. AND EVERY 225 SQUARE FEET.
 - IF SAW-CUT CONTROL JOINT TO BE USED, SAW-CUT WITHIN 24 HOURS OF POUR.

4 TYPICAL SLAB ON GRADE JOINTS

SCALE: 1" = 1'-0"



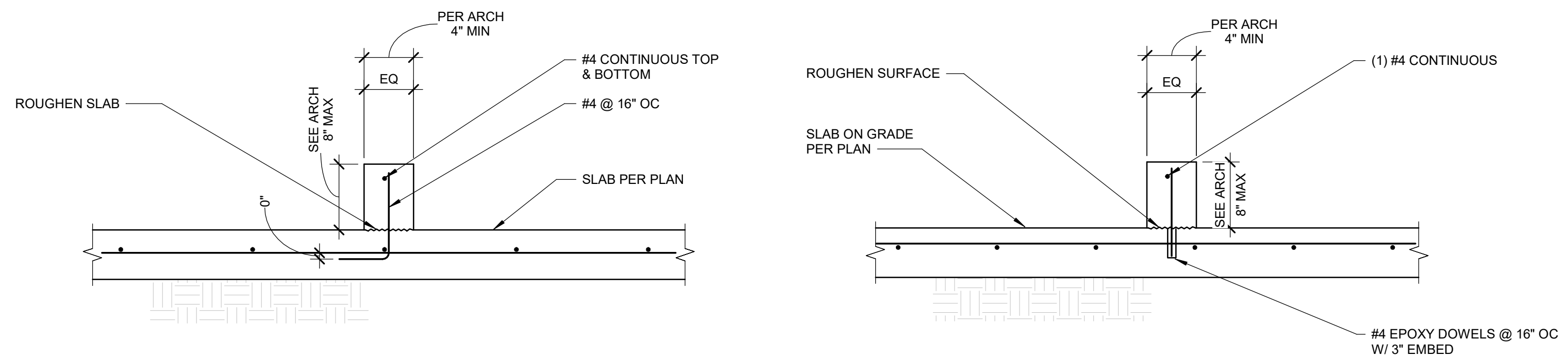
5A STEP IN SLAB ON GRADE (12" MAX)

5B STEP IN SLAB ON GRADE (4" MAX)

5 SLAB ON GRADE DEPRESSIONS

SCALE: 1" = 1'-0"

- NOTES:**
- SEE GENERAL NOTES FOR EPOXY.
 - USE NON-DESTRUCTIVE METHODS TO LOCATE EXISTING REINFORCING BARS. DO NOT CUT EXISTING REINFORCING BARS WHICH MAY CONFLICT WITH ANCHOR LOCATION.



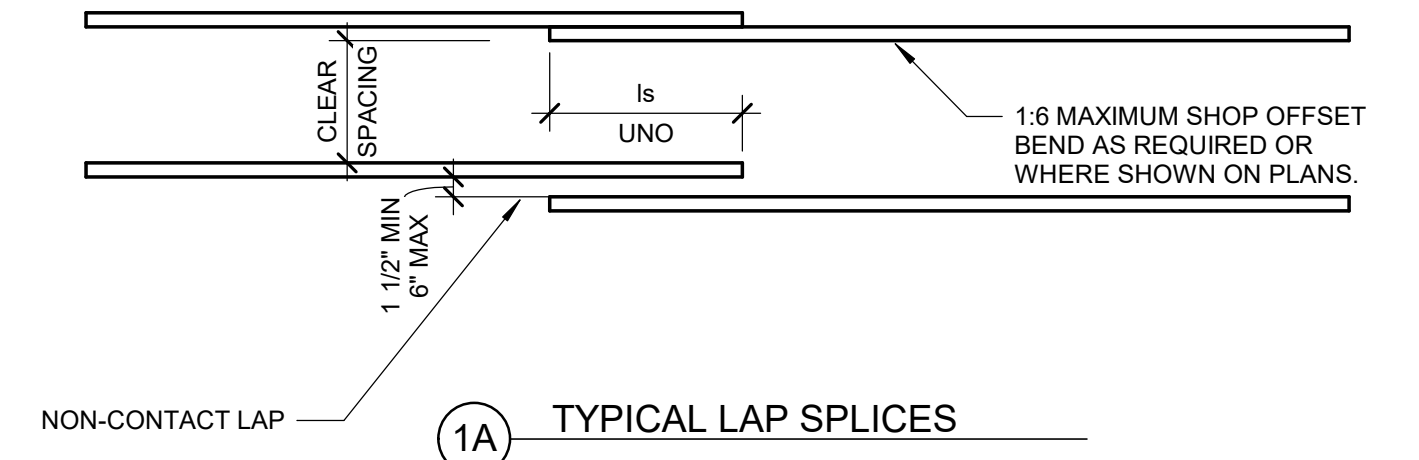
6A CURB AT SLAB ON GRADE

6B EPOXY DOWELED CONCRETE CURB

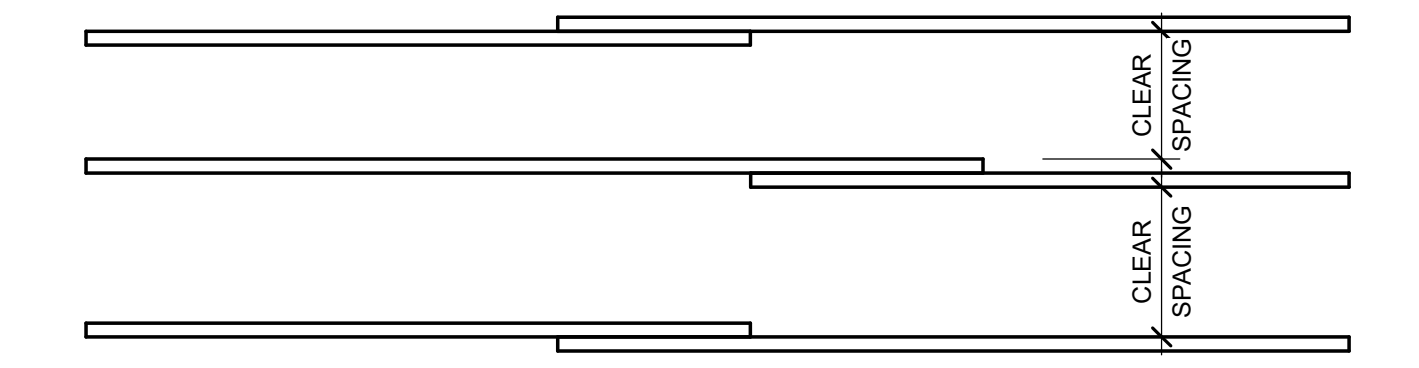
6 TYPICAL CURB DETAILS

SCALE: 1" = 1'-0"

REBAR LAP SPlice LENGTH SCHEDULE					
BAR SIZE	LAP CLASS	NORMAL WEIGHT CONCRETE			
		f _c = 4000 psi			
		Top Bars		Other Bars	
		Case 1	Case 2	Case 1	Case 2
3	A	1'-7"	2'-4"	1'-3"	1'-10"
	B	2'-1"	3'-1"	1'-7"	2'-4"
4	A	2'-1"	3'-1"	1'-7"	2'-5"
	B	2'-9"	4'-1"	2'-1"	3'-1"
5	A	2'-7"	3'-11"	2'-0"	3'-0"
	B	3'-5"	5'-1"	2'-7"	3'-11"
6	A	3'-1"	4'-8"	2'-5"	3'-7"
	B	4'-1"	6'-1"	3'-1"	4'-8"
7	A	4'-6"	6'-9"	3'-6"	5'-3"
	B	5'-11"	8'-10"	4'-6"	6'-9"
8	A	5'-2"	7'-9"	4'-0"	6'-0"
	B	6'-9"	10'-1"	5'-2"	7'-9"
9	A	5'-10"	8'-9"	4'-6"	6'-9"
	B	7'-7"	11'-4"	5'-10"	8'-9"
10	A	6'-7"	9'-10"	5'-1"	7'-7"
	B	8'-6"	12'-9"	6'-7"	9'-10"
11	A	7'-3"	10'-11"	5'-7"	8'-5"
	B	9'-6"	14'-2"	7'-3"	10'-11"



1A TYPICAL LAP SPLICES



1B STAGGERED SPLICING

- NOTES:**
- CASES 1 AND 2 WHICH DEPEND ON CLEAR CONCRETE COVER AND THE CENTER-TO-CENTER SPACING OF THE BARS ARE DEFINED AS:
CASE 1: COVER AT LEAST 1db AND CLEAR SPACING AT LEAST 2db.
CASE 2: COVER LESS THAN 1db OR CLEAR SPACING LESS THAN 2db.
 - TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.
 - OTHER BARS INCLUDE VERTICAL BARS AND HORIZONTAL BARS WITH LESS THAN 12 INCHES OF CONCRETE CAST BELOW HORIZONTAL BARS.
 - BAR SPLICES NOT COVERED BY THIS SCHEDULE ARE SPECIFICALLY DETAILED AND DIMENSIONED PER PLANS.
 - ALL SPLICES SHALL BE CLASS 'B' UNLESS OTHERWISE ON PLANS.
 - FOR DEVELOPMENT LENGTH, L_d, USE CLASS 'A' LAP SPlice LENGTH.
 - FOR LAP SPLICES OF BARS IN SHEAR WALLS, REFER TO LAP SPlice AND DEVELOPMENT SCHEDULE IN CONCRETE WALL ELEVATIONS.
 - THIS SCHEDULE IS FOR GR 60 REINFORCEMENT.
 - CAN USE 1.25 x LAP SPlice IN LIEU OF NOTE 7 AT SHEAR WALLS.
 - FOR STAGGERED SPLICES, CLEAR SPACING SHOWN IN DIAGRAM MAY BE USED TO DETERMINE CASE 1 OR CASE 2 PER NOTE 1.

1 DEVELOPMENT AND LAP SPlice OF CONCRETE REINFORCEMENT BARS- NW CONCRETE

SCALE: 12" = 1'-0"

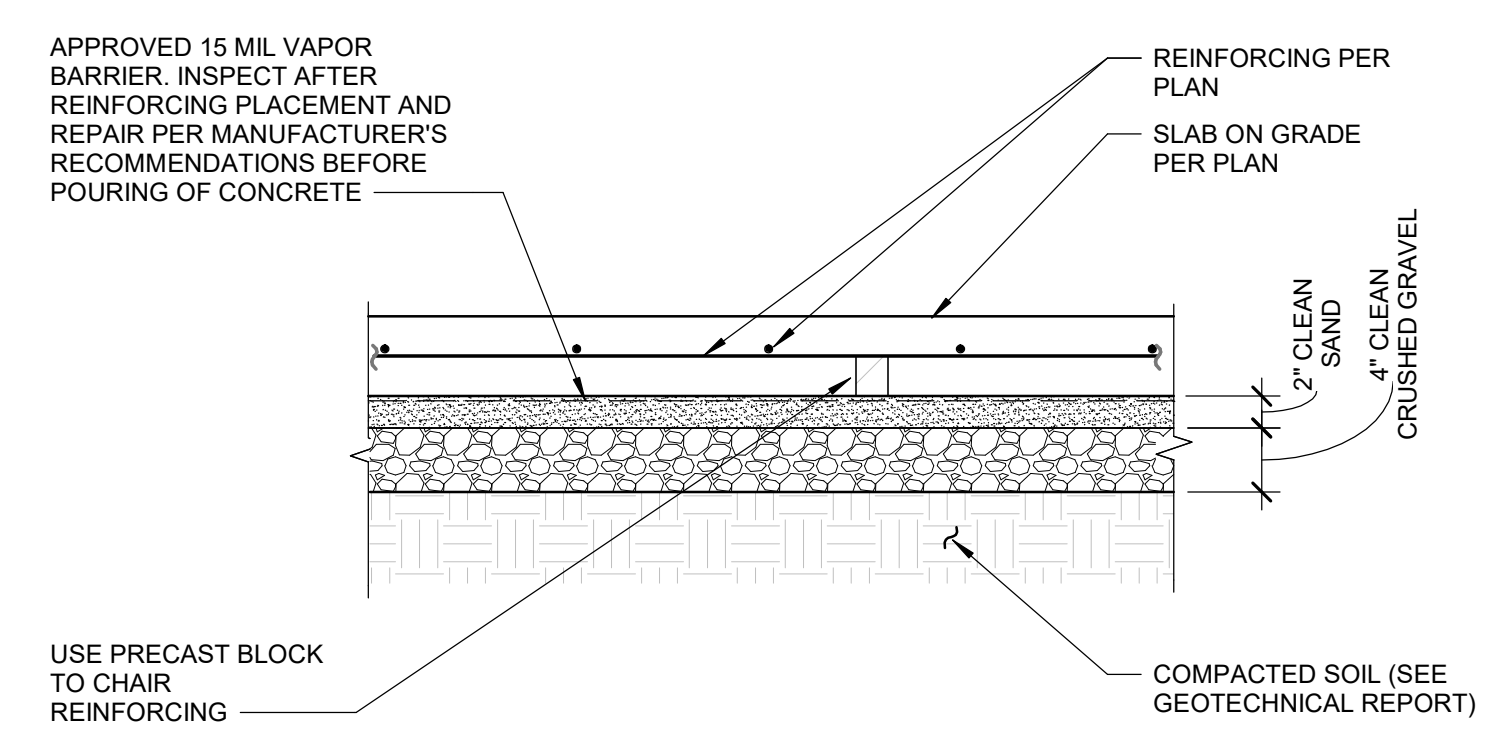
TYPE OF STANDARD HOOK	BAR SIZE	MINIMUM INSIDE BEND DIAMETER, IN	STRAIGHT EXTENSION (1) ℓ _{EXT} , IN	TYPE OF STANDARD HOOK
90-DEGREE HOOK	#3 THROUGH #8	6db	12db	POINT AT WHICH BAR IS DEVELOPED 90° BEND FACE OF CONCRETE ℓ _{EXT} 2" CLR MIN
	#9 THROUGH #11	8db		
	#14 THROUGH #18	10db		
180-DEGREE HOOK	#3 THROUGH #8	6db	GREATER OF 4db AND 2.5 IN	POINT AT WHICH BAR IS DEVELOPED 180° BEND FACE OF CONCRETE ℓ _{EXT} 2" CLR MIN
	#9 THROUGH #11	8db		
	#14 THROUGH #18	10db		

STANDARD HOOK DEVELOPMENT LENGTH "ℓ _{db} "			
BAR SIZE	D	"ℓ _{db} "	NORMAL WEIGHT
#3	2 1/4"	5"	0'-8"
#4	3"	8"	0'-10"
#5	3 3/4"	10	1'-0"
#6	4 1/2"	12	1'-3"
#7	5 1/4"	1'-2"	1'-5"
#8	6"	1'-4"	1'-7"
#9	9 1/2"	1'-7 1/2"	1'-10"
#10	10 3/4"	1'-10"	2'-1"
#11	12"	2'-0 1/2"	2'-3"

- NOTES:**
- ACI 318-14 TABLE 25.3.2.
 - ALL HOOKED BARS SHALL EXTEND AS FAR AS POSSIBLE WITH A MINIMUM 2" END COVER AND WITH EMBEDMENT NOT LESS THAN SHOWN ON THE SCHEDULE. UNO ON PLANS.

2 STANDARD HOOK DEVELOPMENT LENGTH BENDING DETAIL

SCALE: 1" = 1'-0"



- NOTES:**
- REFER TO GEOTECHNICAL REPORT FOR ADDITIONAL REQUIREMENTS.
 - CONTRACTOR SHALL TAKE CARE NOT TO PUNCTURE VAPOR RETARDER DURING CONSTRUCTION OF SLAB ON GRADE.
 - PROVIDE VAPOR RETARDER AT AREAS AND WITH EXTENTS/TERMINATIONS AS SPECIFIED BY ARCHITECT.
 - VAPOR RETARDER AND/OR GRAVEL LAYER MAY BE OMITTED AT PARKING/DRIVE ISLE AREAS. GEOTECHNICAL ENGINEER SHALL CONFIRM BASED ON FIELD TESTING OF IN-SITU SOILS.

3 TYPICAL VAPOR BARRIER DETAIL

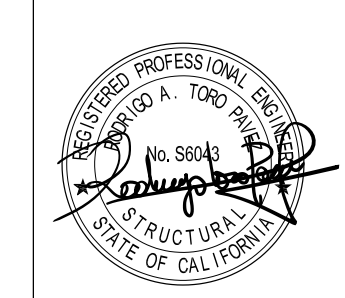
SCALE: 1" = 1'-0"

REV.	DESCRIPTION	CKD	APP	DATE
4	ADDENDUM NO. 4			6/22/26
3	PLAN CHECK 3			4/17/26
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1	PLAN CHECK 1			8/11/25

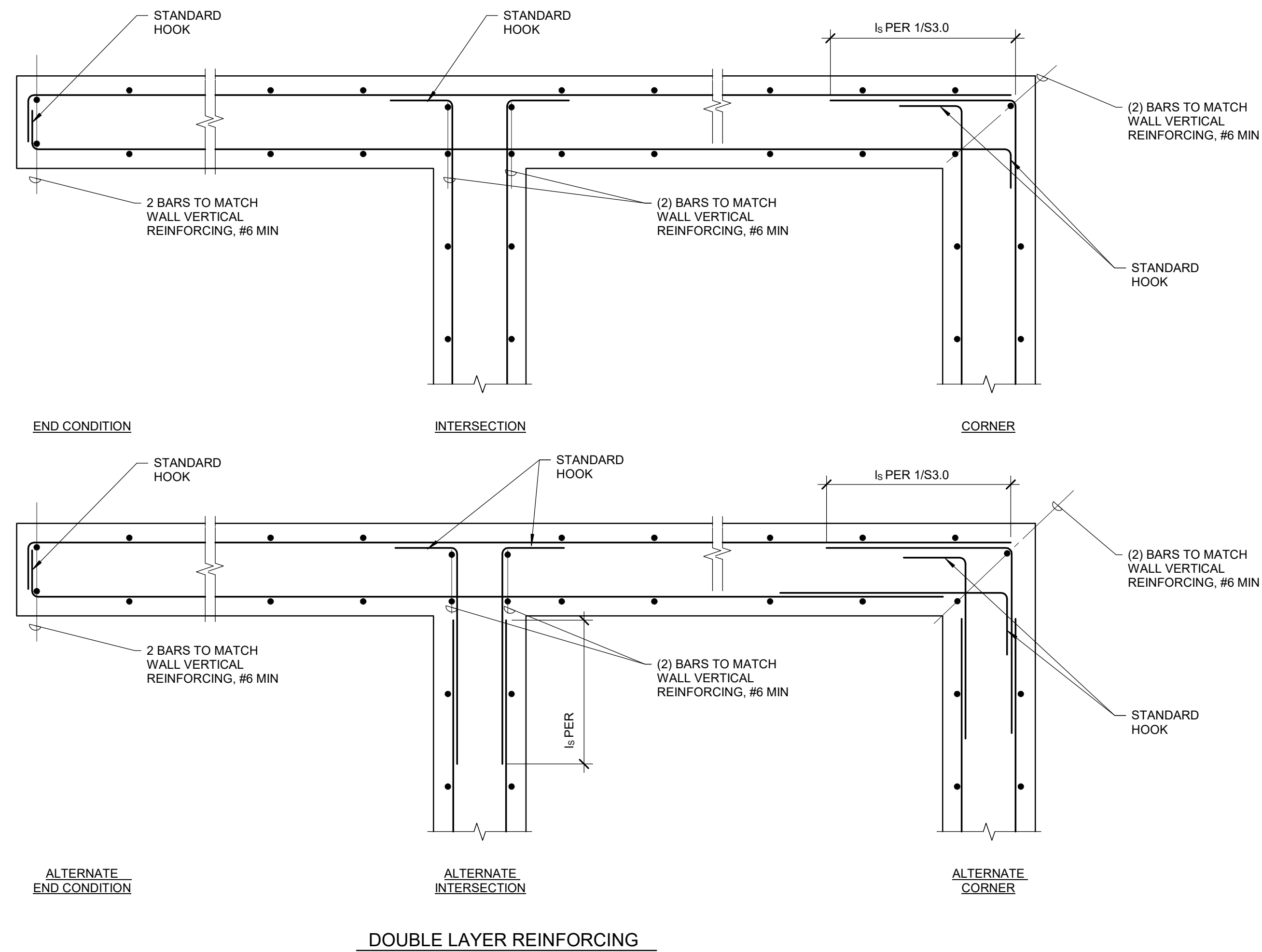
DRN. BY: CC DES. BY: KK CKD. BY: -

PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION
CITY OF SAN BUENAVENTURA
INTERIM FIRE STATION NO. 7
TYPICAL CONCRETE DETAILS

PRINCIPAL ENGINEER _____ DATE _____
 CITY ENGINEER _____ DATE _____
 SPEC. NUMBER 2025-010 PROJECT NO. FA11-1061
 SEPTEMBER 7, 2025 SHEET 29 OF 151 DWG. NO. 2025-D-010



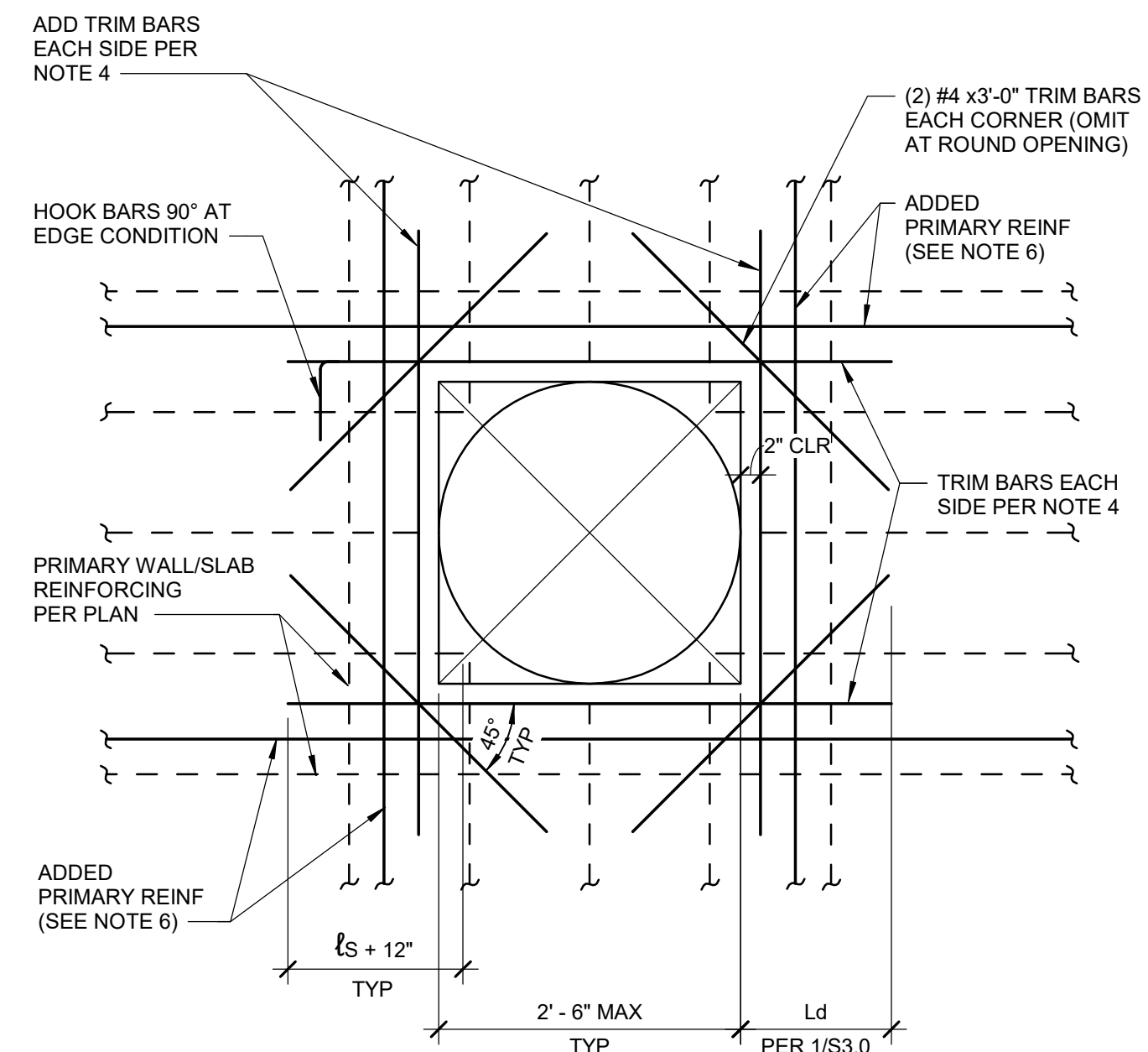
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NOTE:
REINFORCEMENT SHOWN ON WALL ELEVATIONS AND OTHER SPECIFICALLY REFERENCED DETAILS TAKE PRECEDENCE OVER REINFORCEMENT SHOWN HERE.

4 PLAN - CONCRETE WALL & FOOTING DETAIL

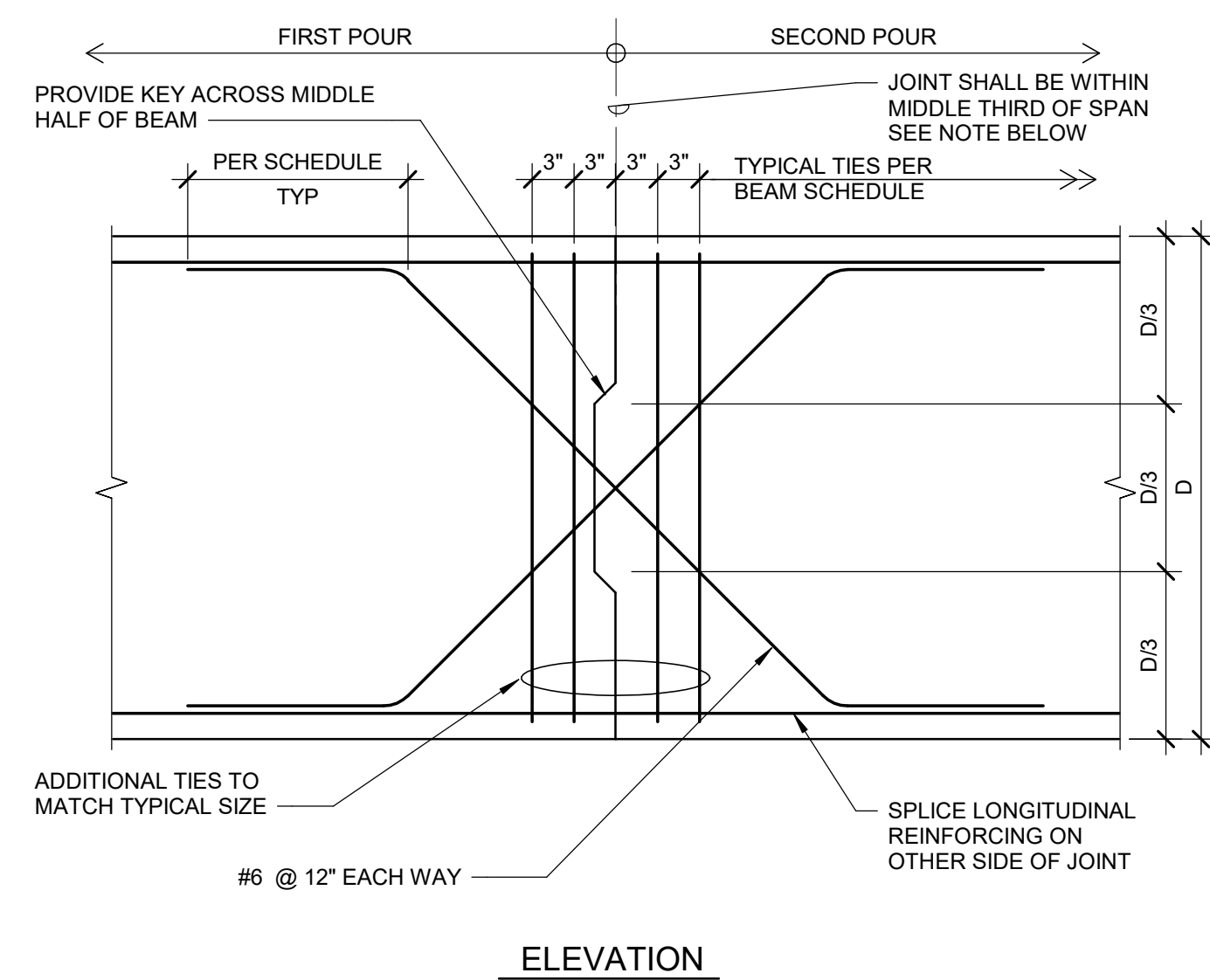
SCALE: 1" = 1'-0"



- NOTES:
- THIS DETAIL INDICATES MINIMUM REINFORCING FOR OPENING NOT DETAILED ELSEWHERE IN THESE DRAWINGS.
 - CLUSTERS OF SMALL HOLES WHOSE OVERALL MEASUREMENT EXCEEDS 1'-0" SHOULD BE REINFORCED AS ONE OPENING.
 - NO OPENING IN SLAB TO BE LOCATED CLOSER THAN 2'-0" CLEAR FROM COLUMN FACE UNLESS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
 - TRIM BARS AT EACH SIDE OF OPENING TO MATCH SIZE AND QUANTITY OF PRIMARY REINFORCEMENT BARS INTERRUPTED DUE TO OPENING, MIN (2) #5 EACH SIDE (EACH FACE FOR WALLS, TOP & BOTTOM FOR SLABS).
 - ANY OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE STRUCTURAL ENGINEER OF RECORD'S ATTENTION BY THE CONTRACTOR PRIOR TO PERFORMING WORK.
 - ADD REINFORCING EQUAL TO INTERRUPTED REINFORCING ON EACH SIDE OF OPENING, TYPICAL. SPACE ADD BARS AT 3" ON CENTER.

6 CONC WALL/SLAB OPENING DETAIL

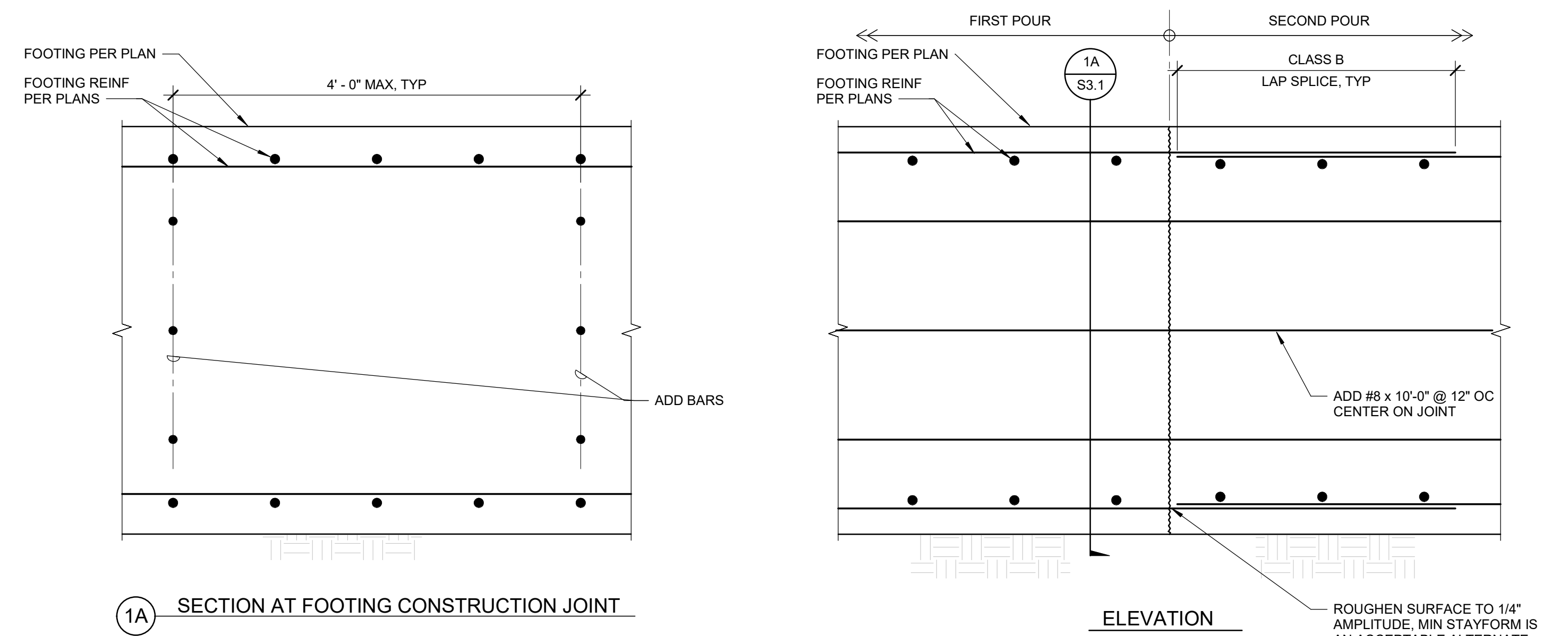
SCALE: 3/4" = 1'-0"



NOTE:
CONTRACTOR TO SUBMIT LOCATIONS OF CONSTRUCTION JOINTS TO THE STRUCTURAL ENGINEER FOR APPROVAL.

5 CONC BEAM CONSTRUCTION JOINT

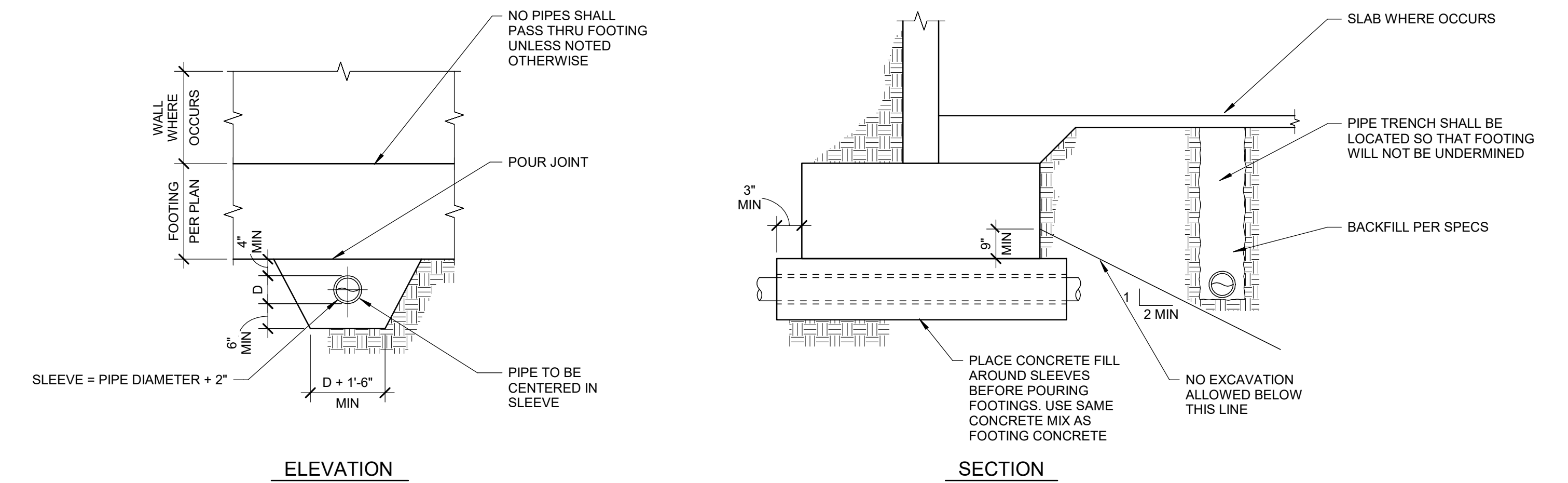
SCALE: 1" = 1'-0"



- NOTES:
- LOCATE CONSTRUCTION JOINT IN MIDDLE 1/3 OF THE DISTANCE BETWEEN COLUMNS.
 - CONTRACTOR TO SUBMIT LOCATIONS OF CONSTRUCTION JOINTS TO SEOR FOR APPROVAL.

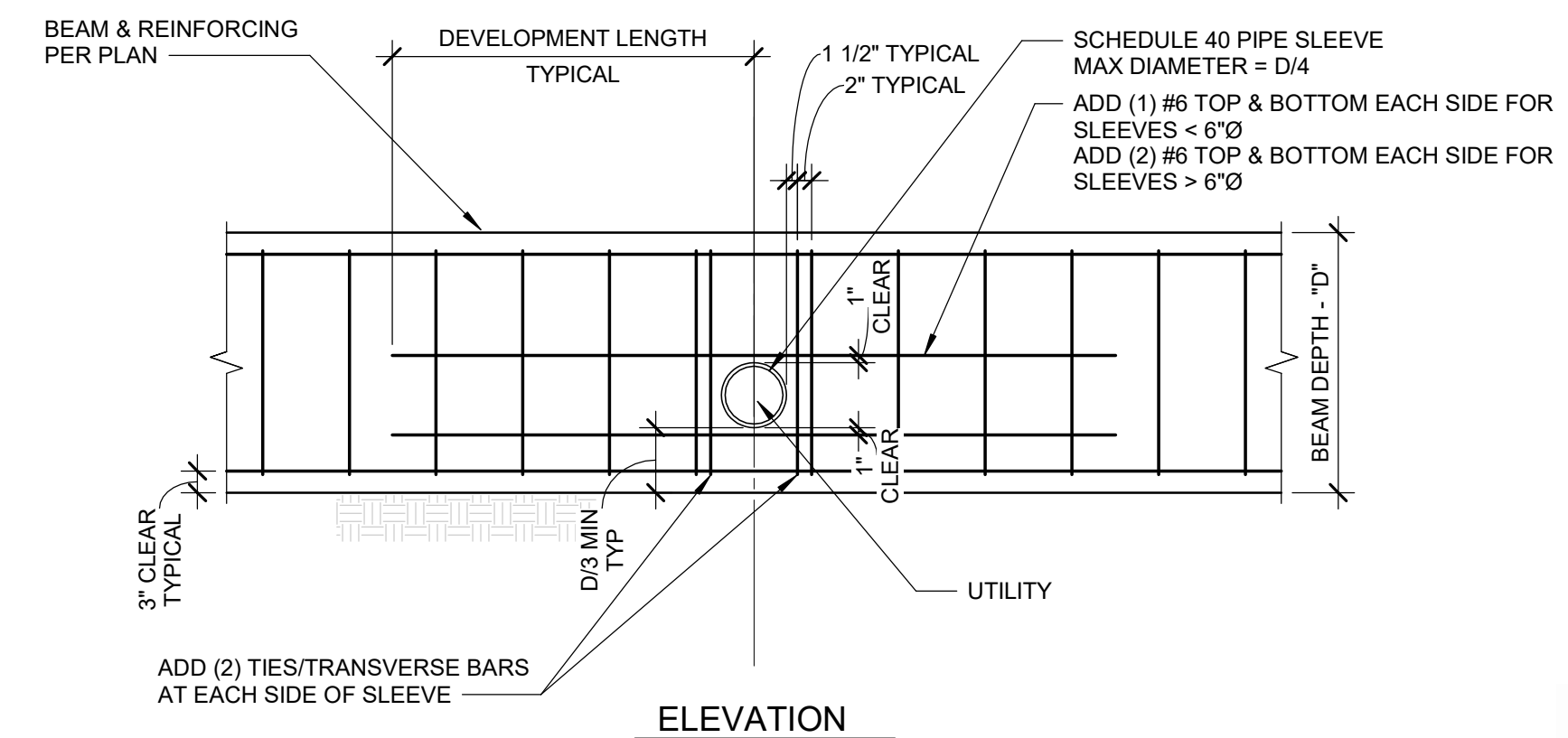
1 CONSTRUCTION JOINT AT MAT FOUNDATION

SCALE: 1" = 1'-0"



2 PIPE TRENCH BELOW & ADJACENT TO FOOTING DETAIL

SCALE: 1" = 1'-0"



- NOTES:
- SEE PLANS FOR SIZE & LOCATION OF SLEEVES.
 - PENETRATIONS NOT SHOWN ON THE DOCUMENTS MUST BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
 - PIPE SLEEVE MUST PROVIDE 1" CLEAR MIN AROUND UTILITY LINE.
 - WHERE SLEEVE IS OUTSIDE D/3 ALERT STRUCTURAL ENGINEER OF RECORD.

3 GRADE BEAM PENETRATION DETAIL

SCALE: 1/2" = 1'-0"

REV.	DESCRIPTION	CKD	APP	DATE
4	ADDENDUM NO. 4			6/22/26
3	PLAN CHECK 3			4/17/26
2	PLAN CHECK 2			2/17/26
1	PLAN CHECK 1			8/11/25

PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION
CITY OF SAN BUENAVENTURA
INTERIM FIRE STATION NO. 7
TYPICAL CONCRETE DETAILS

DRN. BY: CC DES. BY: KK CKD. BY: -

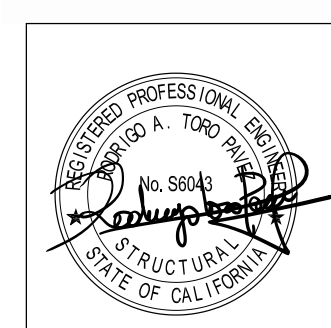
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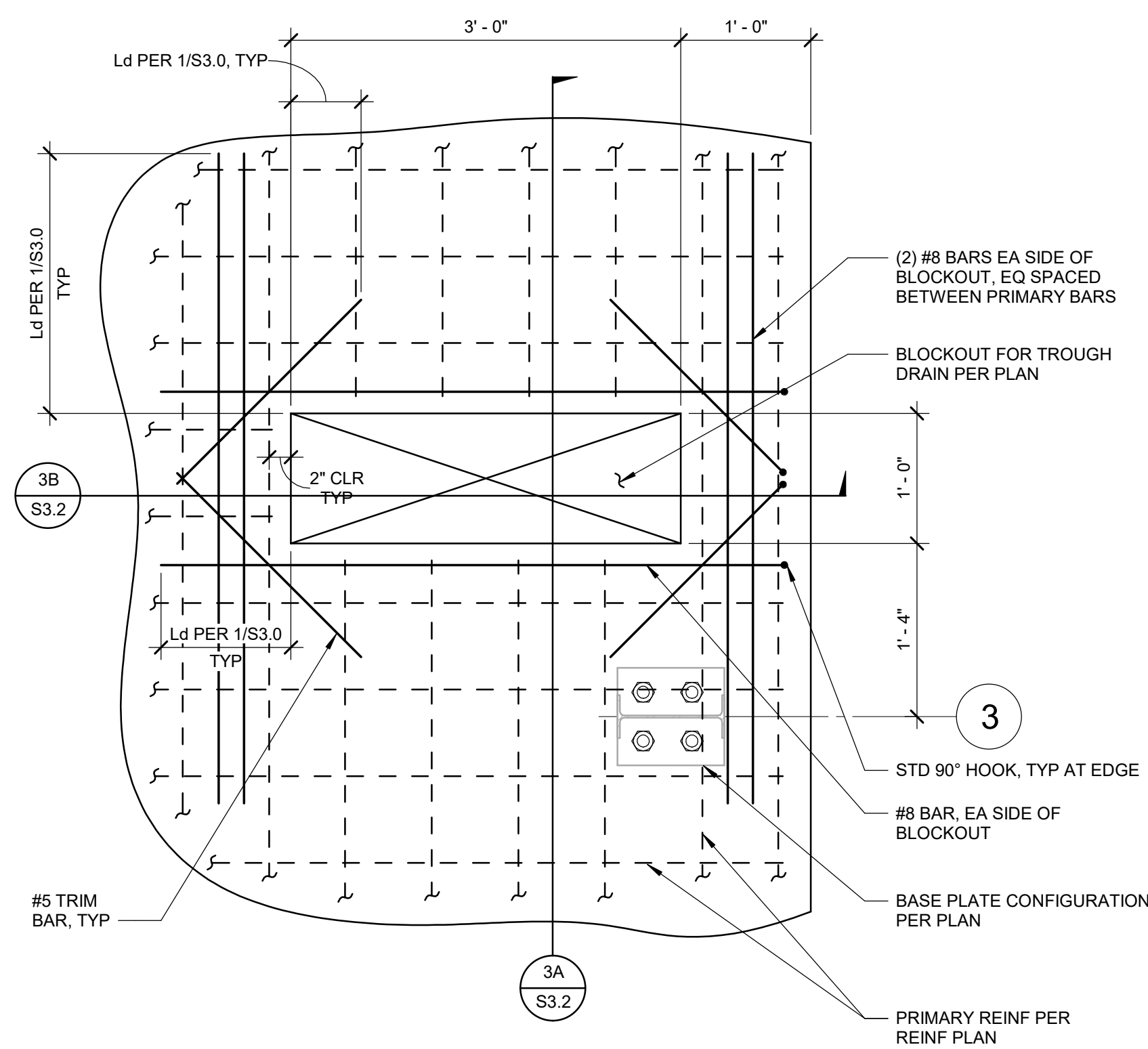
CITY ENGINEER DATE

SPEC. NUMBER 2025-010 PROJECT NO. FA11-1061

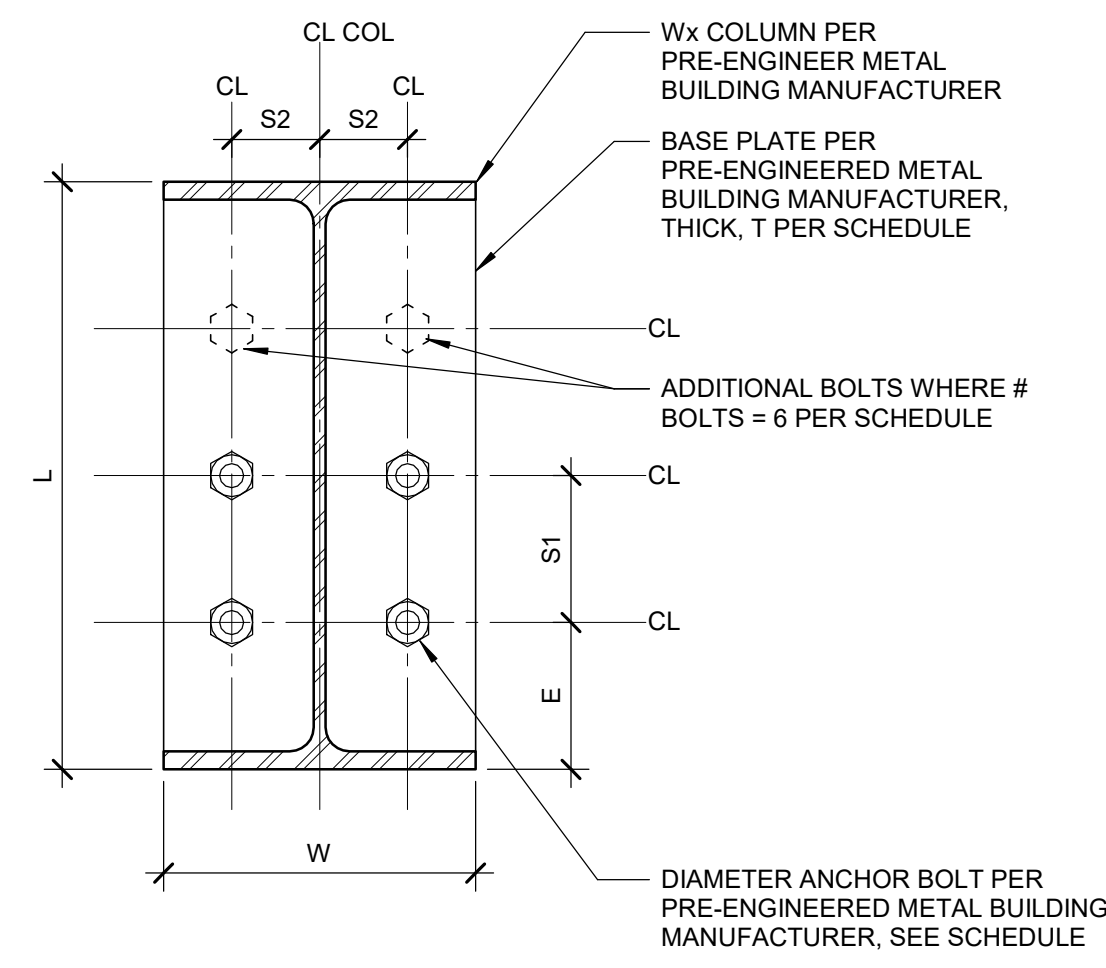
SEPTEMBER 7, 2025 SHEET 30 OF 151 DWG. NO. 2025-D-010

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3C TOP MAT REINFORCING PLAN
1" = 1'-0"



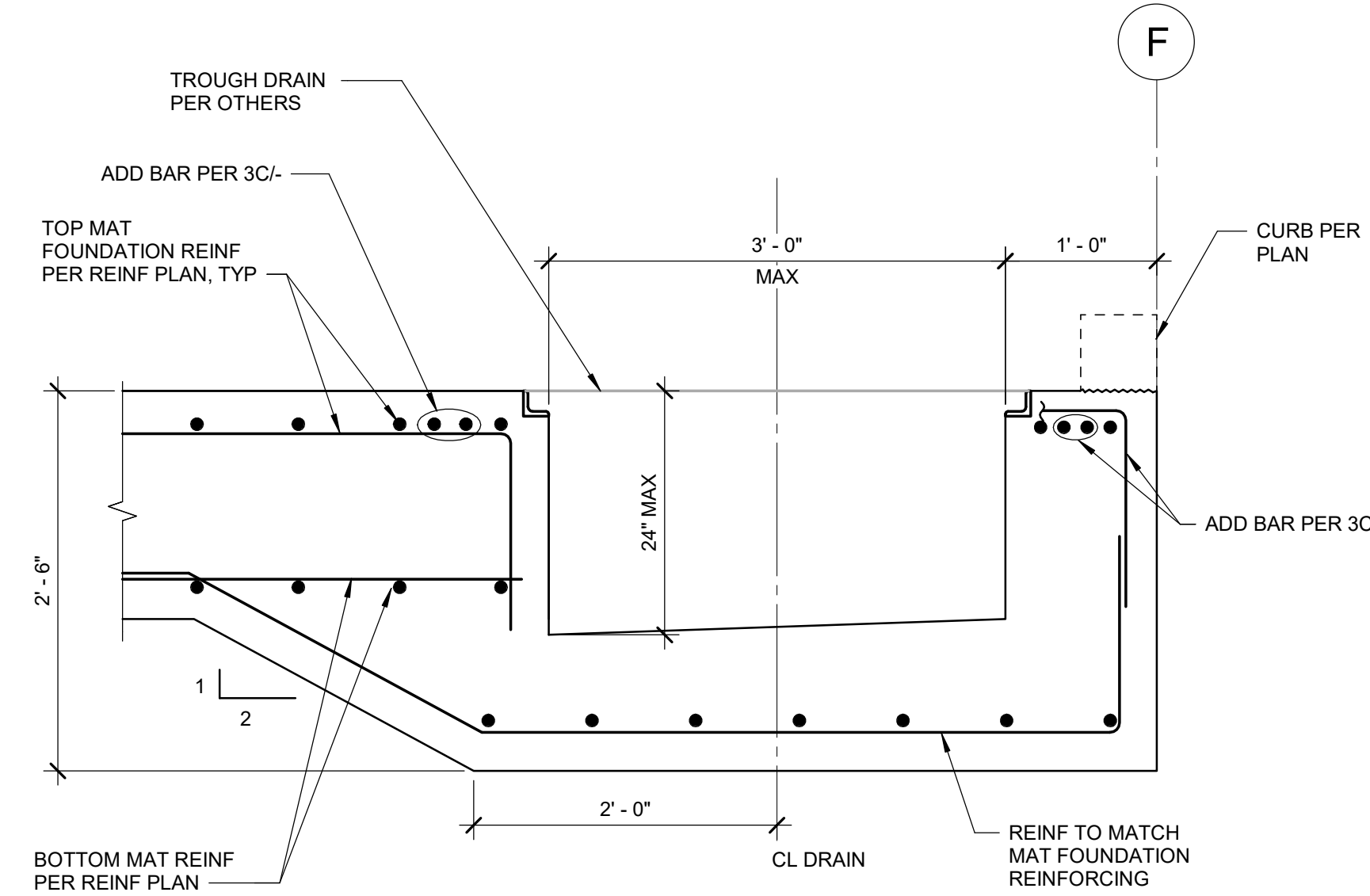
1D TYPICAL BASE PLATE PLAN
3" = 1'-0"

BASE PLATE SCHEDULE

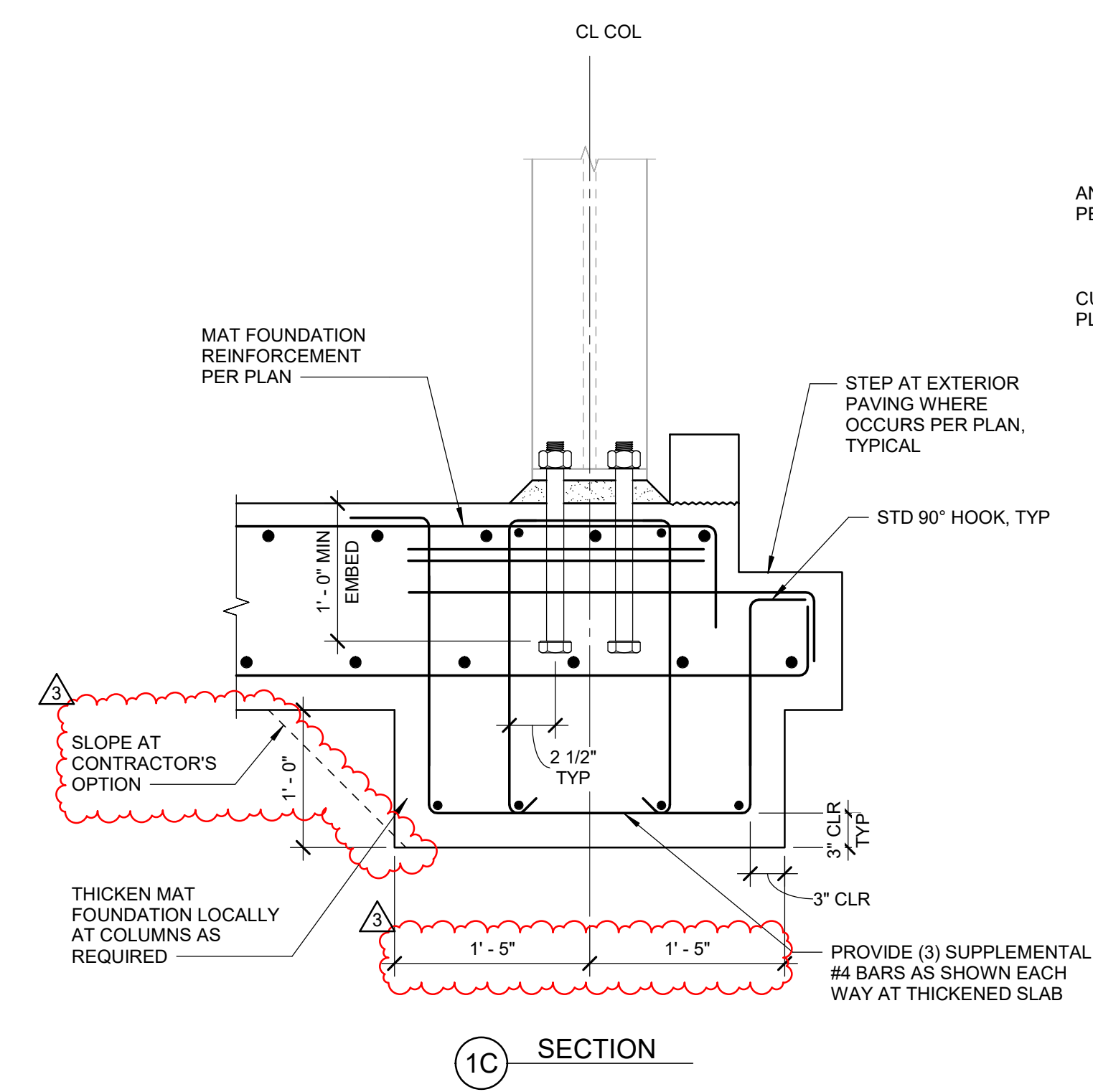
	L	W	E	S1	S2	S3	S4	ANCHOR BOLTS
C1	8"	5"	2"	4"	1 1/2"	10"	N/A	(4) 5/8" Ø HEX HEAD GR 36
C2	17"	10"	3"	5 1/2"	2 3/4"	11"	N/A	(6) 1 3/8" Ø HEAVY HEX HEAD GR 105
C3	16"	10"	3"	5 1/2"	2 3/4"	11"	N/A	(6) 1 1/4" Ø HEAVY HEX HEAD GR 105
C4	17"	10"	3"	5 1/2"	2 3/4"	11"	9 1/4"	(6) 1 1/4" Ø HEAVY HEX HEAD GR 105
C5	18"	11"	3"	6"	3"	11"	N/A	(6) 1 1/2" Ø HEAVY HEX HEAD GR105
C6	9 7/8"	6"	3"	4"	2"	11"	9"	(4) 3/4" Ø HEX HEAD GR 55
C7	9 7/8"	6"	3"	4"	2"	11"	N/A	(4) 3/4" Ø HEX HEAD GR 55
C8	8"	6"	2"	4"	2"	10"	10"	(4) 3/4" Ø HEX HEAD GR 36
C9	10 3/16"	6"	3"	4"	2"	11"	10"	(4) 3/4" Ø HEX HEAD GR 36
C10	9 7/8"	9"	3"	4 1/2"	2 1/4"	11"	N/A	(4) 1 1/8" Ø HEX HEAD GR 105

NOTES:

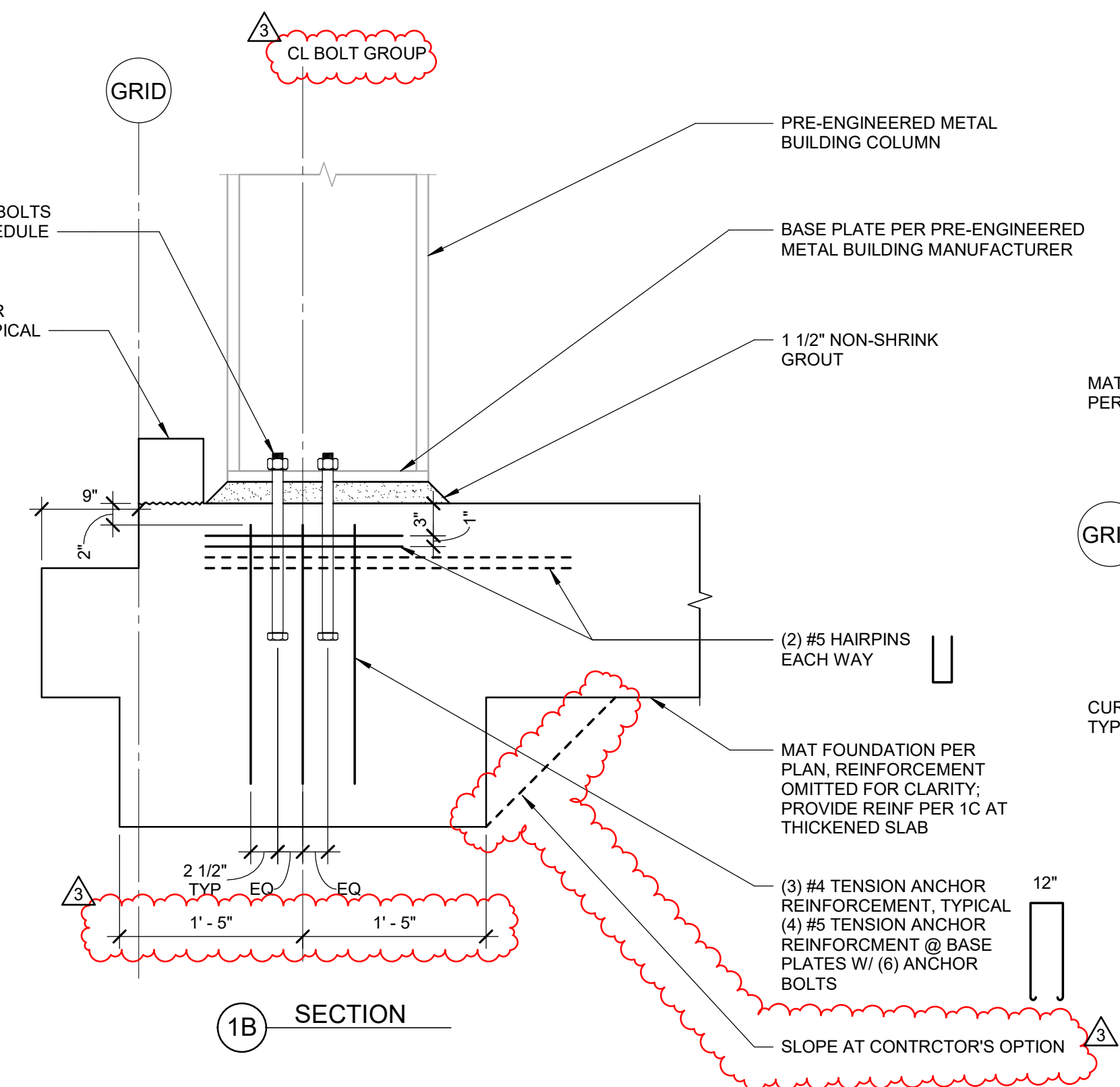
- FOUNDATION DESIGN ASSUMPTIONS BASED ON TOP COLUMN BEING BRACED AT THE ROOF.
- SEE NOTE #7 ON PLAN NOTES.
- ANCHOR REINFORCING CONSISTS OF BOTH HAIRPINS & TENSION ANCHOR REINFORCEMENT.
- REFER TO PRE-ENGINEERED METAL BUILDING DRAWINGS FOR BASE PLATE ORIENTATION.
- ALL ANCHOR BOLTS SHALL BE ASTM F 1554, GALVANIZED.



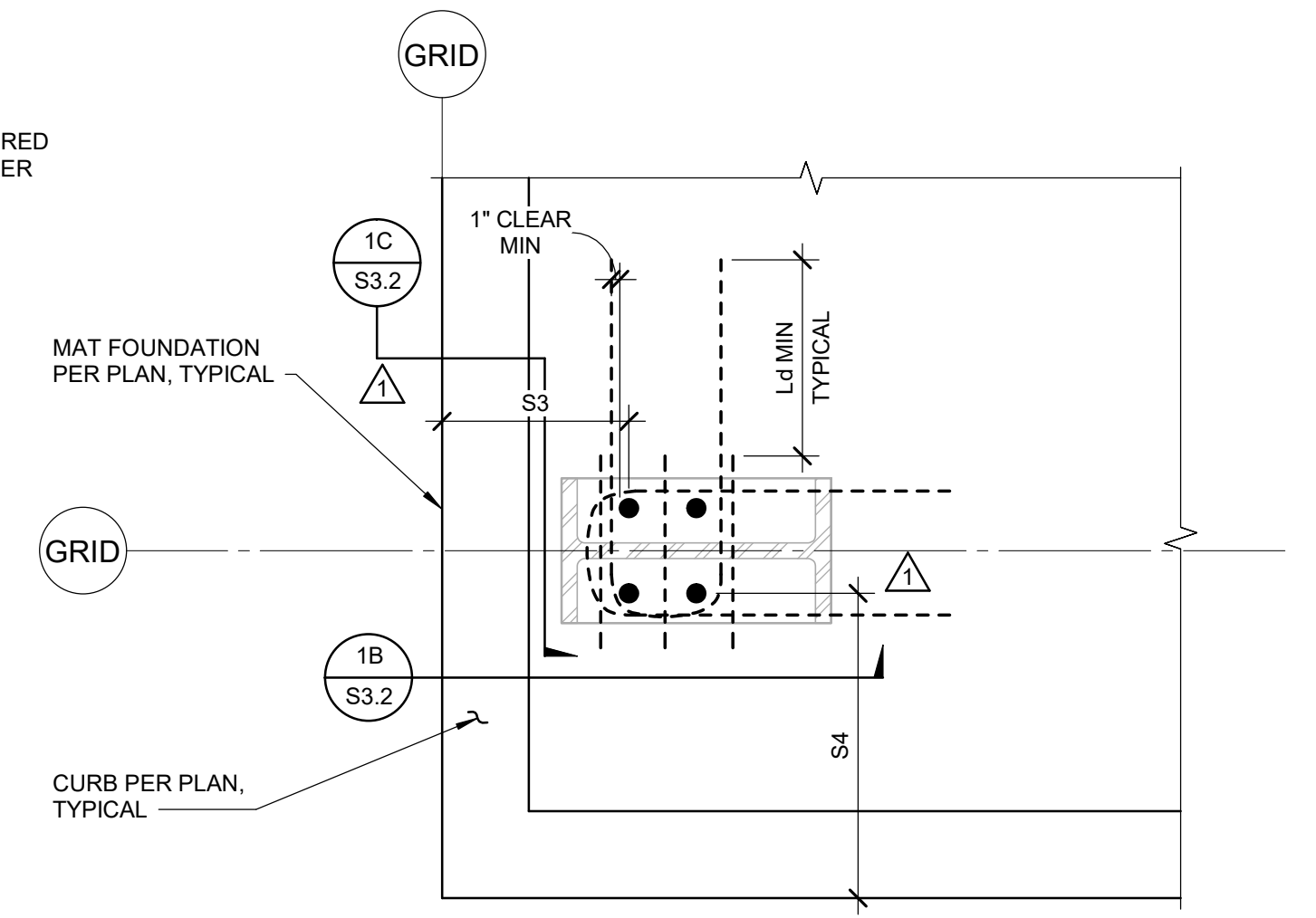
3B TROUGH DRAIN SECTION
1" = 1'-0"



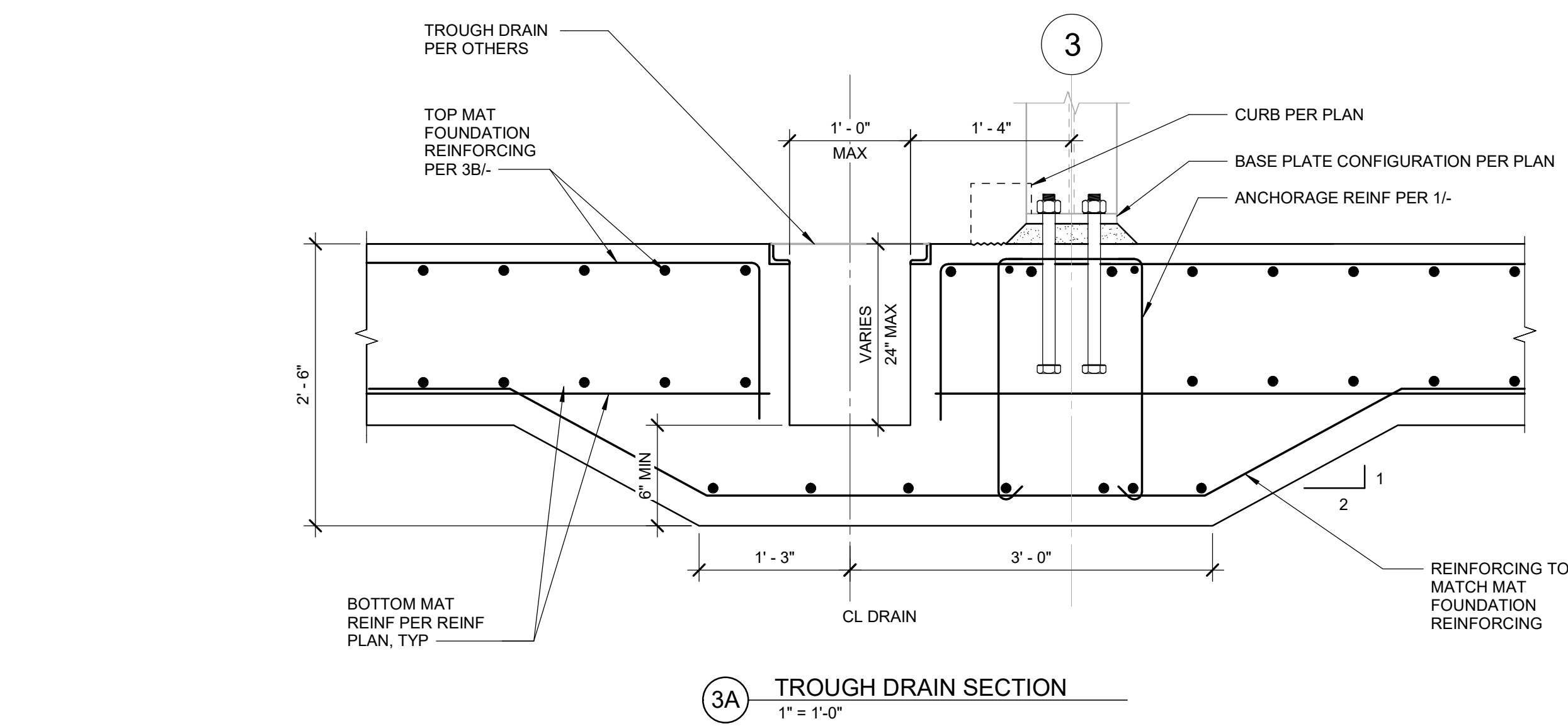
1 FRAME BASE PLATE DETAIL
SCALE: 1" = 1'-0"



1B SECTION



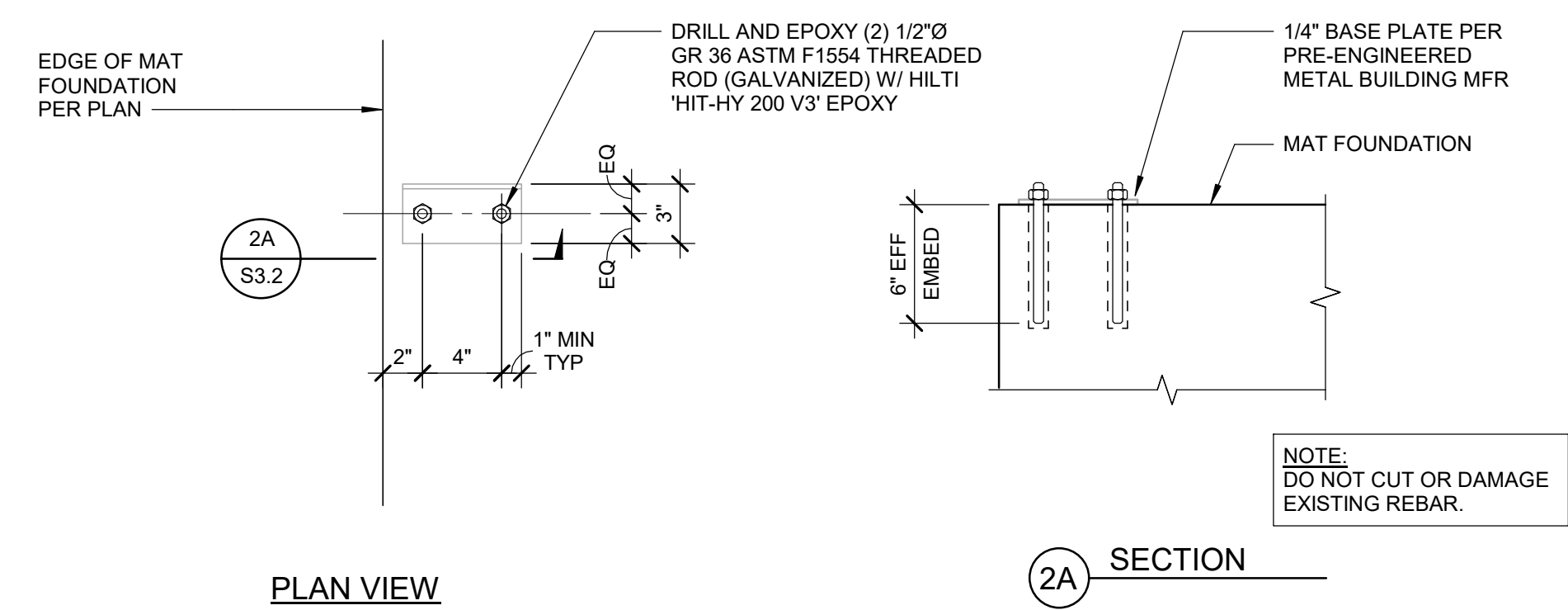
1A PLAN VIEW



3A TROUGH DRAIN SECTION
1" = 1'-0"



3 TROUGH DRAIN BLOCKOUT
SCALE: 1" = 1'-0"



2A SECTION



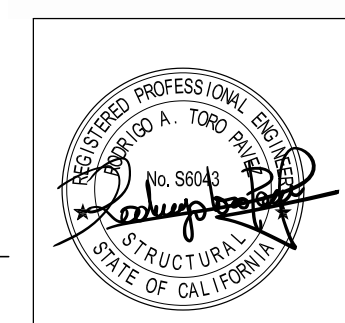
2 ROLLUP DOOR FRAMING ANCHOR (PEMB MFR TAGS F & K)
SCALE: 1 1/2" = 1'-0"

REV.	DESCRIPTION	CKD	APP	DATE
4	ADDENDUM NO. 4			6/22/26
3	PLAN CHECK 3			4/17/26
2	PLAN CHECK 2			2/17/26
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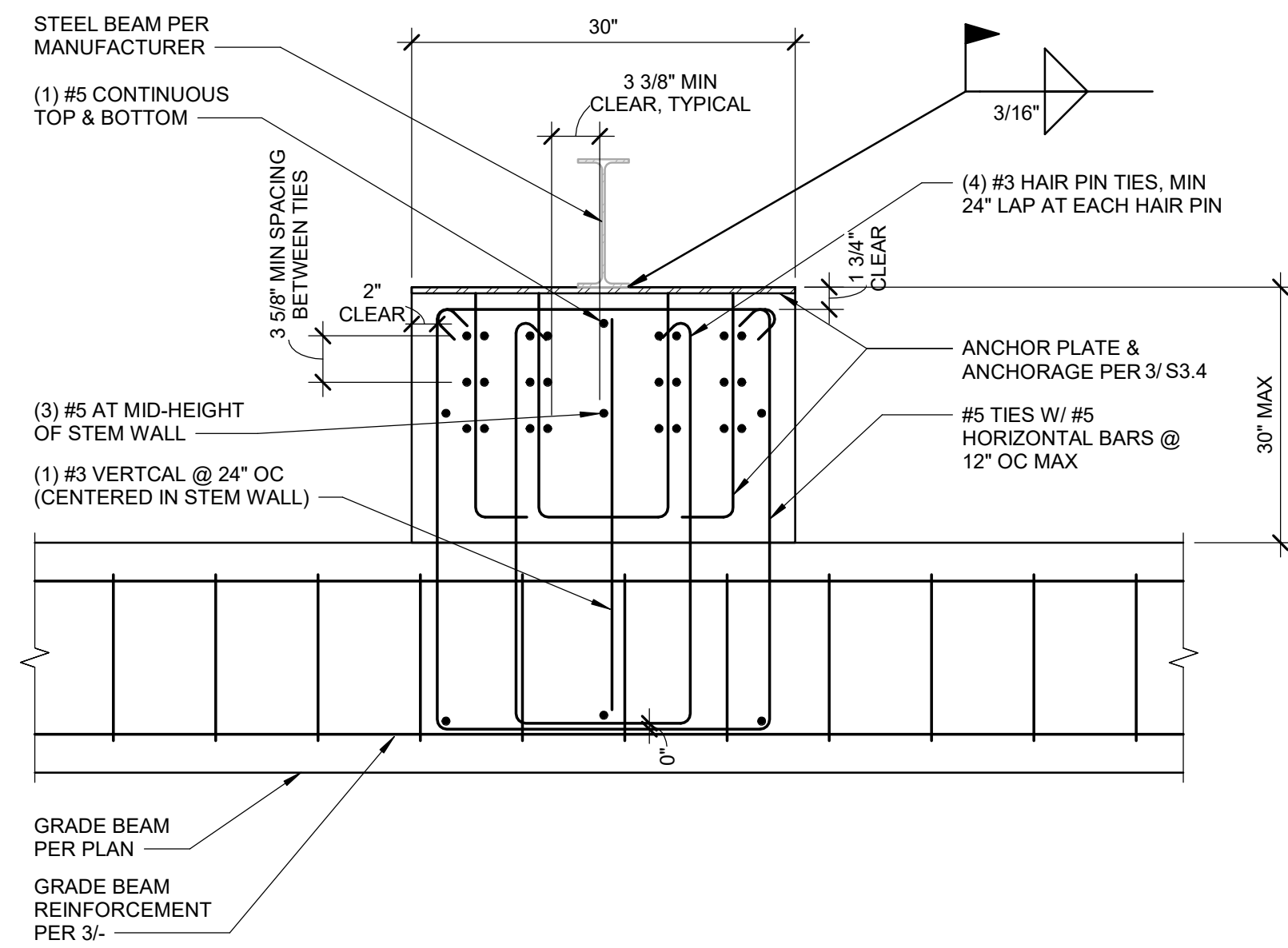
PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION
CITY OF SAN BUENAVENTURA
INTERIM FIRE STATION NO. 7
PEMB ANCHORAGE DETAILS

DRN. BY: Author	DES. BY: Designer	CKD. BY: Checker
-----------------	-------------------	------------------

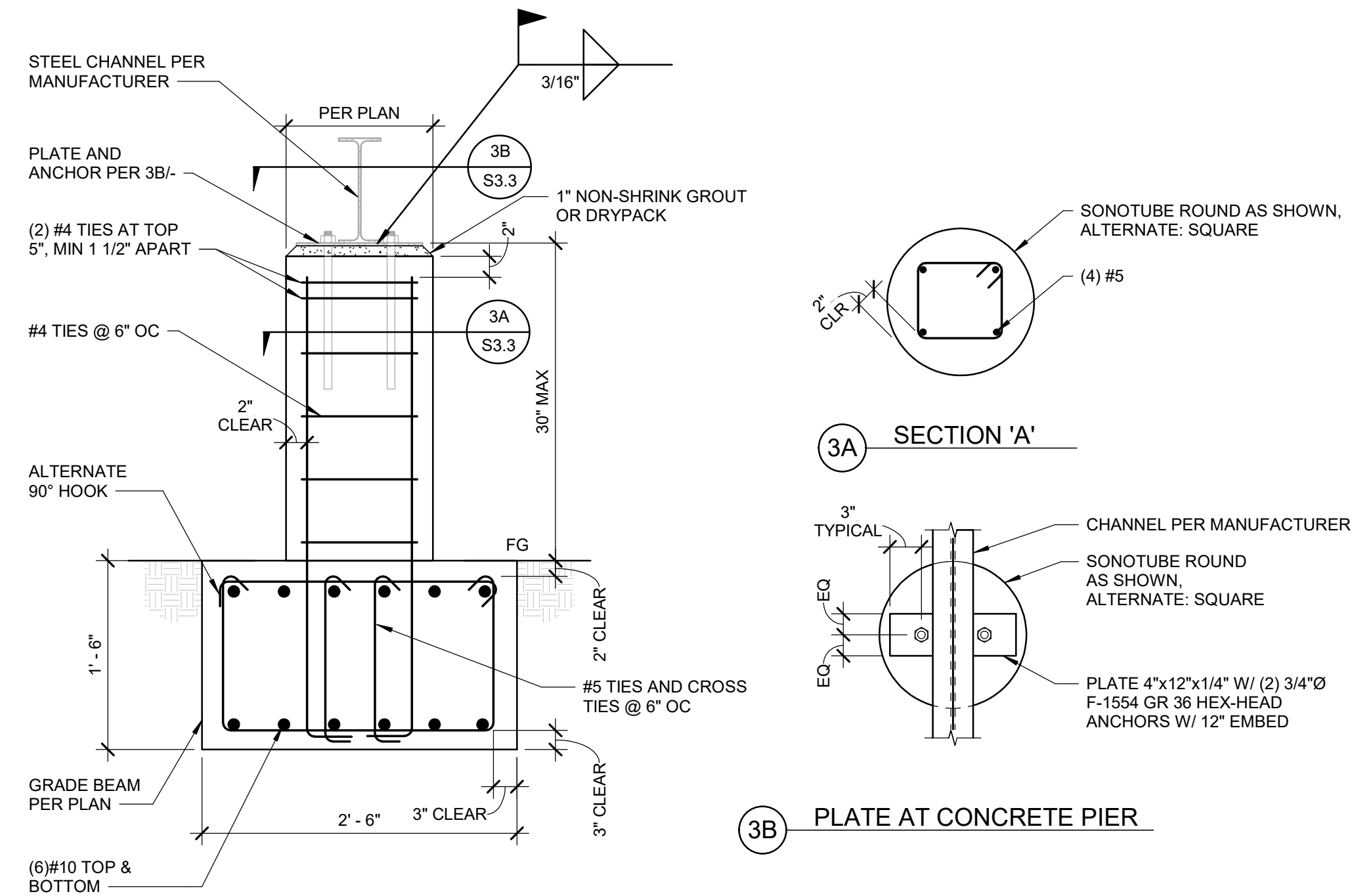
PRINCIPAL ENGINEER	DATE
CITY ENGINEER	DATE
SPEC. NUMBER 2025-010	PROJECT NO. FA11-1061
SEPTEMBER 7, 2025	SHEET 31 OF 151 DWG. NO. 2025-D-010



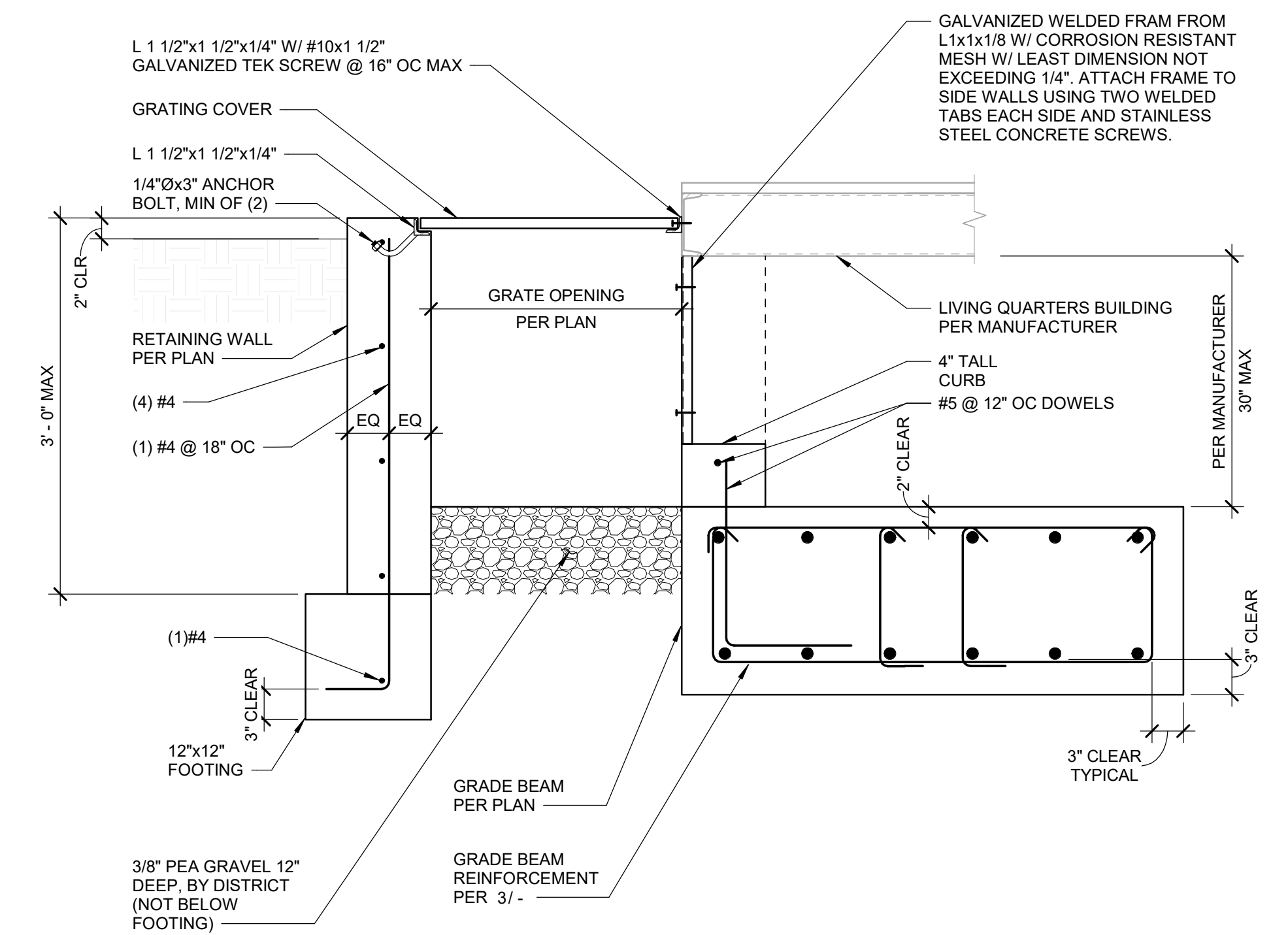
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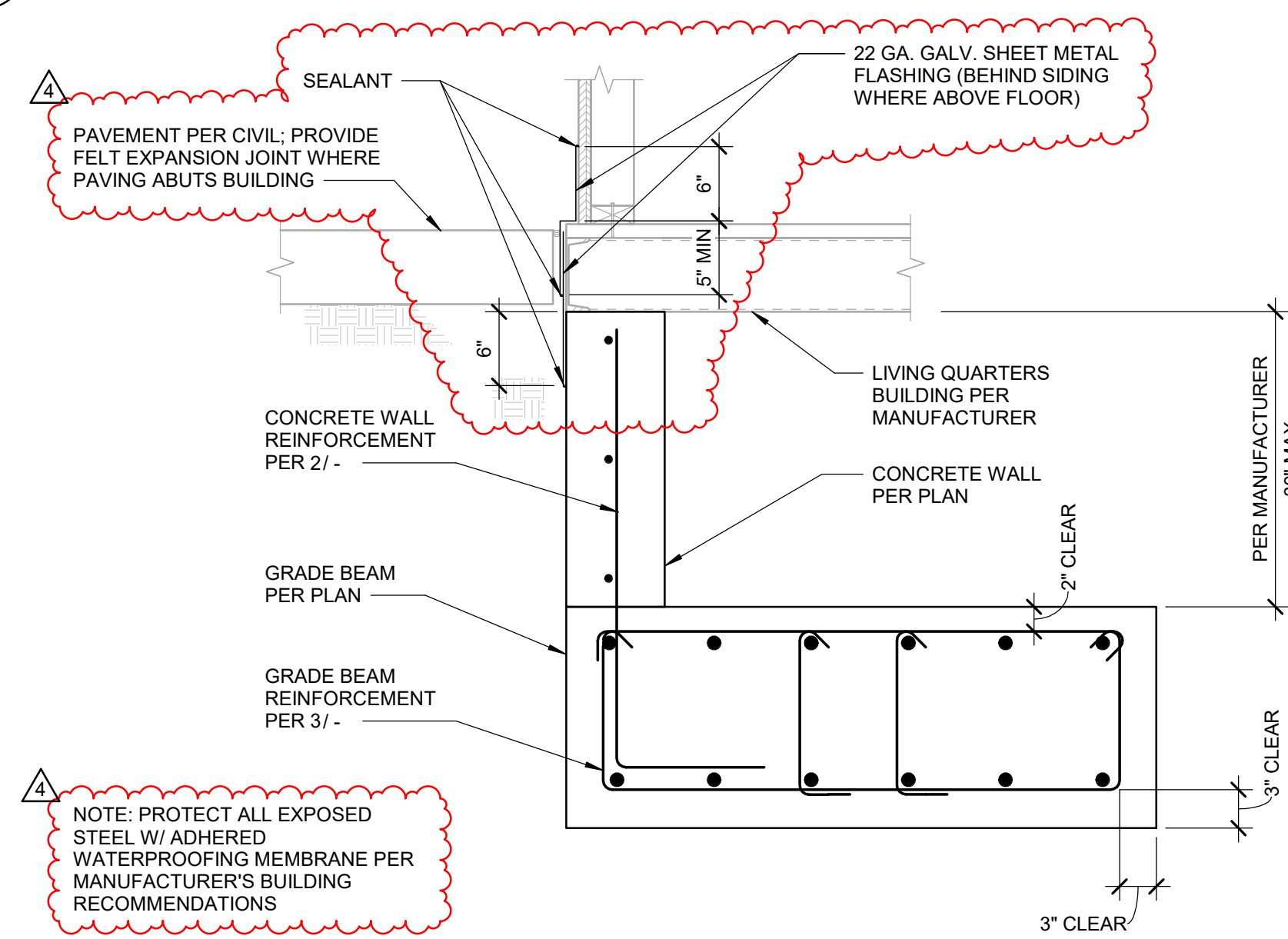
5 INTERIOR SECTION AT MODLINE
SCALE: 1" = 1'-0"



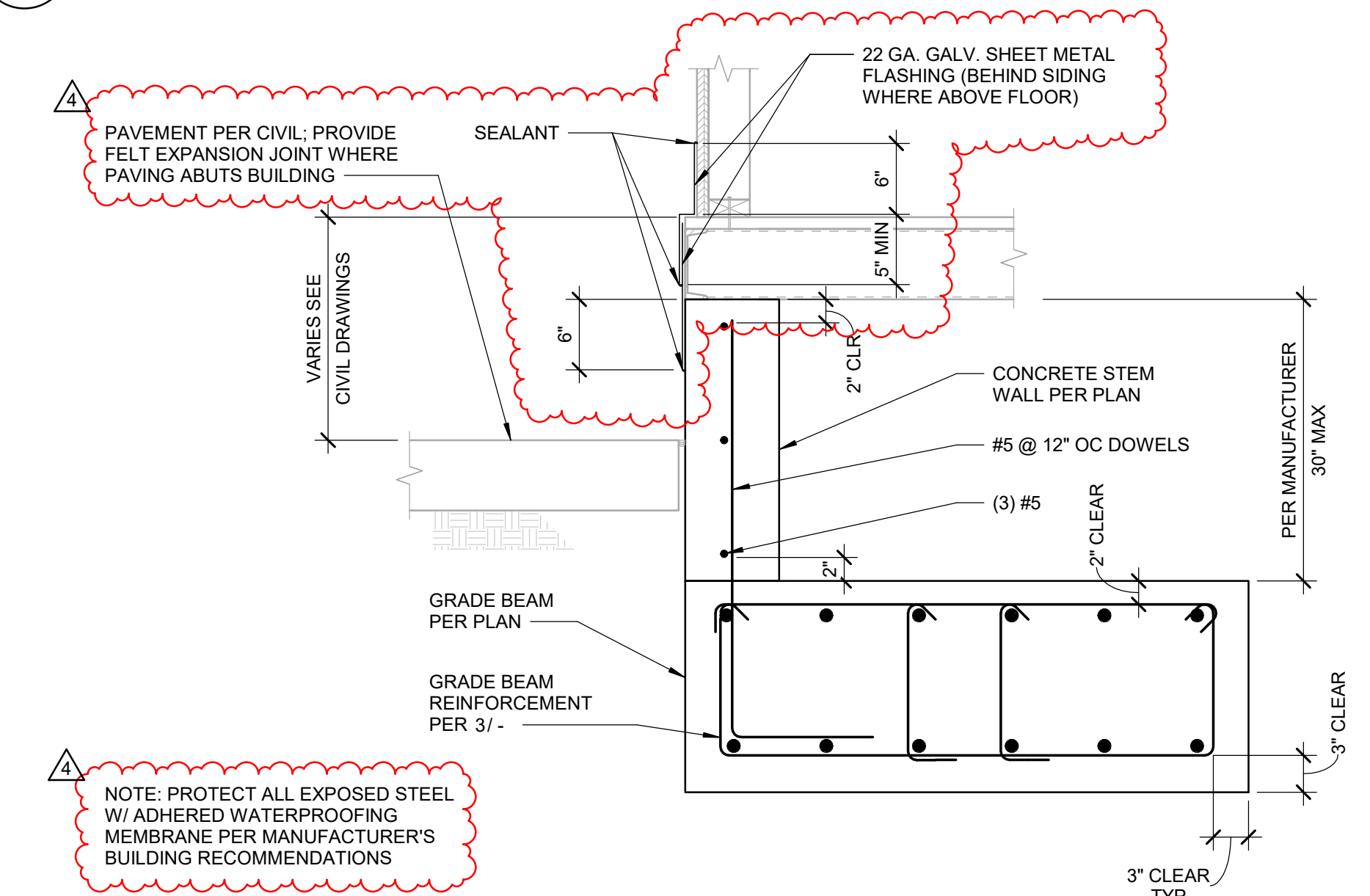
3 INTERIOR SECTION AT MODLINE
SCALE: 1" = 1'-0"



1 DETAIL
SCALE: 1" = 1'-0"



4 DETAIL
SCALE: 1" = 1'-0"

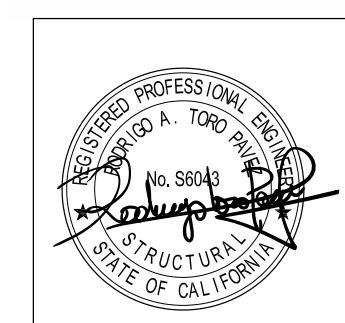


2 DETAIL
SCALE: 1" = 1'-0"

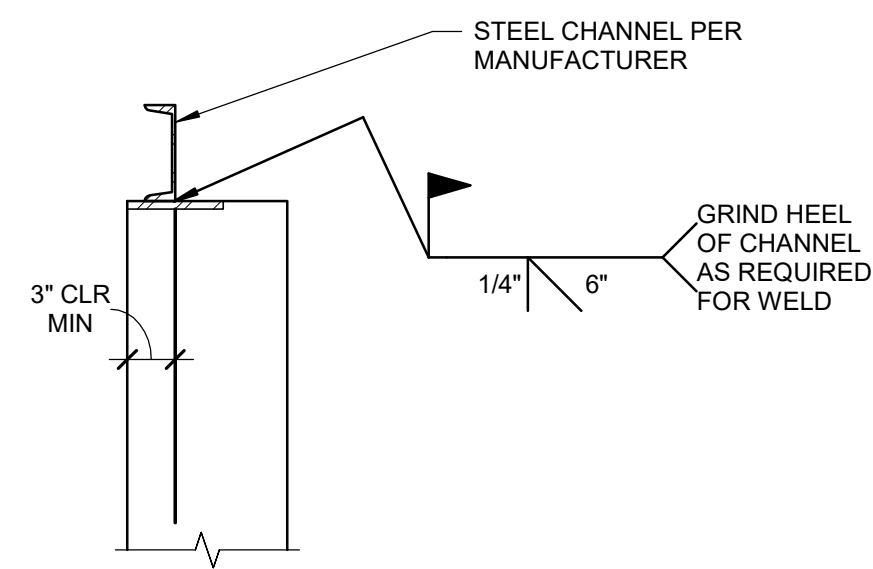
4	ADDENDUM NO. 4	6/22/26
3	PLAN CHECK 3	4/17/26
2	PLAN CHECK 2	2/17/26
1	PLAN CHECK 1	8/11/25

REV.	DESCRIPTION	CKD	APP	DATE
PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION				
CITY OF SAN BUENAVENTURA				
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MODULAR DETAILS				
DRN. BY:	Author	DES. BY:	Designer	CKD. BY: Checker

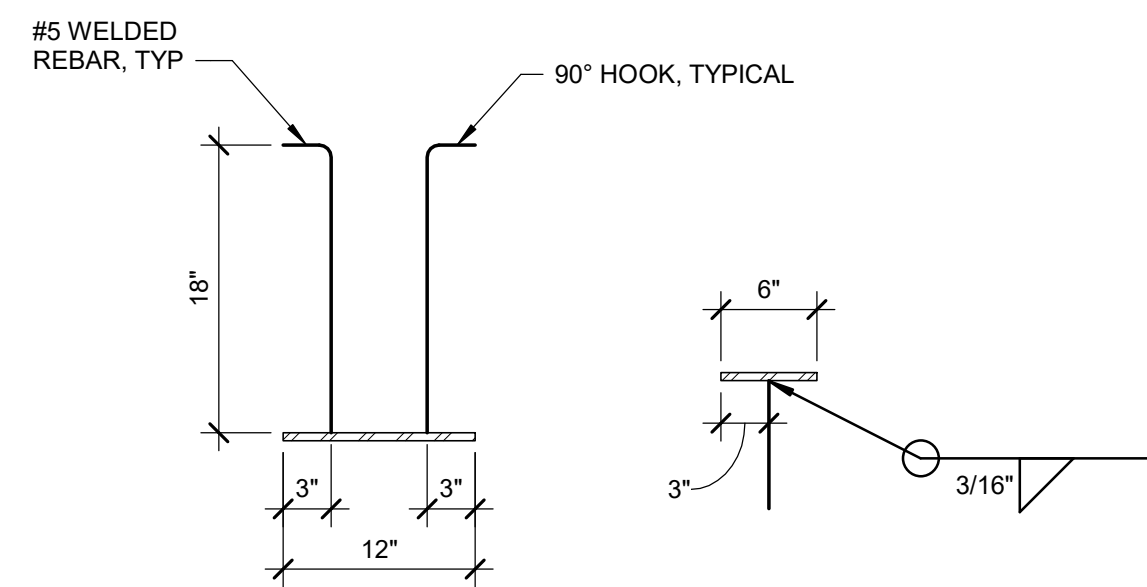
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SPEC. NUMBER	2025-010	PROJECT NO.	FA11-1061
SEPTEMBER 7, 2025	SHEET 32 OF 151	DWG. NO.	2025-D-010

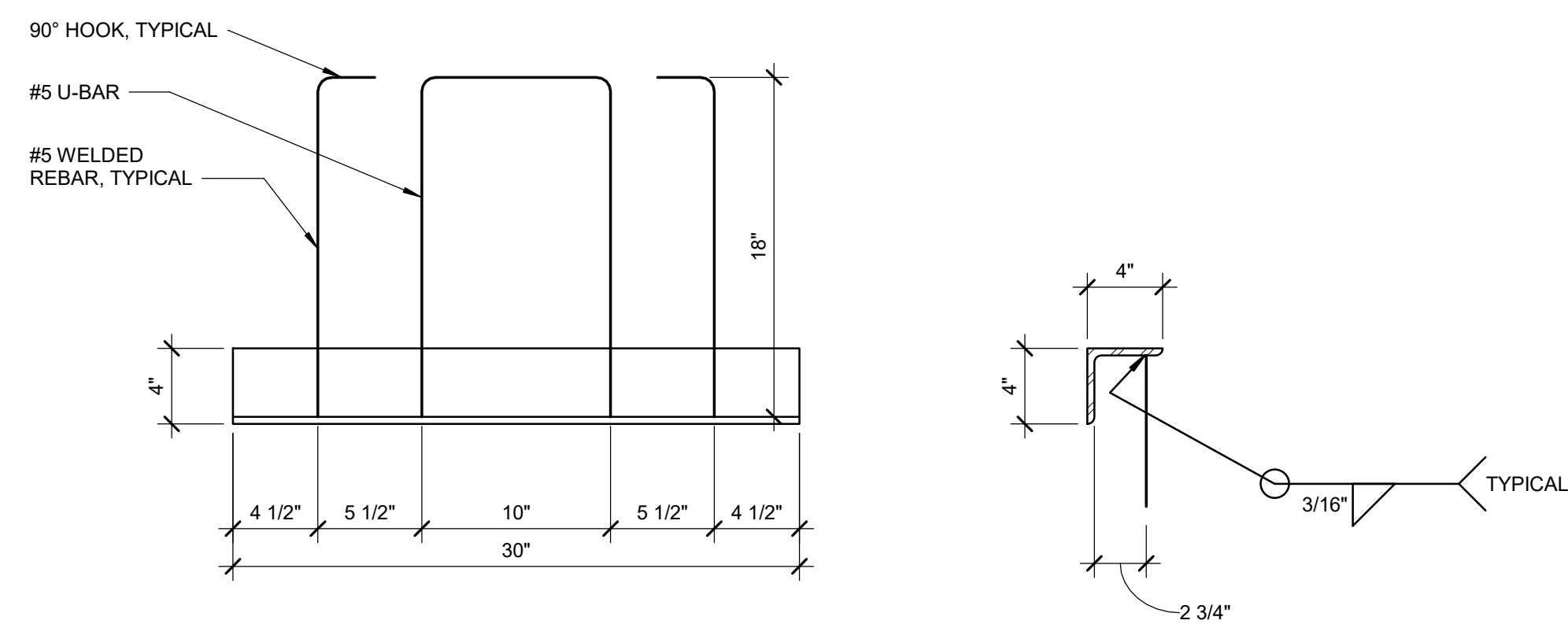


5A ANCHOR PLATE CONNECTION

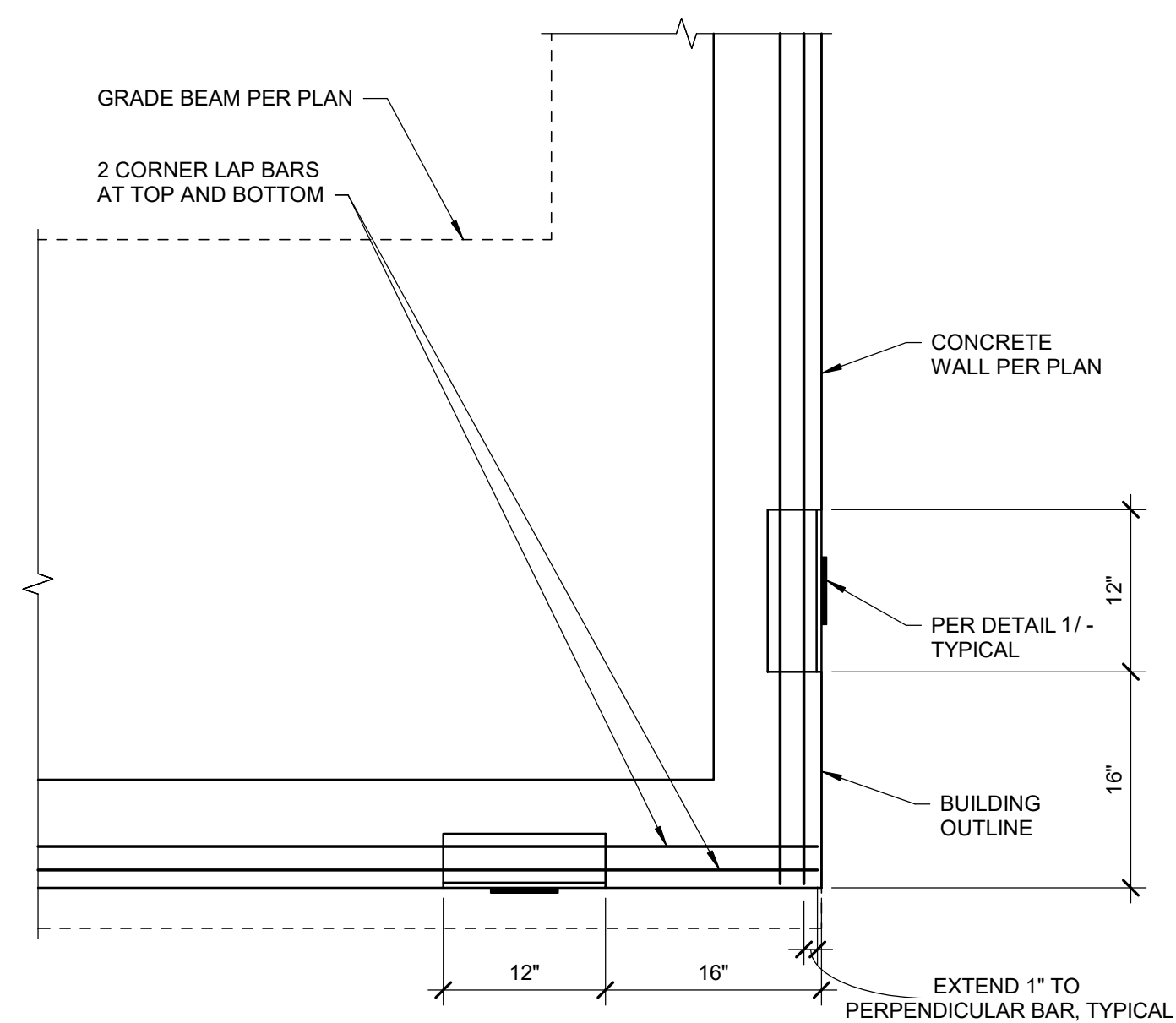


5B ANCHOR PLATE ELEVATIONS

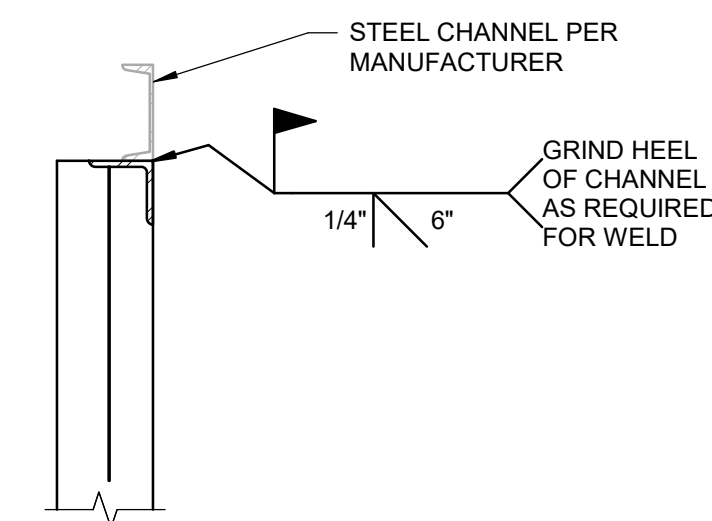
5 ANCHOR PLATE AT GRID A.4 DETAIL
SCALE: 1" = 1'-0"



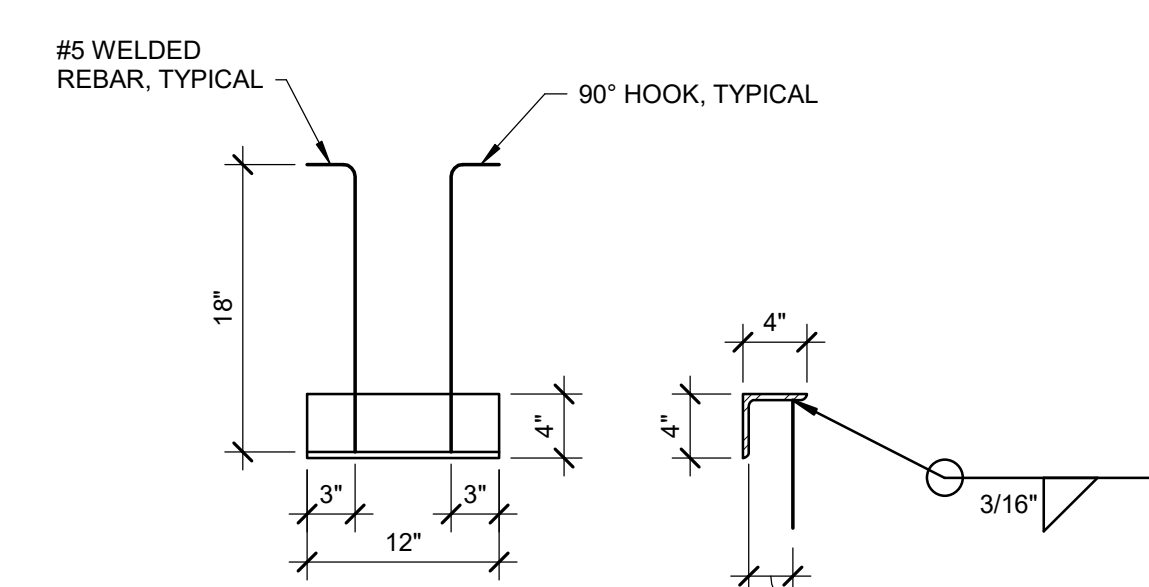
3 ANCHOR PLATE AT MODLINE DETAIL
SCALE: 1 1/2" = 1'-0"



4 ANCHOR PLATE AT CORNER DETAIL
SCALE: 1" = 1'-0"

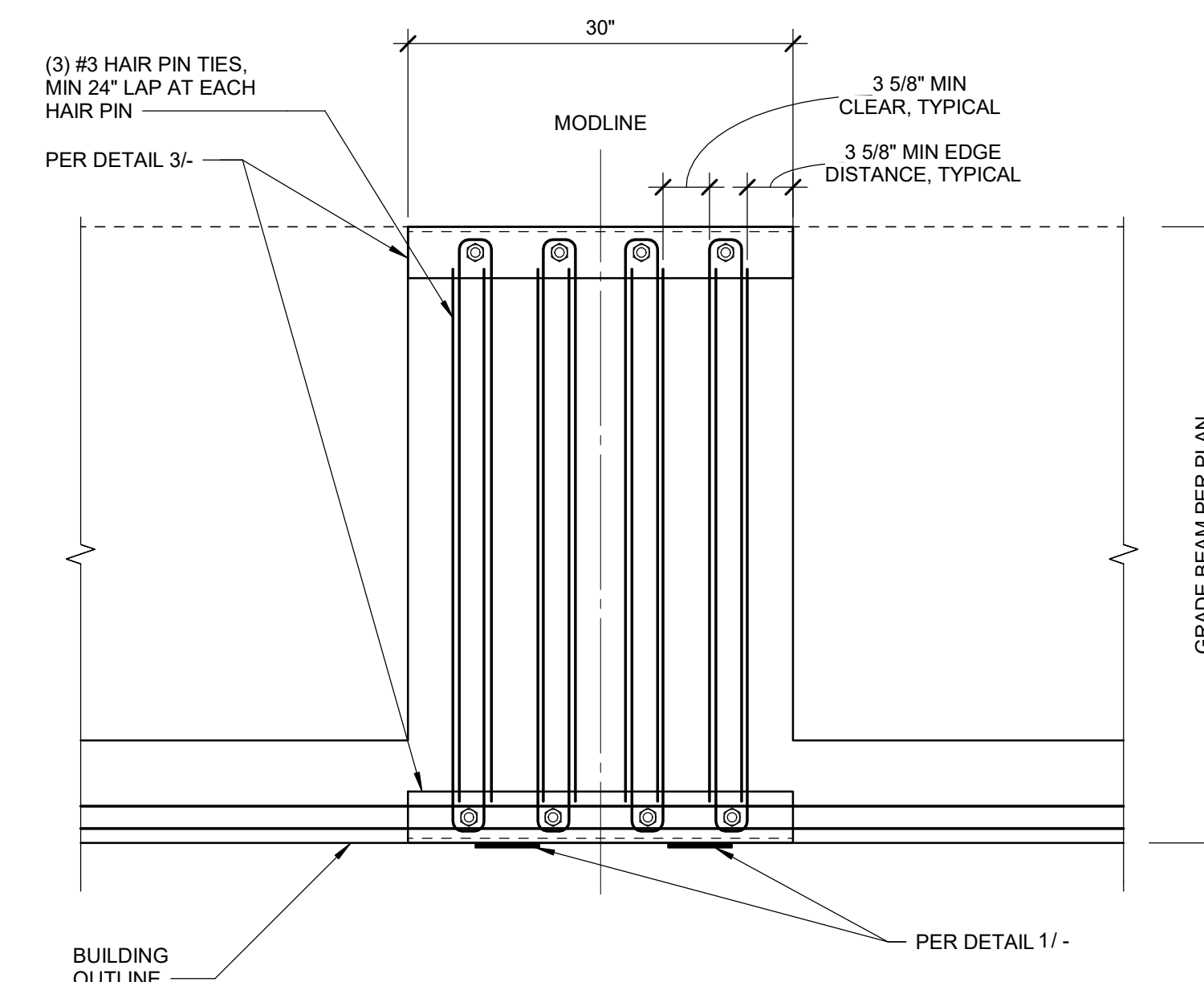


1A ANCHOR PLATE CONNECTION



1B ANCHOR PLATE ELEVATIONS

1 ANCHOR PLATE DETAIL
SCALE: 1" = 1'-0"

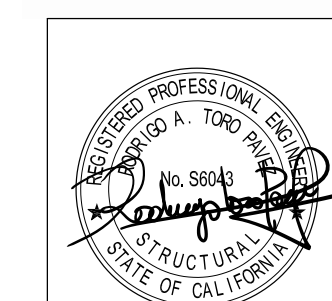


2 ANCHOR PLATE AT MODLINE DETAIL
SCALE: 1" = 1'-0"

4	ADDENDUM NO. 4	6/22/26
3	PLAN CHECK 3	4/17/26
2	PLAN CHECK 2	2/17/26
1	PLAN CHECK 1	8/11/25

REV.	DESCRIPTION	CKD	APP	DATE
PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION CITY OF SAN BUENAVENTURA				
INTERIM FIRE STATION NO. 7 MODULAR DETAILS				
DRN. BY:	Author	DES. BY:	Designer	CKD. BY: Checker

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PRINCIPAL ENGINEER	DATE
CITY ENGINEER	DATE
SPEC. NUMBER 2025-010	PROJECT NO. FA11-1061
SEPTEMBER 7, 2025	SHEET 33 OF 151 DWG. NO. 2025-D-010